

PROJECT CS-1528

WATER MASTER PLAN UPDATE

FINAL REPORT



**CDM
Smith**

Prepared For:

Detroit Water and
Sewerage Department

August 14, 2015

THIS PAGE INTENTIONALLY LEFT BLANK.

This report was prepared by the Water Master Plan Update consulting team

CDM Smith Michigan Inc.

Project Innovations, Inc.

Public Sector Consultants, Inc.

SERCH Consultants

Tetra Tech of Michigan, PC

Tucker, Young, Jackson, Tull, Inc.

Walter Grayman Consulting Engineer

THIS PAGE INTENTIONALLY LEFT BLANK.

Table of Contents

EXECUTIVE SUMMARY	1
THE IMPORTANCE OF AN UPDATED WATER MASTER PLAN	1
SOLUTIONS TO MANAGE COST AND RESPOND TO FUTURE CHANGE	2
CUSTOMER INVOLVEMENT STEERED THE PLANNING AND PRIORITIES	3
MARKET PLAN: FORECASTING FUTURE WATER SALES	4
ADJUSTMENTS TO THE PLAN WILL EMERGE FROM MARKET INDICATORS	8
WATER QUALITY COMPLIANCE AND LONG TERM COST MANAGEMENT	9
COST-EFFECTIVE CONSOLIDATION OF WATER TREATMENT PLANTS	10
OPTIMIZING SERVICE DELIVERY	11
MAJOR CONCLUSIONS AND RECOMMENDATIONS	12
1 INTRODUCTION	15
1.1 BACKGROUND.....	15
1.2 GOALS OF THE MASTER PLAN UPDATE	15
1.3 INTERIM REPORTS MAXIMIZED CUSTOMER REVIEW AND INVOLVEMENT	15
1.4 FORMAT AND DISTRIBUTION OF THIS REPORT	16
2 PLANNING CRITERIA	17
2.1 PLANNING PERIOD	17
2.2 PLANNING AREA.....	17
2.3 LEVEL OF SERVICE GOALS.....	17
2.4 POPULATION PROJECTIONS	17
2.5 BASIS OF COST ESTIMATES.....	22
3 WATER DEMAND PROJECTIONS	25
3.1 GENERAL.....	25
3.2 WATER SALES AND PRODUCTION TRENDS.....	28
3.3 CONTRACT STATUS OF WHOLESALE CUSTOMERS	32
3.4 METHODOLOGY FOR WATER DEMAND PROJECTIONS.....	37
3.5 DOMESTIC DEMANDS.....	38
3.6 BUSINESS WATER DEMANDS	39
3.7 OUTDOOR IRRIGATION.....	39
3.8 NON-REVENUE WATER.....	40
4 GROWTH AND DEMAND MANAGEMENT SCENARIOS	43
4.1 GENERAL.....	43
4.2 MAJOR FACTORS AND TRENDS.....	43
4.3 BASELINE PROJECTIONS AND HIGH AND LOW ESTIMATES	47
4.4 ANNUAL TRACKING, REPORTING AND 5-YEAR ASSESSMENT	48
5 SOURCE PROTECTION AND WATER QUALITY	51
5.1 OVERVIEW OF THE SOURCE PROTECTION AND WATER QUALITY PLAN.....	51
5.2 REGULATIONS GOVERNING SOURCE WATER.....	52
5.3 THREATS TO SOURCE WATER QUALITY	55
5.4 PLAN FOR SOURCE WATER PROTECTION.....	58
5.5 REGULATIONS GOVERNING DRINKING WATER.....	61

5.6	PLAN FOR DRINKING WATER QUALITY.....	63
6	WATER TREATMENT	73
6.1	SUMMARY OF THE WATER TREATMENT PLAN.....	73
6.2	EXISTING WATER TREATMENT FACILITIES.....	74
6.3	NEEDS ASSESSMENT.....	83
6.4	DISCUSSION OF PARTICULAR TYPES OF IMPROVEMENTS.....	84
6.5	SCREENING AND EVALUATION OF ALTERNATIVES.....	86
6.6	SUMMARY OF COST AND NON-MONETARY FACTORS.....	93
6.7	PROPOSED TREATMENT PROGRAMS.....	97
6.8	PROGRAM TO REDUCE TREATMENT PLANT CAPACITY.....	97
6.9	PROGRAM TO REPURPOSE THE NORTHEAST WATER TREATMENT PLANT.....	100
6.10	PROGRAM FOR REGULATORY COMPLIANCE.....	119
6.11	PROGRAM FOR RENEWAL, RELIABILITY AND ENERGY MANAGEMENT.....	120
7	TRANSMISSION	121
7.1	ADAMS BRANCH.....	121
7.2	24-MILE ROAD.....	123
7.3	DOWNRIVER.....	125
7.4	PHASE 1 CONCLUSIONS.....	128
7.5	ENERGY MANAGEMENT.....	137
7.6	PHASE 2 EVALUATIONS.....	144
7.7	PROPOSED TRANSMISSION PROGRAMS.....	164
7.8	PROGRAM TO DECOMMISSION CERTAIN BOOSTER PUMPING STATIONS.....	164
7.9	PROGRAM TO OPTIMIZE DELIVERY TO CERTAIN CUSTOMERS.....	167
7.10	PROGRAM TO IMPROVE TRANSMISSION REDUNDANCY.....	177
7.11	PROGRAM FOR RENEWAL AND RELIABILITY.....	182
8	DISTRIBUTION.....	189
8.1	RETAIL CUSTOMER SERVICE.....	189
8.2	REHABILITATION AND REPLACEMENT OF DISTRIBUTION MAINS.....	190
8.3	SYSTEM CONSOLIDATION.....	201
8.4	LEAD PIPE SERVICES.....	202
8.5	PROPOSED CAPITAL IMPROVEMENT PLAN.....	203
9	METERING AND NON-REVENUE WATER	205
9.1	TRENDS IN NONREVENUE WATER.....	205
9.2	PRELIMINARY WATER BALANCE FOR FY 2012.....	205
9.3	GOALS FOR NONREVENUE WATER.....	206
9.4	CURRENT PRACTICE FOR METERING AND WATER AUDITS.....	207
9.5	TECHNIQUES FOR MANAGING NONREVENUE WATER.....	209
9.6	WHOLESALE METERING FOR DEARBORN AND HIGHLAND PARK.....	213
9.7	WHOLESALE METERING FOR DETROIT.....	216
9.8	RETAIL CUSTOMER METERING IN DETROIT.....	219
9.9	PROPOSED CAPITAL IMPROVEMENT PLAN.....	219
10	FINANCIAL ANALYSIS.....	221
10.1	INTRODUCTION.....	221
10.2	RECENT FINANCIAL PERFORMANCE.....	222

10.3	KEY PERFORMANCE INDICATORS RELATIVE TO PEER UTILITIES	223
10.4	ORGANIZATIONAL INITIATIVES TO IMPROVE PERFORMANCE.....	225
10.5	REVIEW OF FY 2016 BUDGET AND CAPITAL IMPROVEMENT PLAN.....	227
11	IMPLEMENTATION.....	231
11.1	INTRODUCTION	231
11.2	POLICIES TO IMPLEMENT THE MASTER PLAN.....	231
11.3	PROJECT INITIATION.....	238
11.4	CIP EXECUTION MANAGEMENT.....	240
 APPENDICES		
TM-1	CUSTOMER INVOLVEMENT PROGRAM SUMMARY	
TM-2	REGIONAL POPULATION PROJECTIONS	
TM-3	EVALUATION OF WHOLESALE METERS	
TM-4	PLANNING CRITERIA	
TM-5	WATER DEMANDS	
TM-6	LIFE CYCLE COST ANALYSIS	
TM-7	BLUE WATER ECONOMY	
TM-8	WATERSHED MANAGEMENT AND PROTECTION	
TM-9	DRINKING WATER REGULATIONS	
TM-10	WATER QUALITY MONITORING	
TM-11	HYDRAULIC MODEL	
TM-12	EMERGENCY RESPONSE PLAN	
TM-13	NEEDS ASSESSMENT FOR WATER TREATMENT	
TM-14	NEEDS ASSESSMENT FOR BOOSTER PUMPING AND RESERVOIRS	
TM-15	SUMMARY OF WHOLESALE CUSTOMER PROJECTIONS	
TM-16	ENERGY MANAGEMENT	
TM-17	CAPITAL IMPROVEMENT PROGRAM	

THIS PAGE INTENTIONALLY LEFT BLANK.

Executive Summary

The Importance of an Updated Water Master Plan

DWSD completed its last water master plan in 2004. The 2004 master plan projected growth in water demand and contemplated a potential expansion of the service area. It proposed extensive upgrade and rehabilitation of pipelines, treatment and pumping facilities for anticipated new water demands and regulatory requirements. The capital improvement program (CIP) totaled \$9 billion over the 50-year planning period.

Master planning is typically done in intervals of 20 years or longer. However, in 2013, DWSD acted to accelerate the start of a new plan, in order to consider significant changes since the last master plan. These changes include:

- Water sales have declined significantly in recent years.
- Relatively little population growth is projected for the next 20 years.
- The 2007 Model Contract for wholesale water service has promoted demand management and successfully achieved a modification of peak hour demands among multiple customers.
- DWSD wholesale customers have voiced concerns through the Technical Advisory Committee process that the assumptions for population growth and capital investment underlying the 2004 plan needed to be revised so that investments could be properly structured.
- Since 2012, DWSD has been undertaking a major optimization and reorganization program, which include labor force down-sizing, an overhaul of job classifications, piloting of improved business processes, and new financial, information technology, and asset management systems. The updated master plan needs to optimize the infrastructure in tandem with the progress underway with people, technology and processes at DWSD.
- DWSD's organizational structure is changing:
 - After 36 years, Federal oversight ended in 2013.
 - DWSD is becoming financially independent of the City, as directed by the Federal Court.
 - After years of discussion and planning, a new regional authority, The Great Lakes Water Authority, will assume all of DWSD's regional water and wastewater utility responsibilities, while the City of Detroit will operate its distribution and wastewater collection systems.

DWSD is championing extensive involvement by retail and wholesale customers in the Water Master Plan Update. Retail and wholesale customer steering teams were formed, and a structure

was created for regular engagement with the wholesale customer Technical Advisory Committee and its several specialty work groups.

Solutions to Manage Cost and Respond to Future Change

The Master Plan Update is based on new forecasts of population and water use, a new set of financial conditions, and a commitment to seek collaborative, least cost and affordable solutions through partnership between DWSD and its customers.

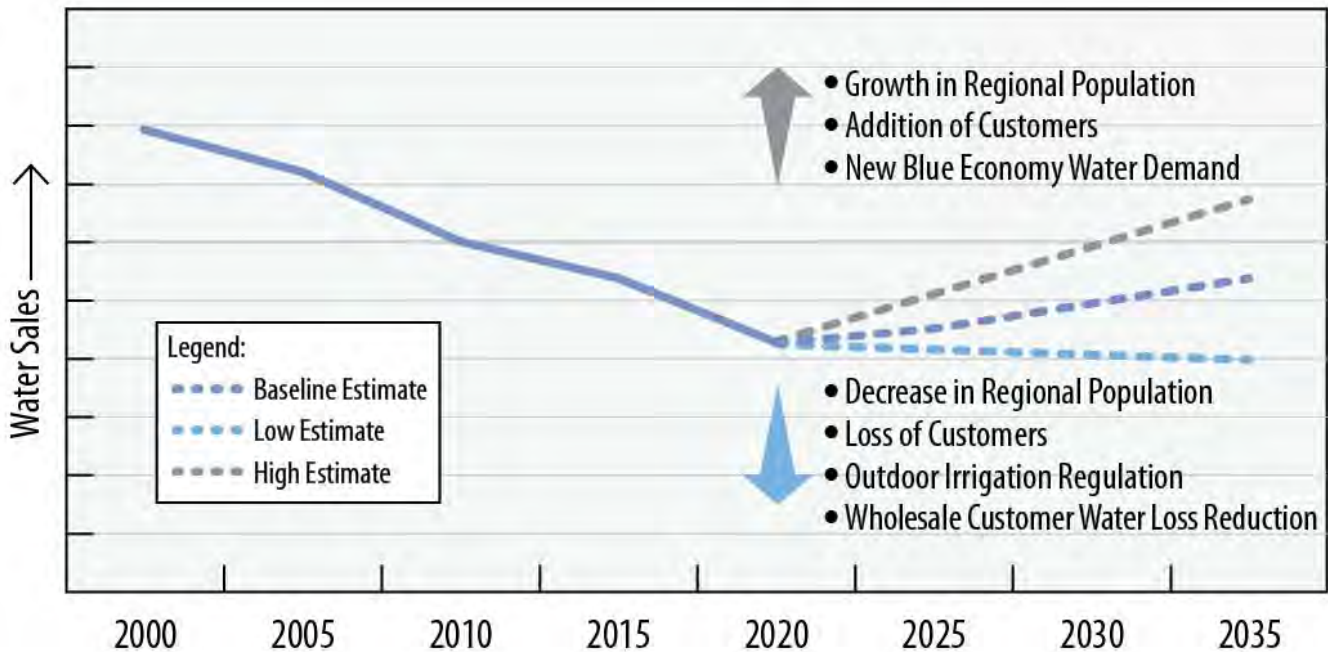
Several factors are driving the cost-saving actions proposed in the master plan:

- The number of people in the region in 2035 will be very similar to what it is today. The service area population will actually be lower than it was in the year 2000, under even the most optimistic projections.
- Water demands for domestic, industrial and commercial users have declined, and average demand per user is likely to decline further in the future.
- New demand management practices have already been implemented and others are expected.
- The City of Flint terminated its water service contract in May 2014, and Genesee County has given notice to terminate service in 2017.

To respond to these factors, a series of future growth scenarios was developed with customer input. As shown in the figure below, the scenarios range from an expansion of the service area and growth in water-dependent industries, to additional loss in wholesale customers and decreasing per capita water use.

At the urging of customers, indicators have been established to monitor future conditions at 5-year intervals. These indicators will provide proactive guidance to assure that DWSD's ongoing investments in infrastructure continue to reflect on-the-ground realities.

Factors Affecting Future Water Demand



Customer Involvement Steered the Planning and Priorities

The Water Master Plan Update was a 24-month project. In order to maximize opportunity for customer and stakeholder involvement, understanding, and input, the plan was prepared as a series of 6 interim reports at scheduled intervals during the project.

The first interim report focused on processes for engaging customers and creating a sense of ownership in the plan. The **Customer Involvement Plan** was prepared in July 2013. It resulted in the formation of formal steering committees of retail customers and wholesale customers. It provided connectivity to the broader wholesale customer Technical Advisory Committee work groups and other stakeholder audiences. It also resulted in the creation of internet-based access to planning meeting summaries, presentations and interim reports.

Retail and wholesale customer committees, specialty work groups, stakeholders, and DWSD staff have been engaged regularly for fact finding and decision making in the planning process, and over 70 meetings and workshops were held.

Customer involvement shaped the recommendations of the second interim report on **Priorities for the First 5 Years**, which was issued in March 2014. This report identified seven specific priority actions for implementation between 2015 and 2020. These actions are summarized below.

1. Reduce water treatment plant capacity to align with projected water demand: The current rated capacity of 1,720 MGD should be reduced to 1,040 MGD. This should be done by a combination of repurposing the Northeast water treatment plant and reducing the capacity of other plants.

2. Reduce non-revenue water: Non-revenue water is the difference between the amount of water produced at the plants and the amount of water actually sold. Non-revenue water is currently 30 percent of water production. Non-revenue water is expected to be substantially reduced through water metering improvements already being implemented by DWSD. Additional longer-term action to reduce leakage is also underway.
3. Reduce energy use and energy costs: Optimize operational practices to reduce electrical demand charges. Extend seasonal operation practices to reduce annual power costs, and apply technology for energy recovery from hydraulic turbines, solar and wind sources.
4. Collaborate with customers to address long standing water transmission problems: Investigations performed by the master planning team have identified solutions to pressure and supply redundancy problems in Wayne, Oakland and Macomb counties. These are collaborative solutions between DWSD and the affected wholesale customers, and the new solutions have significantly lower cost than original estimates.
5. Update water quality goals and monitoring efforts: Minimize the cost of future water quality regulation through new water quality investigations, partnerships, additional water quality monitoring services to wholesale customers, and mutual aid agreements. These types of proactive, collaborative, problem-solving activities are important for managing the long term cost of regulatory compliance.
6. Optimize return on investments using asset management to prioritize capital improvements: New information technology currently being implemented by DWSD can be combined with new condition assessment, inspection and preventive maintenance techniques to extend the service life of equipment. This new approach replaces specific assets rather than systems of assets.
7. Renew and rehabilitate the water distribution system in the City of Detroit: Renewal and rehabilitation of Detroit's water distribution mains will be critical to reducing leakage in the distribution system, reducing the frequency of water main breaks in the future, and maintaining reliable high quality water to the City. Selective retirement of water mains in areas of the City that are vacant is a long term strategy to reduce the cost of distribution system renewal and rehabilitation.

Market Plan: Forecasting Future Water Sales

DWSD's system supplies water to approximately 3.5 million people in 128 communities in Southeast Michigan. Average daily water sales were 372 MGD in Fiscal Year 2014. City of Detroit water sales were 69 MGD, and wholesale customer sales were 303 MGD.

A long term trend in declining water sales is now documented. The reasons for this trend are analyzed and accounted for in this market plan. At the same time, potential changes to the future forecast during the planning period are analyzed, so that expectations of future water sales can be reasonably framed.

Specifically, regional water sales declined 32 percent from an average of 550 MGD in 2000 to 372 MGD in 2014. This decline is a combination of a national trend in reduced per capita water use and regional

economic conditions in Southeast Michigan. Most notably, the regional water service area economy employed 443,000 in manufacturing jobs in the year 2000, but this number is expected to fall to 185,000 by 2035. Historically, these manufacturing jobs also generated high water use. New employment growth is expected to be largely in knowledge-based services and health care which have relatively lower water use than the older manufacturing processes.

Future Projections of Water Service Area Population and Water Use (Most Probable Forecast)

	2010	2015	2025	2035
Population Served (Thousands)	3,600	3,539	3,454	3,542
Average Day Water Demand (MGD)	530	500	490	480
Maximum Day Water Demand (MGD)	1,040	1,000 over the planning period. Weather dependent, may occur every 3 to 5 years.		
Annual Water Sales (Million Gallons)	146,000	139,000	138,000	137,000

When considering the regional market for future water sales, four particular market drivers were considered:

1. Contract commitments of the existing customer base
2. Expansion or contraction of the regional service area
3. Initiatives to expand water-enabled and water-dependent industries (see Blue Economy discussion below)
4. Demand management practices to change demand patterns or reduce water use or loss. The average day demand projections shown above are based on continuing reduction of water loss over the planning period.

These market drivers are discussed below.

Existing Customers Will Provide Most of the Future Water Demand

DWSD's existing customer base is the majority of the anticipated future market. 92 percent of current wholesale water purchases are committed through long-term supply contracts that extend to 2038 or beyond. More long term contractual commitments are expected from most of the remaining wholesale customers purchasing water under an older contract.

A detailed technical survey of wholesale customers was performed to establish a customer perspective of the next 20 years of population and water use. Through a combination of population growth and extension of service areas, wholesale customers anticipate an additional 100,000 people will receive regional service over the next 20 years. This suburban increase will offset a decline in population and water sales in the City of Detroit. The City of Detroit population is projected to decline until 2020, and then slowly increase.

Addition or Loss of Customers in the Regional Service Area

Scenarios for addition or loss of customers in the service area were developed. Their impact on future water sales and system capacity were evaluated. The result: maximum day water demand ranging from 1,100 MGD on the high end to 930 MGD on the low end. The 1,040 MGD maximum day demand represents the most likely future demand.

Following is a summary of potential changes in the future service area:

- Wholesale customers in Genesee County are in transition. The City of Flint ended continuous water purchases in May 2014. Flint is currently under an agreement providing only for emergency service. Genesee County has provided notice to DWSD that it will end water purchases in 2016. Similar to Flint, they will then become an emergency supply customer.
- Lapeer County customers continue to purchase water from DWSD and are in discussion with DWSD regarding water supply beyond 2016.
- Some wholesale customers in the Downriver area have not yet signed a long term service contract.
- An examination of smaller water service providers to the west of the regional water service area was performed to document population growth, source water supply and cost of water service. Based on this analysis, it is possible that some communities in southern Oakland County and northern Washtenaw County could seek water service from DWSD during the planning period.
- Larger water service providers, such as Monroe and Ann Arbor, could seek emergency supply agreements to address seasonal water supply.
- Lastly, some water service providers along the St. Clair River have expressed interest in considering regional water service.

New Blue Economy Water Use

A Blue Economy integrates water resources with jobs and development. On-going initiatives underway in Michigan's Blue Economy intend to grow businesses and jobs in water-enabled and water-related sectors by utilizing the region's abundant resources – water, academia, engineering and manufacturing workforces. Water-enabled and water-related industries are those portions of the economy that produce, implement, or are significantly affected by water research and innovation.

At present, Blue Economy water users represent less than 5 percent of DWSD's annual water sales. Food and beverage companies have particular potential for growth, and these represent half of the current Blue Economy water use. A review was performed of ongoing initiatives, potential for growth, and techniques for stimulating growth in Blue Economy water use. Based on this review, recommendations are presented for a collaborative effort by DWSD, the City of Detroit, Oakland, Wayne and Macomb counties, and business organizations to plan for and attract new Blue Economy businesses to the regional water service area. Over the last 12 years, water use by major industries in the DWSD service area has declined from 45 MGD in 2002 to 23 MGD today.

Blue Economy Initiatives have the potential for restoring some of the losses in industrial water demand with new highly sustainable industrial and commercial water users. However, these changes are not expected to have a major impact on average daily demand, peak daily demand, or the many implementation actions proposed to reduce the system's capacity in order to lower fixed costs.

Demand Management

Demand management is the proactive application of technology or regulation to minimize water use or modify water use patterns. There are four principle categories of demand management actions that will continue to impact DWSD water sales and operational efficiency in the next 20 years.

1. Increasing transition to plumbing fixtures that conserve water
2. More investment in storage of water by wholesale customers to reduce their peak demand
3. Increased use of outdoor irrigation regulations and public education to reduce peak demand
4. Reduction in water loss by fixing leaks and replacing older pipes.

Water conserving plumbing fixtures have been increasingly regulated through building and plumbing codes since 1982. Much of the impact of these regulations has already been experienced, but it can be expected to continue to impact per capita domestic water use through the planning period. Current estimates of domestic water use in the DWSD service area range from 58 gallons per capita per day (GPCD) to 77 GPCD. A range of 50 to 60 GPCD is possible by the end of the planning period.

Wholesale customers are building storage to reduce the cost of peak hour water purchases from DWSD. Wholesale customer storage currently totals 95 million gallons at 30 locations. 15 million gallons has been built in the last 7 years and an additional 9 million gallons of storage is planned for the remainder of the planning period.

The impacts of this type of storage are significant in two ways:

1. It reduces costs to customer communities as well as peak hour pumping requirements and associated energy costs for DWSD.
2. Reduction in revenue to DWSD, because peak hour rates have less impact.

45 of DWSD's wholesale customers have regulations or have public education programs to mitigate peak hour water use for **outdoor irrigation**. A detailed evaluation of the quantitative impact of these regulations and public education programs has not been performed. However, peak hour water demand has fallen by more than 200 MGD in the last 10 years, and this can be attributed to a combination of peak hour storage, outdoor irrigation regulation and public education.

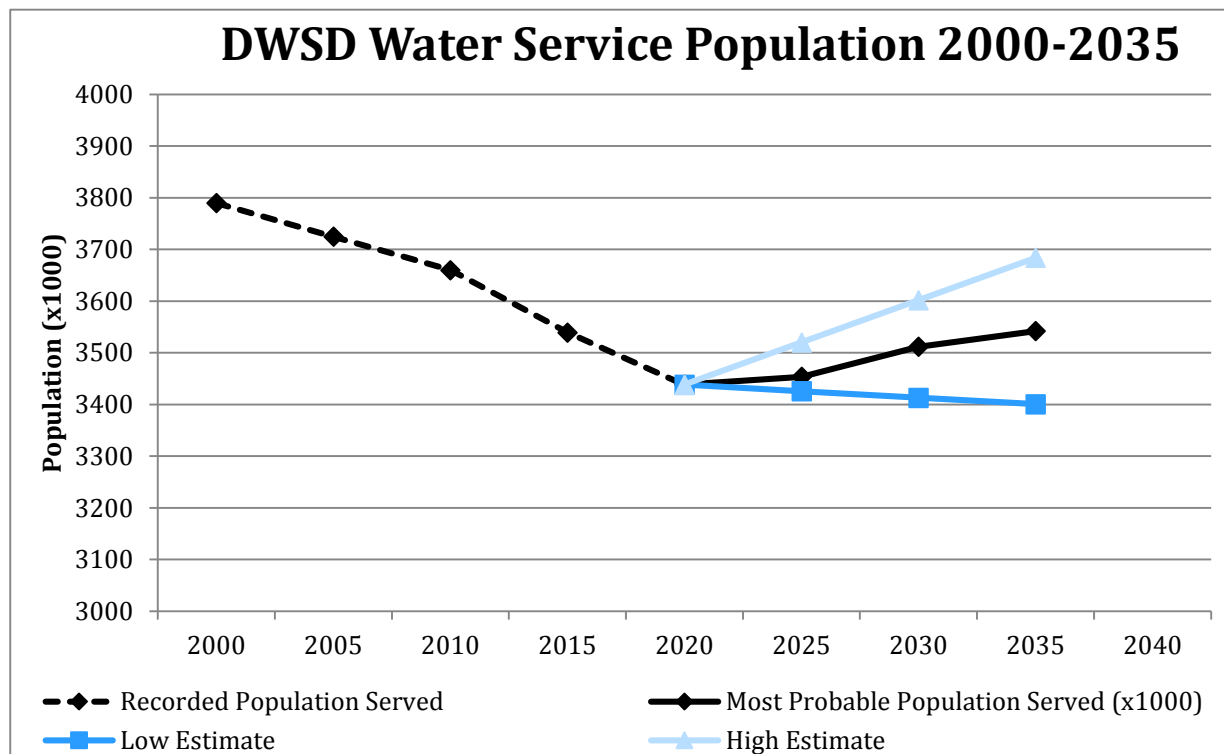
A final component of demand management is **reduction in water loss**. The first interim report on *Priorities for the Next 5 Years* identified reduction in non-revenue water as a key priority. Real water loss through leakage in the City of Detroit is estimated to be 50 to 80 MGD. DWSD is vigorously applying a number of methods to find and eliminate leakage.

Adjustments to the Plan Will Emerge from Market Indicators

The key driver for DWSD’s future water sales is population change. Other factors include changes in the service area, trends in domestic water use, industrial water use, outdoor irrigation trends, and outdoor irrigation practices.

The figure below shows service area population from 2000 to 2015, along with projections to 2035.

The black line is the most probable projection based on input from wholesale customers and the Southeast Michigan Council of Governments (SEMCOG). The light blue line is the high projection, and the darker blue line is the low projection, both based on SEMCOG’s range of future projections. Most noteworthy is that service area population under any 2035 scenario is still less than the service area population in 2000.



In order to monitor and assess future trends, the following indicators are proposed for tracking on an annual basis. See Chapter 4 for a discussion of why these indicators were selected.

- Employment trend and forecast
- Population trend and forecast
- Major industrial water use
- Per capita domestic water use
- Water production

- Water sales
- Reduction in water loss through leak reduction and pipe replacement
- Customer updates on projected water purchases
- Customer satisfaction surveys and water service contract execution
- Federal and state regulations

It is proposed that a customer outreach work group be convened in 2020 to assist in performing the first 5-year assessment of indicators and trends. Upon completion of that effort, the work group should recommend a process for the subsequent updates in 2025 and thereafter. Prior to the first formal plan review in 2020, a customer outreach workgroup should assess progress toward achieving the first five-year goals in late 2017.

The forecast of growth, the range of scenarios, and 5-year monitoring of indicators provide a guide map that will allow DWSD to continue its focus on optimization of facilities, work force development, improving customer service and efficiency. DWSD can continue to make informed decisions for infrastructure investment, and to make these investments in measured steps to meet objectives in management of cost of service and affordability.

Water Quality Compliance and Long Term Cost Management

The Product Plan includes actions over the next twenty years to protect source water quality, stay in compliance with drinking water regulations, update water quality goals, and update the monitoring and emergency response plans.

Major elements of the source projection and water quality plan include the following future actions:

1. Update the Source Water Assessment, which was originally prepared in 2002.
2. Prepare Surface Water Intake Protection Plans for all three intakes
3. Expand participation in Partnership for Safe Water, MiWARN, and the Huron to Erie Monitoring System.
4. Increase the number of water quality staff to increase DWSD's role in these partnerships, and to perform the new monitoring and special water quality studies recommended herein.
5. Update water quality goals for 2016 to 2025 with emphasis on repurposing and optimizing conventional filtration capacity at four plants and converting to direct filtration at the Lake Huron plant.
6. After 2025, update water quality goals again in response to new regulations that may take effect, and for new initiatives for multi-barrier disinfection and conversion from chlorine gas to sodium hypochlorite.

The plan proposes goals that in some cases go beyond existing minimum federal and state regulatory requirements, but are consistent with the best practices of major peer water utilities.

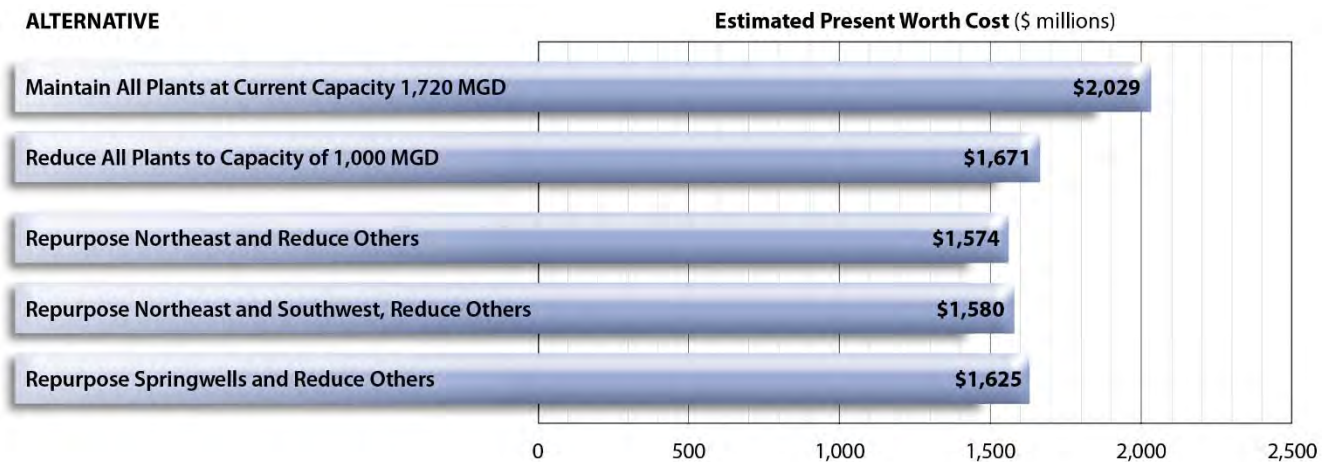
Cost-Effective Consolidation of Water Treatment Plants

DWSD owns and operates 5 water treatment plants. These plants have a design capacity of 1,720 MGD, they are in compliance with current regulatory requirements, and they produce high quality water, but they have over 700 MGD excess capacity relative to projected water demands during the planning period.

A needs assessment for all water treatment plants was performed in order to determine the most cost-effective approach to reduce excess capacity and maintain or improve the level of service to customers throughout the planning period. Improvements were identified for regulatory compliance, safety, energy and operational efficiency. The projected capital needs for all 5 plants are approximately \$997 million over the planning period in today’s prices.

In addition to the needs assessment, a life cycle cost analysis was performed to consider O&M as well as capital needs, the cost to decommission and repurpose plants, the cost to build new transmission mains for water supply, and the cost to assure a level of redundancy typical of large water utilities.

Life Cycle Cost Comparison of Alternatives for Water Treatment Plant Consolidation



The figure shown above summarizes the life cycle cost analysis and shows that repurposing the Northeast and Southwest plants is the alternative with the lowest life cycle cost. A detailed review of the costs of repurposing Northeast and Southwest plants shows that under some economic forecasts, the most cost-effective alternative is to repurpose only the Northeast plant and continue to operate the Southwest plant. Therefore, it is proposed that DWSD move forward to reduce active treatment capacity in all of its plants to a total of 1,040 MGD by 2020, and by the same time repurpose the Northeast water treatment plant for only reservoir and high lift pump operations. After 2020, with new information available, re-examine the cost effectiveness of continuing to operate the Southwest plant.

DWSD has already taken the initial steps in its FY2016 CIP to reduce the capacity of its water treatment plants. The FY2016 CIP was developed with substantial input from the Water Master Plan Update.

The new consolidation and repurposing program will reduce DWSD's costs for operation and maintenance, as well as reduce its immediate capital costs for plant upgrades and long term costs for regulatory compliance. This program builds on DWSD's ongoing initiatives for optimization, asset management, and customer service, and it maintains high level of service standards in drinking water quality, pressure and volume.

Optimizing Service Delivery

The service management plan proposes changes to the transmission system to efficiently meet wholesale customer contract requirements and projected 2035 water demands. The changes to the transmission system allow for the repurposing of the Northeast Water Treatment Plant and the decommissioning of older booster pumping stations that were originally part of the Wayne County water system and acquired by DWSD in 1960. Pumping capacity in DWSD's high lift pumping stations exceeds 2,500 MGD, but future requirements are 1,500 MGD, so substantial idle capacity can be removed from service to reduce costs for preventive maintenance and electrical demand charges, and reduce costs of future capital upgrades. In conjunction with these changes, the Service Management plan recommends several improvements to add transmission redundancy and new isolation gates on transmission mains. These improvements will add reliability to locations which have historically been areas of concern and risk for the utility and its wholesale customers. DWSD is aggressively moving forward to reduce nonrevenue water, and the Service Management Plan recommends additional short term and long term projects and operational actions to reach a non-revenue water goal of 15 percent by 2035.

Changing the treatment and transmission system is complex, and must be done in a carefully planned and sequenced approach. Therefore, the Service Management Plan identifies specific projects and a project sequence and schedule to accomplish the goals of the master plan. The Service Management Plan also anticipates the departure of Genesee County as a wholesale customer in July 2016, and includes projects to maintain water quality to wholesale customers who will remain served by the 72-inch main to Flint.

DWSD is in transition to two successor agencies: the Great Lakes Water Authority for the regional system, and the City of Detroit for retail water service in the City of Detroit. The Service Management Plan identifies specific capital projects for GLWA on the regional system and specific capital projects for the City of Detroit for the work within the City. The capital improvement plan for this Master Plan Update includes a total of 320 capital projects for the next 20 years. The first five years of the planning period, 2015 to 2020, will provide new information and new ideas from DWSD's successor agencies. Implementation of the Service Management Plan should be reviewed annually based on progress and new conditions. As provided in other aspects of the plan, formal reviews of the 20-year plan should be conducted every 5 years to account for new information and changes from anticipated conditions.

Major Conclusions and Recommendations

Recommendations of the master plan are presented in the form of a 20-year capital improvement program. The 20-year CIP is presented in Technical Memorandum 17, and in summary form in Chapter 10. Chapters 6, 7, 8 and 9 describe the major analyses, alternatives and important early action projects. Chapter 11 provides recommendations to drive implementation of the plan and expansion of the customer base. These policies include planning, water rates, water audits, wholesale customer service, and asset management.

The recommended capital improvement plan is approximately \$2.9 billion over the next 20 years. On an annual basis, the capital improvement program for the GLWA averages \$120 million annually, and for the City of Detroit retail, it averages \$25 million annually.

Implementation of the recommended projects in the Master Plan Update will provide the following benefits to customers in the service area:

Reinforces the level of service

- DWSD's traditionally high level of service in supplying superior quality water and meeting retail and wholesale customer requirements for pressure, volume and water quality and reliability would be reinforced.
- Improved procedures have been developed for emergency response, if a local and short term service disruption requires a reduced water supply in the impacted area.

Right-sizing treatment and pumping capacity will reduce future capital costs and operating costs

- The proposed 20-year capital improvement program is \$500 million less expensive (life cycle cost basis) than the cost of continuing to maintain and operate all existing water treatment plants at today's capacity.
- In addition to water treatment plant savings, the proposed decommissioning of certain booster pump stations and right-sizing high lift pumping will reduce capital costs by another \$90 million over the planning period.

Provides flexibility for a range of future scenarios

- Growth projections show the need to right-size treatment plant capacity to approximately 1,040 MGD maximum day demand and average day of 480 MGD.
- The right-sizing of plants will be done in a way so that if there is unexpected growth or expansion of today's service area, treatment capacity can be restored cost-effectively.
- Trends in water consumption for households and business have been decreasing in recent years. If these trends continue, the plan sets forth a pathway to reduce treatment and pumping capacity further.
- Improves transmission redundancy and reliability

- Proposed transmission system improvements will increase redundancy of supply to each operating zone.
- Use of inter-customer connections have been proposed as low cost solution to certain redundancy needs.
- Three water intakes will be maintained, and emergency level short term water supply can be provided with any one plant out of service.

Optimizes service delivery to wholesale customers

- The existing service area of the Northeast water treatment plant will be supplied from the Lake Huron plant and the Water Works Park plant.
- Transmission constraints will be eliminated at Water Works Park plant, and this plant will cost-effectively provide finished water to the repurposed Northeast high lift pump station
- Rehabilitation of existing pumps in the Water Works Park high lift station will optimize the station for both low demand and high demand periods.
- Changing certain flow splits within wholesale customer contracts provides a mechanism for future savings, particularly energy savings

Proactive approach to manage the cost of compliance with future regulations

- DWSD has already begun work on Surface Water Intake Protection Program (SWIPP) for each of its intakes and analyzing requirements for chlorine boosting for Lapeer City and Mayfield.
- A pilot study for a direct filtration process at Lake Huron is proposed for mid-planning period to reduce the cost of treatment at that plant.
- A process has been developed to update the water quality goals that govern treatment plant operations. Updating the goals will help to maintain compliance with regulatory requirements.
- \$250 million is budgeted address new water quality regulations, which are not required now, but could potentially be required in the future

Provides a new strategy for water main renewal in Detroit

- DWSD has traditionally used open-cut construction and ductile iron pipe. Looking forward, the variety in development type and density within the City of Detroit provides opportunity for a new mix of technologies currently used within Detroit's suburbs and peer cities, with a goal of reduce the cost of water mains from \$300 per foot to less than \$200 per foot.
- A 3-year pilot program is proposed to develop design criteria, construction standards, and application criteria for the new mix of technologies. The benefit of new approach is to reduce the potential long term cost of distribution system renewal from \$5 billion over the next 100 years to \$3 billion or less.

Proactive asset management approach to CIP planning

- The 20-year capital improvement program includes \$35 million for condition assessment studies of transmission mains, treatment plants and booster pump stations.
- Accurate and current condition assessment data, combined with computerized maintenance management and asset management systems, can reduce the future costs of infrastructure replacement by providing alternatives for rehabilitation or smaller scale replacement of weak links in the infrastructure.

Reducing nonrevenue water to improve system efficiency

- The Master Plan Update sets a goal of reducing nonrevenue water from 30 percent today to 15 percent by 2035.
- New production metering projects underway, and DWSD is piloting District Metering Area approach to manage leakage within geographic limits.
- New wholesale meter are proposed for Detroit, Dearborn and Highland Park. Within Detroit, the goal is to complete wholesale metering for half of the City by 2035, and to perform this work in coordination with concurrent water distribution main renewal projects to minimize the cost.

1 Introduction

1.1 Background

The Detroit Water and Sewerage Department (DWSD) and its predecessor agencies have provided public water supply since 1836. Master planning has been used by the department for over 100 years as a way to establish direction for growth and facilities in 20 to 50 year periods.

The last comprehensive master plan was completed in 2004. The economic recession of 2007 created significant changes to the projections used in the 2004 master plan. Also, starting in 2003, the wholesale water customers of DWSD became more involved in analysis and planning for the system through a customer outreach initiative.

Consequently, DWSD and its wholesale customers agreed in 2012 to prepare an update to the previous master plan. DWSD also established a formal retail customer steering committee, to complement the wholesale customer involvement effort.

1.2 Goals of the Master Plan Update

The goals of the Master Plan Update are:

1. Perform the planning with extensive customer involvement from retail customer and wholesale customer steering committees and specialty working groups.
2. Examine a range of future scenarios and establish indicators to guide realistic projections of population and water demand, to plan capital facilities, and to respond to change.
3. Resolve legacy issues on requirements for plant capacity and transmission system improvements that have been pending for several years.
4. Focus on actions that result in cost efficiency, maximizing use of existing assets, and improving reliability, and long term financial sustainability.

1.3 Interim Reports Maximized Customer Review and Involvement

The Water Master Plan Update was a 24-month project, extending from June 2013 to June 2015. In order to provide maximum opportunity for customer and stakeholder input, results of the Master Plan Update were presented in a series of six interim reports:

- **Customer Involvement Plan Interim Report.** This interim report was prepared in July 2013, and it established the framework and procedures for wholesale customer and retail customer involvement activities in the planning study.
- **Priorities for First Five Years Interim Report.** This was prepared in March 2014, and it outlined priorities for the first 5 years of the planning period.

- **Market Plan Interim Report.** This interim report was prepared in September 2014, and it established a forecast for the service area, population served, and water use projections to 2035.
- **Source Water and Water Quality Plan Interim Report.** This interim report was prepared in December 2014, and it examined watershed management and source water protection requirements. It also addressed water quality monitoring and compliance with future regulations.
- **Infrastructure and Service Management Plan Interim Report.** This interim report was prepared in March 2015, and it analyzed requirements for pumping, storage, transmission, and distribution facilities to meet level of service goals for retail and wholesale customers.
- **Financial Analysis.** This analysis was prepared in April 2015, presented the proposed 20-year capital improvement program (CIP) and forecasts of water sales for multiple scenarios. The 20-year capital improvement plan and water sales forecasts were incorporated into concurrent financial analyses performed for transition to the new Great Lakes Water Authority. (See Chapter 10.)

Each of the interim reports were accompanied by workshops and meetings to receive comments and examine particular issues in depth. The final report was prepared cumulatively through building blocks of each interim report, followed by continuing review and refinement of planning objectives, technical issues, and recommendations over the 24-month period.

1.4 Format and Distribution of this Report

This report has been prepared and organized for general distribution as a PDF file to representatives of DWSD, the Master Plan Steering Team, Retail Customer Steering Committee, and wholesale customers. The report and previous interim reports were distributed through the DWSD Oureach.org internet portal and it is readable with Adobe Reader Version XI or earlier versions. Printed copies of the final report and associated digital project deliverables were provided to DWSD.

The technical appendices for this report are presented in a series of technical memoranda. These are referenced in the report by the abbreviation TM-2 for Technical Memorandum 2, for example.

The content of the interim reports noted in Section 1.3 has been updated for this final report. Headings at the top of Chapters 3 to 11 indicate the original interim report from which the chapter content is derived.

2 Planning Criteria

This chapter presents the basic planning criteria for the Water Master Plan Update. Additional information is presented in TM-2 and TM-4, which provide detail on population projections, level of service goals, redundancy, reliability, cost estimates, and life cycle cost evaluations.

2.1 Planning Period

DWSD's goal is to provide an update to its earlier 50-year Comprehensive Master Plan, which covered the years 2000 to 2050. This Master Plan Update covers a shorter planning period of 20 years.

In order to synchronize with DWSD's annual Capital Improvement Program, the planning period for this Master Plan Update has been established as July 1, 2015, to June 30, 2034.

2.2 Planning Area

Figure 2-1 on the follow page shows the current service area in May 2015. Characteristics of the service area are described in Chapter 3. The planning area is primarily the current water service. Areas not served by public water supply that adjoin the current service area, and other adjoining water service providers were considered on a limited basis as potential future wholesale customers. Potential new wholesale customers are discussed in Chapter 4.

2.3 Level of Service Goals

Level of service goals were established to guide decision-making on new infrastructure and rehabilitation of existing infrastructure. The goals cover the areas of redundancy, reliability, customer service pressure and volume, hydraulics of the transmission system, storage capacity, and water quality. TM-4 presents the level of service goals.

2.4 Population Projections

Population projections for the DWSD service area were developed based on regional forecasts made by Southeast Michigan Council of Governments, Genesee County Metropolitan Planning Commission and a private forecaster, Woods & Poole Economic, Inc.

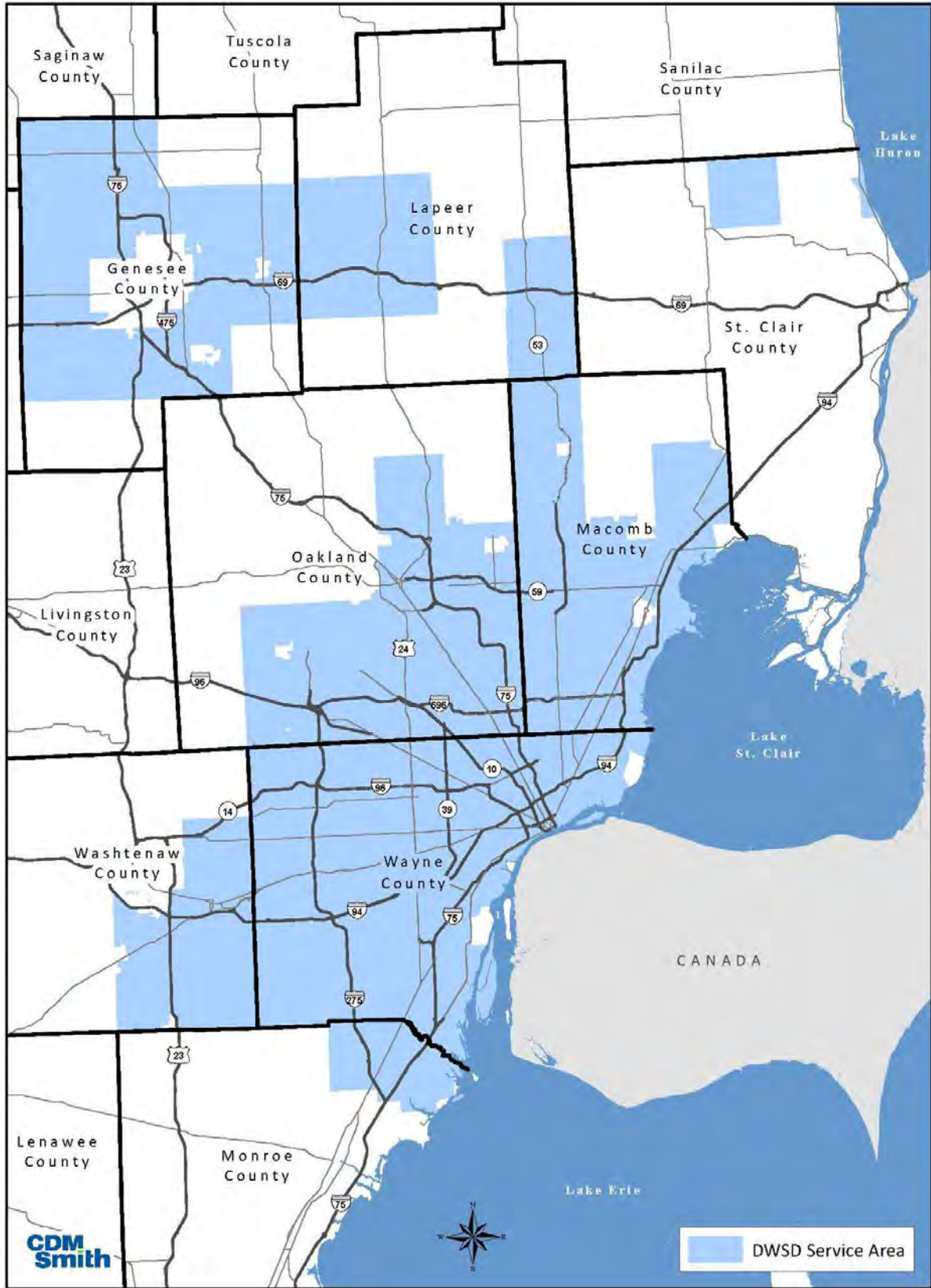


Figure 2-1: DWSO Service Area as of May 2015

During the period December 2013 to March 2014, the master planning team sent customized data packages to all wholesale customers. These data packages included recent trends in building permits, population and employment past trends and projections, identification of major industrial water users, and maps of major water facilities and service area. The objective of these data packages was to allow each wholesale customer the opportunity to review and amend projections for this Master Plan Update.

The wholesale customer projections are generally consistent with the regional population and employment projections. The wholesale customer projections also identify the current and projected residential population and the geographic areas to be served by DWSD water, which adds precision to the estimates of future water demand.

Tables 2-1 and 2-2 show 20-year projections for population growth. See Appendix TM-2 for more detail on these projections. In these tables, the term “non-customers” refers to residents and workers who are served by private wells or other water service providers.

Table 2-1: Regional Population Projections Showing DWSD Water Customers

County	2010	2015	2020	2025	2030	2035
Genesee Co. Total	425,100	421,531	421,711	422,231	422,645	422,895
Genesee Co. Non-customers	113,100	211,531	421,711	422,231	422,645	422,895
Genesee Co. DWSD Customers ¹	312,000	210,000	0	0	0	0
Lapeer Co. Total	88,189	91,275	95,474	99,784	104,107	108,423
Lapeer Co. Non-customers	67,189	69,575	72,774	75,984	79,307	82,623
Lapeer Co. DWSD Customers ²	21,000	21,700	22,700	23,800	24,800	25,800
Macomb Co. Total	840,978	855,378	863,380	872,733	884,846	896,401
Macomb Co. Non-customers	51,333	52,218	52,835	52,819	52,675	52,661
Macomb Co. DWSD Customers	789,645	803,160	810,545	819,914	832,171	843,740
Monroe Co. Total	152,021	155,696	156,602	158,347	160,865	163,246
Monroe Co. Non-customers	135,357	138,218	138,602	139,830	141,971	144,175
Monroe Co. DWSD Customers	16,664	17,478	18,000	18,517	18,894	19,071
Oakland Co. Total	1,202,362	1,215,322	1,218,432	1,221,340	1,230,734	1,232,649
Oakland Co. Non-customers	311,271	319,325	319,031	319,111	321,989	320,377
Oakland Co. DWSD Customers	891,091	895,997	899,401	902,229	908,745	912,272
St. Clair Co. Total	163,040	161,667	161,497	162,541	164,643	166,652
St. Clair Co. Non-customers	157,494	156,001	155,829	156,543	158,537	160,427
St. Clair Co. DWSD Customers	5,546	5,666	5,668	5,998	6,106	6,225
Washtenaw Co. Total	344,791	350,784	354,116	360,366	368,297	377,220
Washtenaw Co. Non-customers	208,858	213,237	213,772	217,751	221,219	225,103
Washtenaw Co. DWSD Customers	135,933	137,547	140,344	142,615	147,078	152,117
Wayne Co. Total (outside Detroit)	1,106,807	1,093,946	1,076,145	1,063,050	1,054,944	1,047,933
Wayne Co. Non-customers	52,559	49,622	48,183	47,072	46,394	45,966
Wayne Co. DWSD Customers (outside Detroit)	1,054,248	1,044,324	1,027,962	1,015,978	1,008,550	1,001,967
City of Detroit	713,777	648,350	624,705	612,442	609,745	613,709
Regional Total	5,037,065	4,993,949	4,972,062	4,972,834	5,000,826	5,029,128
Regional Non-customers	1,097,161	1,209,727	1,422,737	1,431,341	1,444,737	1,454,227
Regional DWSD Customers	3,939,904	3,784,222	3,549,325	3,541,493	3,556,089	3,574,901
Regional DWSD Customers excluding Genesee County in 2010 and 2015	3,627,904	3,574,222	3,549,325	3,541,493	3,556,089	3,574,901

¹Genesee Co. DWSD Customers includes 2010 Census Population for the City of Flint and surrounding Genesee County customers. 2015 DWSD customers include only the estimate of residents in areas served through Genesee County Drain Commission (see TM-15 for population connected to water service). After 2017 Genesee Co. has indicated they will no longer be a DWSD customer - entire Woods & Poole population projection for Genesee Co. shown as Non-Customer 2020-2035.

²Lapeer Co. DWSD Customer population estimated based on 2010 Census data (locations with >1 person/acre) and Woods & Poole County-wide population projection trends.

Table 2-2: Regional Employment Projections Showing DWSD Water Customers

County	2010	2015	2020	2025	2030	2035
Genesee Co. Total	184,015	194,140	202,408	211,192	220,474	230,236
Genesee Co. Non-customers	48,915	97,440	202,408	211,192	220,474	230,236
Genesee Co. DWSD Customers	135,100	96,700	0	0	0	0
Lapeer Co. Total	31,197	32,869	34,256	35,647	37,047	38,449
Lapeer Co. Non-customers	23,797	25,069	26,156	27,147	28,247	29,349
Lapeer Co. DWSD Customers	7,400	7,800	8,100	8,500	8,800	9,100
Macomb Co. Total	362,517	377,116	379,981	387,217	395,239	403,398
Macomb Co. Non-customers	29,646	30,745	31,271	31,953	32,688	33,478
Macomb Co. DWSD Customers	332,871	346,371	348,710	355,264	362,551	369,920
Monroe Co. Total	53,761	56,262	56,928	57,637	58,669	60,081
Monroe Co. Non-customers	49,003	51,267	51,850	52,491	53,422	54,722
Monroe Co. DWSD Customers	4,758	4,995	5,078	5,146	5,247	5,359
Oakland Co. Total	842,222	901,219	921,533	936,923	951,622	964,459
Oakland Co. Non-customers	86,581	92,693	94,357	96,118	97,682	99,092
Oakland Co. DWSD Customers	755,641	808,526	827,176	840,805	853,940	865,367
St. Clair Co. Total	62,614	65,243	66,061	67,045	68,350	70,038
St. Clair Co. Non-customers	61,823	64,432	65,231	66,200	67,498	69,165
St. Clair Co. DWSD Customers	791	811	830	845	852	873
Washtenaw Co. Total	236,676	246,721	252,598	260,024	268,528	277,576
Washtenaw Co. Non-customers	177,029	184,613	188,972	194,649	201,208	208,060
Washtenaw Co. DWSD Customers	59,647	62,108	63,626	65,375	67,320	69,516
Wayne Co. Total (outside Detroit)	509,859	522,832	527,342	531,131	535,371	540,431
Wayne Co. Non-customers	25,123	25,450	25,735	26,004	26,341	26,635
Wayne Co. DWSD Customers (outside Detroit)	484,736	497,382	501,607	505,127	509,030	513,796
City of Detroit	347,545	357,247	353,242	352,394	352,670	354,075
Regional Total	2,630,406	2,753,649	2,794,349	2,839,210	2,887,970	2,938,743
Regional Non-customers	501,917	571,709	685,980	705,754	727,560	750,737
Regional DWSD Customers	2,128,489	2,181,940	2,108,369	2,133,456	2,160,410	2,188,006
Regional DWSD Customers excluding Genesee County in 2010 and 2015	1,993,389	2,085,240	2,108,369	2,133,456	2,160,410	2,188,006

The following statements can be made in summary of **Tables 2-1 and 2-2** and additional information in TM-2:

1. Employment growth within DWSD customer communities is expected to increase 9.7% over the planning period.
2. The number of residential households within DWSD customer communities is expected to increase by 3.7 percent. (See TM-2 for a tabulation of households.)

3. Population within DWSD customer communities (excluding Genesee County) is expected to decrease 1.6 percent relative to 2010.
4. Compared to the population in the service area in the year 2000, which was approximately 3.78 million, the 2035 projection represents a 6.1 percent decline.
5. Current residential population projections to the year 2035 are approximately 12 percent lower than the projections that were the basis for the previous 2004 Comprehensive Water Master Plan.

2.5 Basis of Cost Estimates

Cost estimates for the Water Master Plan Update are based on planning level investigations, and these cost estimates are intended for comparison of alternatives and general capital budgeting. Estimates in this report are expected to change as more detailed definition develops through facilities planning and design investigations. A cost contingency allowance is added to planning level estimates to allow for potential cost increase in later phases of implementation.

The following terms and values are used in the planning level cost estimates:

Construction Cost: Estimated construction contract cost for furnishing materials, equipment and labor, testing and warranties. Construction cost estimates are generally based on recent bids for similar construction. All construction costs are presented based on prevailing prices in metropolitan Detroit in December 2013. Where cost estimates prepared in previous years were used, these were escalated to December 2013 prevailing prices.

Unit Prices: Unitized construction costs expressed as “\$ per foot” for transmission mains or “\$ per gallon” for reservoirs. Unit prices provide a basis for comparing the relative cost of different projects.

Engineering Cost: The cost of engineering during design and construction. An allowance of 15 percent is used for the estimates in this report. If substantial geotechnical investigation or other special services are anticipated, additional allowances are added for these.

Administrative and Legal: The cost of the owner (DWSD) to procure, award, contract manage, administer and provide routine legal support for a project. An allowance of 5 percent has been used for the estimates in this report.

Land and Easements: The estimated cost of land and easement acquisition for a project. These are estimated on a case-by-case basis.

Contingency: An allowance for additional requirements that may be discovered in subsequent stages of engineering. An allowance of 15 percent is used for this report. This includes 10 percent for new requirements during the design phase and 5 percent for change to construction contract price based on unexpected field conditions or requested changes. This type of allowance is continually reduced after planning is completed, and design progresses.

Cost estimates have been prepared in accordance with standards of the Association for the Advancement of Cost Engineering International (AACE International). They are generally based on

Class 4, 10 percent conceptual or preliminary design. Construction estimates are expected to be accurate with a range of 50 percent higher to 30 percent lower, and are for use as budgetary values only.

The previous Needs Assessment reports for water treatment plants prepared in 2002 used different factors for engineering, legal, and administrative services and project contingencies. Also, the 2004 Comprehensive Water Master Plan presented its estimates of capital cost based on different factors. The factors in those previous efforts ranged from 26 to 56 percent total allocations, compared to the 35 percent used for this Water Master Plan Update. The current plan includes more investigation and review of previous project concepts and cost estimates with DWSD staff and wholesale customers.

Technical Memorandum TM- 4 provides detail on the basis of construction cost estimates. Construction cost estimates are based on recent available bid prices within the DWSD service area. The scope of work for this project included an update of costs from previous needs assessment reports prepared in 2002. In that case, current construction costs are estimated based on escalation according to the Engineering News Record Construction Cost Index.

2.5.1 Life Cycle Cost Evaluation

One of the major tasks for the previous Product Plan Interim Report and ongoing analysis of water treatment plant consolidation was an evaluation of the economics for water plant closures and repurposing. This evaluation required the consideration of capital costs, operating costs, and the staging of when construction would occur. There are several alternatives for water plant closures, and these alternatives have significantly different operating and capital costs. In order to compare all alternatives on a consistent economic basis, a life cycle cost evaluation was performed. The life cycle cost evaluation considers a time series of new capital, replacement, salvage, and annual operating costs over the 20-year planning period. All costs in the time series are then represented by one number, called the Present Worth, as of March 2014.

The alternative with the lowest Present Worth cost is the most cost-effective in consideration of expenditures and benefits. In developing the different alternatives, it is important that all meet the same threshold of level of service, reliability and redundancy.

Not all factors in the plant closure evaluation can be equated into annual costs and capital costs. There are non-monetary factors, such as potential future scenarios for regulations and growth, and certain risks that are best understood as additional decision criteria outside of the life cycle cost evaluation. These non-monetary factors are discussed in Chapter 6.

The life cycle cost evaluation was performed in accordance with the United States Office of Management and Budget Circular A-94.Revised, titled: "Guidelines and Discount Rates for Benefit-Cost Analysis of Federal Programs." These guidelines are generally used in programs when federal funding is provided for water and transportation projects.

In the context of the OMB Circular A-94 guidelines, the discount rate is an important economic factor.

This factor is used to translate future expenditures and benefits over time to the single Present Worth value described above. In order to compute Present Worth, it is necessary to discount future benefits and costs. The OMB guidance on the use of discount rates is presented below:

“This discounting reflects the time value of money. Benefits and costs are worth more if they are experienced sooner. All future benefits and costs, including non-monetized benefits and costs, should be discounted. The higher the discount rate, the lower is the present value of future cash flows. For typical investments, with costs concentrated in early periods and benefits following in later periods, raising the discount rate tends to reduce the net present value.”

In the context of the water plant closure evaluation, the alternatives are structured so that benefits are consistent for all alternatives, as measured by the level of service goal. The largest uncertainty for DWSD and its customers is how costs could rise in the future, in order to achieve the level of service benefits. Future costs include construction, financing, and energy, chemical, labor and benefits.

In order to address the uncertainty around future costs, the life cycle evaluation was performed twice, once with a lower discount rate, then again with a higher discount rate. The low discount rate was 4 percent per year, and the high rate was 7 percent per year.

This range of rates is typical of the range of values currently used in the United States for cost-benefit studies performed in accordance with Circular A-94. The higher discount rate reflects a scenario of costs increasing at a higher rate than has been the case over the last 5 years. The lower discount rate reflects a scenario of costs increasing at approximately the same rate as over the last 5 years.

3 Water Demand Projections

3.1 General

Water demand projections for the twenty-year planning period were prepared based on a survey of wholesale customers and water use characterization within the City of Detroit and representative wholesale customer communities. The approach to this task included collaborative work with wholesale customers. All projections were reconciled with independent information from two sources of regional population and employment projections.

Water demand, for the purpose of master planning, includes water sales, fire flow requirements, and non-revenue water. Water sales include wholesale customer sales and retail customer sales.

- Water consumption (i.e. sales) by domestic, industrial, commercial and institutional users follows diurnal and annual patterns. Annual patterns have the highest demand in the summer months, and the maximum day and peak hour water demands are typically driven by outdoor irrigation. Climate change could have an impact—up or down—on outdoor irrigation demands.
- Fire flow requirements for wholesale customers are accounted for in the volumes and pressures provided in their service contracts. Fire flows for the City of Detroit are based on requirements established by the ISO (Insurance Service Office) based on land use and type and density of structures. In large systems, such as the DWSD system, fire flows primarily impact design of localized distribution and transmission facilities. Changes in firefighting practices and technology are expected to reduce this water demand in the future.
- Non-revenue water includes unmetered public use, water loss through leaks and water main breaks, and metering and accounting losses.

Table 3-1 presents a summary of the water demand projections for the planning period. The remaining sections of this chapter describe recent trends, and how the projections were developed. TM-5 presents supporting details.

Table 3-1: Projected Water Demands for 2015 and 2035

Communities Served	2015 Demand (MGD)		2035 Demand (MGD)	
	Average Day	Maximum Day	Average Day	Maximum Day
ALLEN PARK	3.46	7.35	3.41	7.24
ASH TOWNSHIP	0.87	1.30	0.91	1.36
AUBURN HILLS	4.20	9.99	4.37	10.39
BELLEVILLE	0.36	0.52	0.35	0.51
BERLIN TWP	0.59	1.16	0.78	1.54
BROWNSTOWN TWP	2.87	6.20	3.00	6.47
BRUCE TWP	0.03	0.11	0.31	0.73
CANTON TWP	8.09	19.05	8.52	20.07
CENTER LINE	0.73	1.17	0.75	1.19

Table 3-1: Projected Water Demands for 2015 and 2035

Communities Served	2015 Demand (MGD)		2035 Demand (MGD)	
	Average Day	Maximum Day	Average Day	Maximum Day
CHESTERFIELD TWP	3.97	8.42	4.55	9.64
CLINTON TWP	9.58	19.69	10.05	20.66
COMMERCE TWP	2.27	6.00	3.56	9.38
DEARBORN	14.02	25.66	15.52	28.40
DEARBORN HTS	4.69	8.36	4.70	8.37
DETROIT	179.29	328.10	166.27	304.27
EASTPOINTE	2.43	3.94	2.69	4.36
ECORSE	2.92	3.62	2.95	3.66
FARMINGTON	1.15	2.19	1.26	2.40
FARMINGTON HILLS	9.29	22.56	9.82	23.86
FERNDALE	1.47	2.85	1.53	2.97
FLAT ROCK	1.33	2.42	1.32	2.41
FLINT (Genesee County only)	12.00	12.00	Emergency only	Emergency only
FRASER	1.51	3.07	1.53	3.10
GARDEN CITY	1.74	3.24	1.75	3.27
GIBRALTAR	0.37	4.04	0.37	4.05
GLCUA	2.26	3.69	2.67	4.37
GROSSE ILE TWP	1.02	2.02	0.97	1.93
GROSSE POINTE PARK	1.34	3.04	1.34	3.03
GROSSE POINTE SHORES	0.36	1.30	0.33	1.18
GROSSE POINTE WOODS	1.85	5.53	1.73	5.19
HAMTRAMCK	1.37	1.73	1.25	1.58
HARPER WOODS	1.28	2.28	1.22	2.18
HARRISON TWP	2.03	4.06	2.19	4.39
HAZEL PARK	1.01	1.03	1.04	1.06
HIGHLAND PARK	TBD	TBD	TBD	TBD
HURON TWP	1.30	3.72	1.22	3.50
INKSTER	2.23	3.43	2.06	3.17
KEEGO HARBOR	0.24	0.47	0.25	0.49
LENOX TWP	0.31	0.54	2.76	4.78
LINCOLN PARK	3.22	5.84	3.07	5.55
LIVONIA	12.40	29.66	12.36	29.56
MACOMB TWP	7.66	22.08	8.48	24.45
MADISON HTS	3.30	5.45	3.42	5.65
MELVINDALE	0.97	1.98	0.96	1.96
NEW HAVEN	0.33	0.65	0.37	0.73
NORTHVILLE	0.69	1.49	0.69	1.47
NORTHVILLE TWP	3.58	9.11	3.66	9.33

Table 3-1: Projected Water Demands for 2015 and 2035

Communities Served	2015 Demand (MGD)		2035 Demand (MGD)	
	Average Day	Maximum Day	Average Day	Maximum Day
NOVI	6.72	15.28	7.28	16.56
OAK PARK	2.31	5.07	2.24	4.91
OAKLAND COUNTY	0.16	3.44	0.16	3.44
ORION TWP	3.01	7.57	6.00	15.07
PLYMOUTH	0.96	1.74	1.00	1.82
PLYMOUTH TWP	3.92	9.87	4.01	10.10
PONTIAC	6.72	11.70	6.84	11.92
REDFORD TWP	3.97	6.63	3.73	6.23
RIVER ROUGE	1.16	1.76	1.06	1.60
RIVERVIEW	1.11	2.06	1.07	1.99
ROCHESTER HILLS	7.97	18.84	8.94	21.12
ROCKWOOD	0.26	0.53	0.26	0.52
ROMEO	0.21	0.45	0.22	0.47
ROMULUS	4.72	8.38	4.81	8.54
ROSEVILLE	4.23	6.51	4.23	6.51
ROYAL OAK TWP	0.26	0.53	0.28	0.57
Saint Clair Shores	5.12	9.70	5.36	10.16
SHELBY TWP	9.33	21.31	9.69	22.12
SOCWA	30.59	65.87	32.20	69.35
SOUTH ROCKWOOD	0.12	0.20	0.13	0.22
SOUTHGATE	2.68	4.82	2.68	4.81
ST CLAIR COUNTY	0.40	2.92	0.40	2.92
Saint Clair Co. DPW (Burchville TWP)	0.16	0.48	0.18	0.55
STERLING HTS	14.22	35.70	14.84	37.27
SUMPTER TWP	0.77	1.14	0.79	1.18
SYLVAN LAKE	0.16	0.36	0.16	0.38
TAYLOR	6.51	15.20	6.25	14.61
TRENTON	1.86	2.99	1.87	3.01
TROY	10.86	26.66	11.20	27.49
UTICA	0.40	1.23	0.41	1.28
VAN BUREN TWP	3.13	7.32	3.10	7.26
W BLOOMFIELD TWP	6.18	15.08	6.50	15.86
WALLED LAKE	0.72	1.38	0.74	1.42
WARREN	15.68	30.28	16.17	31.24
WASHINGTON TWP	1.59	4.64	2.42	7.07
WAYNE	2.24	7.49	2.24	7.48
WESTLAND	7.17	12.92	6.74	12.15
WIXOM	1.74	4.52	1.89	4.90

Table 3-1: Projected Water Demands for 2015 and 2035

Communities Served	2015 Demand (MGD)		2035 Demand (MGD)	
	Average Day	Maximum Day	Average Day	Maximum Day
WOODHAVEN	1.58	3.53	1.54	3.43
YCUA	10.85	20.99	10.89	21.06
Total	500	1,009	493	1,014

3.2 Water Sales and Production Trends

Figure 3-1 shows the trend in water sales from 2000 to 2014. Over the last 15 years, water sales have declined 27 percent from an average of 550 MGD in the year 2000 to 400 MGD in 2014. In FY2013, City of Detroit water sales were 75 MGD, and wholesale customer sales were 321 MGD. In contrast, during the year FY2000, City of Detroit water sales were 125 MGD and wholesale customer sales were 410 MGD. The reduction is a combination of national trends in water consumption and regional economic change.

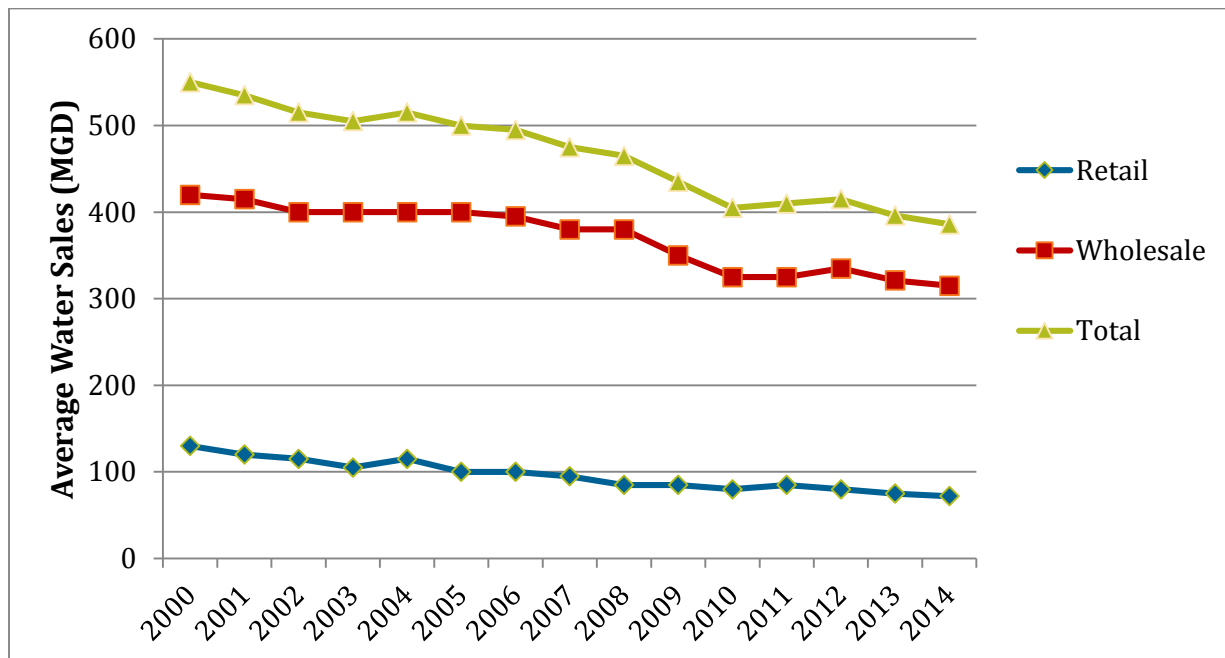


Figure 3-1: Water Sales Trend

During the same period, nonrevenue water has increased to an annualized average of 165 MGD in FY 2014. Nonrevenue water is discussed in more detail in Section 3.8. **Figure 3-2** shows the principle components of DWSD water production for FY2013.

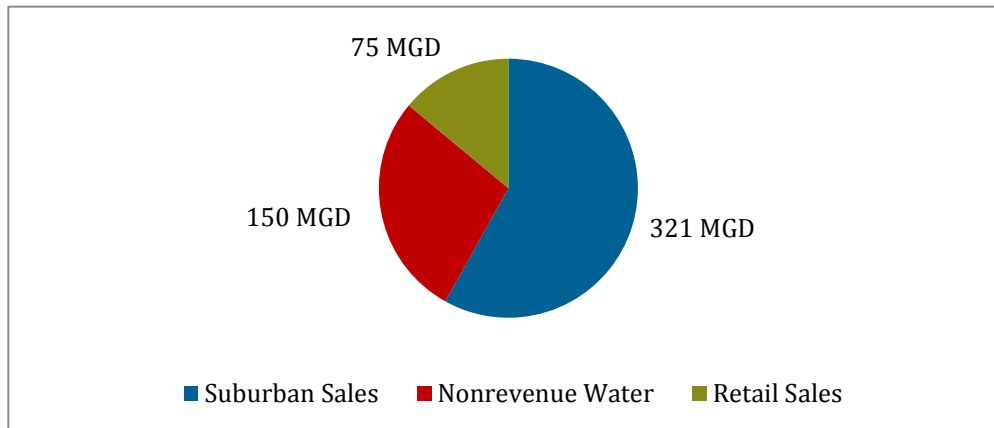


Figure 3-2: Current Use of Total Water Production FY2013

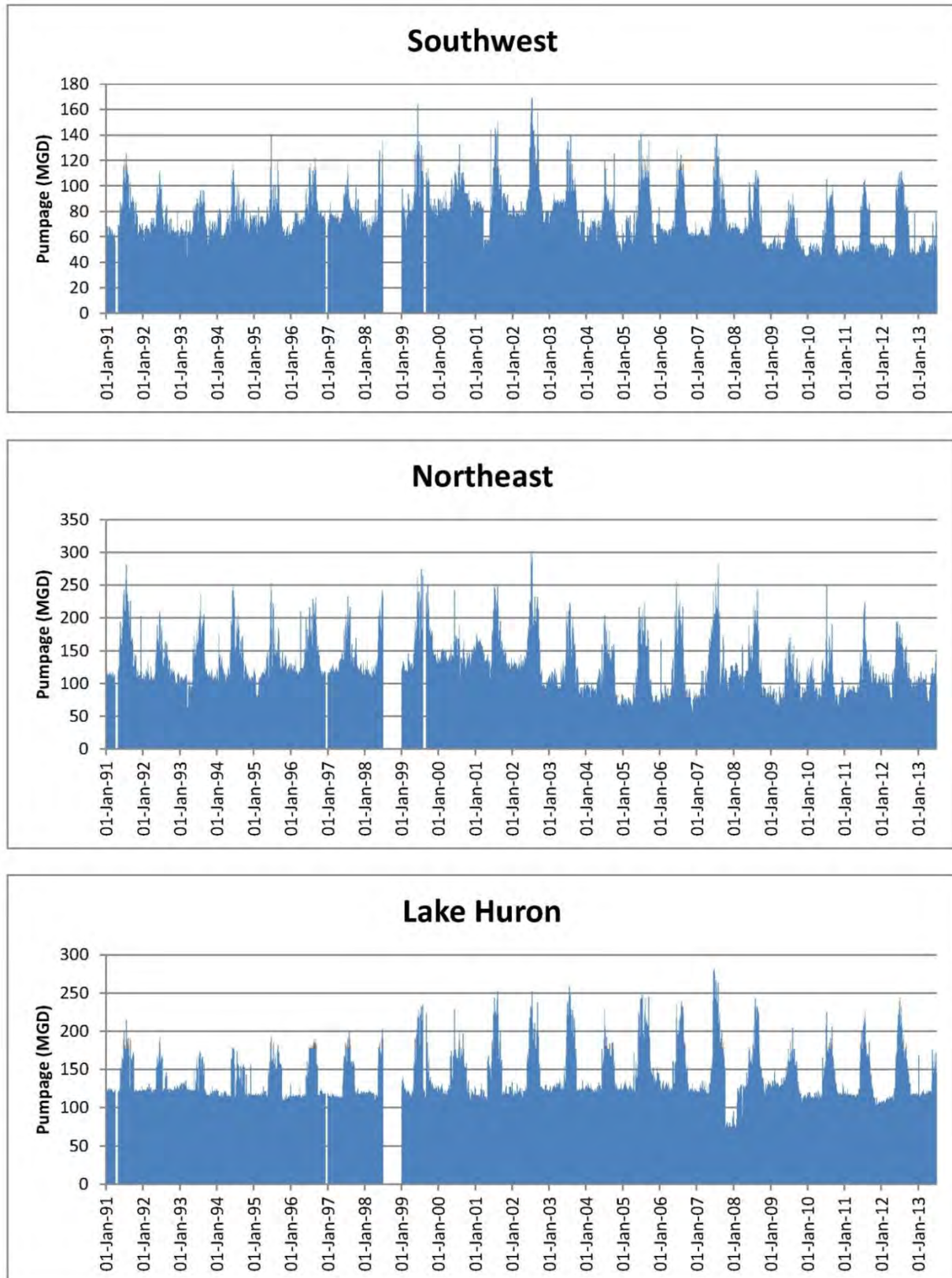
Figures 3-3 and 3-4 show the past ten years of water production trends. These figures show daily water production at each water treatment plant, along with total production. Average production exceeded 600 MGD prior to 2004, and is now approximately 550 MGD. There are some time periods when production numbers were missing from the available data.

These charts show the trend in water production from each plant. The five plants operate at an average of 30 percent rated capacity in non-peak season, and recent maximum day production has been 53 percent of rated capacity, as shown in **Table 3-2**.

Table 3-2: Recent Plant Production Relative to Rated Capacity 2003 to 2013

Plant	Rated Capacity MGD	2003 Average Production	2013 Average Production	2013 Maximum Day	2013 Average Percent Loading	2013 Maximum Percent Loading
Southwest	240	75	55	100	23	42
Northeast	300	130	105	200	35	67
Lake Huron	400	125	110	225	28	56
Water Works Park	240	100	80*	90*	33	38
Springwells	540	200	175	300	32	56
Total	1,720	630	525	915	30	53

*Water production at Water Works Park is currently limited by yard piping restrictions. DWSD has designed the required improvements, and this project is in the current CIP for construction.



*Data were not available for July and December 1998.

Figure 3-3: 10-Year Pumping Record for the Northeast, Southwest and Lake Huron WTPs

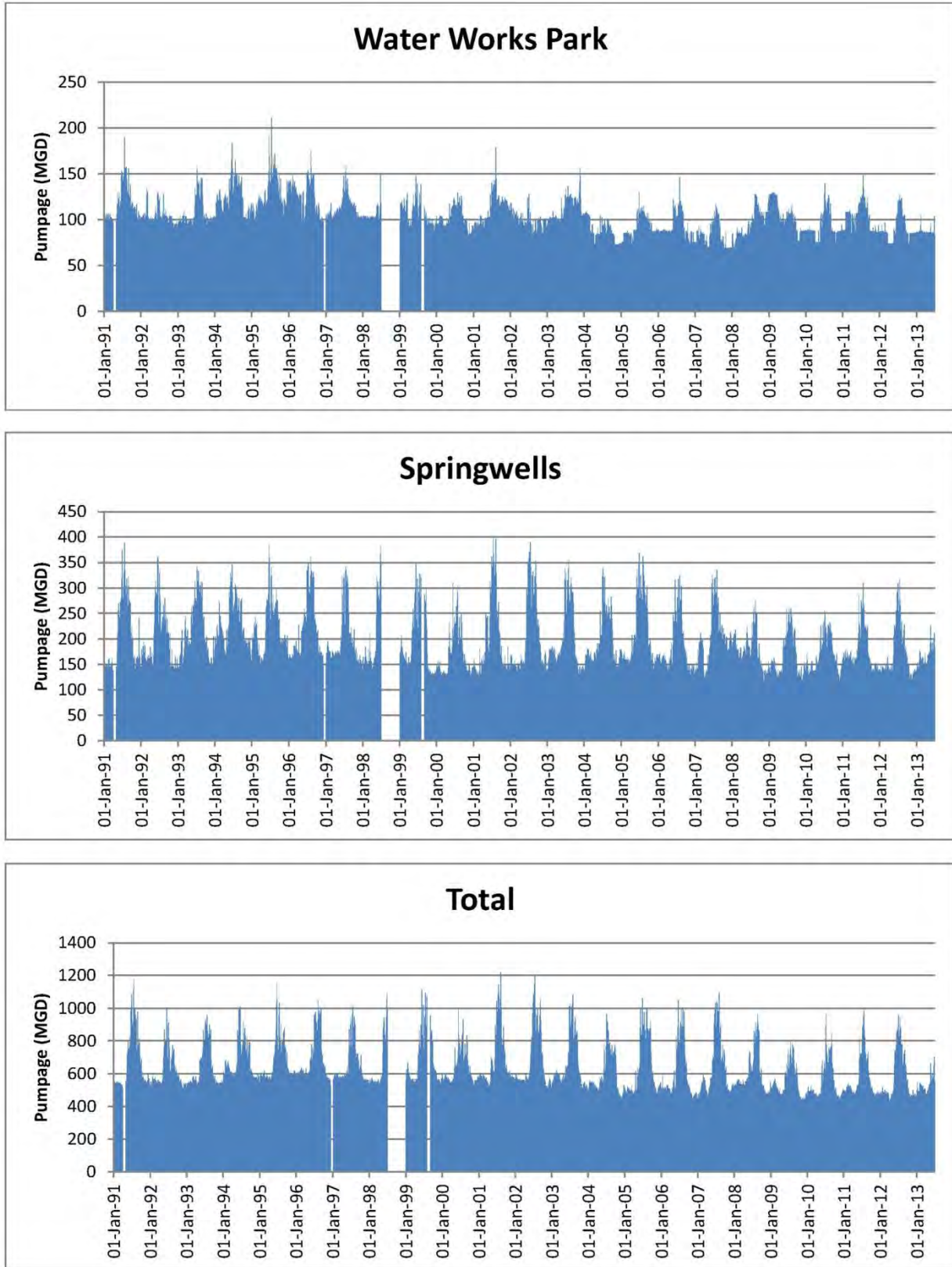


Figure 3-4: 10-Year Pumping Record for the Water Works Park and Springwells WTPS, and Total 10 Year Pumping Record

3.3 Contract Status of Wholesale Customers

DWSD provides wholesale water supply to 85 suburban customers and retail service for the City of Detroit. DWSD is providing interim retail water service to the City of Highland Park under an agreement with the State of Michigan.

Table 3-3 presents a list of DWSD's wholesale customers, the local units of government that they serve, and the status of the customer water supply contracts. 76 of DWSD's wholesale customers have new 30-year water service contracts that extend from 2038 to 2045. Genesee County is not a direct customer of DWSD, but has been supplied through a former service contract with the City of Flint. Genesee County has announced its intent to terminate continuous water purchases in July 2016, but then remain as an emergency supply customer. Other customers have agreements pending renewal or that extend to 2021.

Table 3-3: Current Status of Wholesale and Retail Customer Water Service Contracts

Customer	Local Government Jurisdictions Served	DWSD Contract Status
ALLEN PARK	City of Allen Park	Under Contract to September 8, 2038
ALMONT	City of Almont	Under Contract to June 30, 2045
ASH TOWNSHIP	Ash Township, Village of Carleton	Under Contract to April 27, 2039
BELLEVILLE	City of Belleville	Under Contract to September 22, 2038
BERLIN TWP	Berlin Township	Under Contract to March 16, 2039
BROWNSTOWN TWP	Brownstown Township	Under Contract to March 16, 2039
BRUCE TWP	Bruce Township	Under Contract to March 22, 2040
ST. CLAIR CO. DPW (BURTCHVILLE TWP)	Burtchville Twp	Under Contract to July 26, 2040
CANTON TWP	Canton Township	Under Contract to May 19, 2038
CENTER LINE	City of Center Line	Under Contract to Oct 13, 2038
CHESTERFIELD TWP	Chesterfield Township	Under Contract to January 20, 2044
CLINTON TWP	Clinton Township	Under Contract to February 23, 2039
COMMERCE TWP	Commerce Township	Under Contract to July 26, 2040

Table 3-3: Current Status of Wholesale and Retail Customer Water Service Contracts

Customer	Local Government Jurisdictions Served	DWSD Contract Status
DEARBORN	City of Dearborn	<i>Indefinite Contract with one year notice required for termination</i>
DEARBORN HTS	City of Dearborn Heights	Under Contract to September 8, 2038
DETROIT	City of Detroit	Retail Service
EASTPOINTE	City of Eastpointe	Under Contract to September 29, 2038
ECORSE	City of Ecorse	Under Contract to July 26, 2040
FARMINGTON	City of Farmington	Under Contract to June 22, 2039
FARMINGTON HILLS	City of Farmington Hills	Under Contract to September 14, 2039
FERNDALE	City of Ferndale	Under Contract to May 19, 2038
FLAT ROCK	City of Flat Rock	Under Contract to May 3, 2040
FLINT	City of Flint	<i>Contract Terminated Effective April 17, 2014</i>
FRASER	City of Fraser	Under Contract to January 12, 2039
GARDEN CITY	City of Garden City	Under Contract to March 16, 2039
GENESEE CO.	Burton, Clayton Twp, Clio, Davison Twp, Flint Twp, Flushing, Flushing Twp, Gaines Twp, Genesee Twp, Grand Blanc Twp, Montrose, Montrose Twp, Mt. Morris, Mt. Morris Twp, Mundy Twp, Richfield Twp, Swartz Creek, Vienna Twp	<i>No DWSD Contract-City of Flint Customer. Genesee County sent Notice to DWSD in July 2014 to Terminate Water Service in July 2016</i>
GIBRALTAR	City of Gibraltar	<i>Under Contract to 2021; one year notice required for termination</i>
GLCUA	<i>Imlay City, Imlay Twp, City of Lapeer, Mayfield Twp</i>	<i>Contracted Terminated Effective July 15, 2015</i>
GROSSE ILE TWP	Grosse Ile Township	<i>Under Contract to 2015; one year notice required for termination</i>
GROSSE PTE PK	City of Grosse Pointe Park	<i>Indefinite Contract with one year notice required for termination</i>
GROSSE PTE SHRS	City of Grosse Pointe Shores	Under Contract to September 20, 2040
GROSSE PTE WDS	City of Grosse Pointe Woods	Under Contract to July 20, 2039

Table 3-3: Current Status of Wholesale and Retail Customer Water Service Contracts

Customer	Local Government Jurisdictions Served	DWSD Contract Status
HAMTRAMCK	City of Hamtramck	Under Contract to June 14, 2040
HARPER WOODS	City of Harper Woods	Under Contract to May 17, 2040
HARRISON TWP	Harrison Township	Under Contract to July 26, 2040
HAZEL PARK	City of Hazel Park	Under Contract to May 3, 2040
HIGHLAND PARK	City of Highland Park	Service Being Provided through an Agreement with the State of Michigan
HURON TWP	Huron Township	Under Contract to May 18, 2039
INKSTER	City of Inkster	Under Contract to 2021
KEEGO HARBOR	City of Keego Harbor	Under Contract to September 14, 2039
LENOX TWP	Lenox Township	Under Contract to September 14, 2039
LINCOLN PARK	City of Lincoln Park	Under Contract to January 20, 2044
LIVONIA	City of Livonia	Under Contract to July 20, 2039
MACOMB TWP	Macomb Township	Under Contract to July 20, 2039
MADISON HTS	City of Madison Heights	Under Contract to September 14, 2039
MELVINDALE	City of Melvindale	Under Contract to July 20, 2039
NEW HAVEN	Village of New Haven	Under Contract to May 3, 2040
NOCWA	Auburn Hills, Orion Twp., Pontiac, Rochester Hills	Under Contract to February 4, 2045
NORTHVILLE	City of Northville	Under Contract to June 23, 2038
NORTHVILLE TWP	Northville Township	Under Contract to June 23, 2038
NOVI	City of Novi	Under Contract to July 20, 2039
OAK PARK	City of Oak Park	Under Contract to March 16, 2039
OAKLAND COUNTY (Kuhn District)	Kuhn Drain Facility	Under Contract to November 19, 2039

Table 3-3: Current Status of Wholesale and Retail Customer Water Service Contracts

Customer	Local Government Jurisdictions Served	DWSD Contract Status
PLYMOUTH	City of Plymouth	Under Contract to October 13, 2038
PLYMOUTH TWP	Plymouth Township, Salem Township	Under Contract to May 18, 2039
REDFORD TWP	Redford Township	Under Contract to August 7, 2038
RIVER ROUGE	City of River Rouge	Under Contract to July 26, 2040
RIVERVIEW	City of Riverview	<i>Contract expired 2014; one year notice required for termination</i>
ROCKWOOD	City of Rockwood	Under Contract to January 11, 2040
ROMEO	Romeo Industrial District	Under Contract to July 16, 2042
ROMULUS	City of Romulus	Under Contract to April 27, 2039
ROSEVILLE	City of Roseville	Under Contract to October 13, 2038
ROYAL OAK TWP	Royal Oak Township	Under Contract to May 17, 2040
SHELBY TWP	Shelby Township, Portion of City of Rochester through SY-08/RH-01	Under Contract to May 17, 2040
SOCWA	Berkley, Beverly Hills, Bingham Farms, Birmingham, Bloomfield Hills, Bloomfield Twp, Clawson, Huntington Woods, Lathrup Village, Pleasant Ridge, Royal Oak, Southfield, Southfield Twp, Village of Franklin	Under Contract to September 14, 2039
SOUTH ROCKWOOD	Village of South Rockwood	Under Contract to February 23, 2039
SOUTHGATE	City of Southgate	Under Contract to March 16, 2039
ST CLAIR COUNTY / Greenwood DTE supply	St. Clair County Board of Public Works	<i>Contract expired 2009; one year notice required for termination</i>
ST CLAIR SHORES	City of St Clair Shores	Under Contract to September 29, 2038
STERLING HTS	City of Sterling Heights	Under Contract to July 21, 2038
SUMPTER TWP	Sumpter Township	Under Contract to July 26, 2040

Table 3-3: Current Status of Wholesale and Retail Customer Water Service Contracts

Customer	Local Government Jurisdictions Served	DWSD Contract Status
SYLVAN LAKE	City of Sylvan Lake	Under Contract to September 20, 2040
TAYLOR	City of Taylor	Under Contract to September 8, 2038
TRENTON	City of Trenton	<i>Contract Expires 2020; one year notice required for termination</i>
TROY	City of Troy	Under Contract to June 23, 2038
UTICA	City of Utica	Under Contract to January 26, 2039
VAN BUREN TWP	Van Buren Township	Under Contract to June 23, 2038
W BLOOMFIELD TWP	West Bloomfield Township, Orchard Lake	Under Contract to June 23, 2038
WALLED LAKE	Walled Lake	Under Contract to October 5, 2039
WARREN	City of Warren	Under Contract to February 21, 2041
WASHINGTON TWP	Washington Township	Under Contract to June 23, 2038
WAYNE	City of Wayne	Under Contract to October 13, 2038
WESTLAND	City of Westland	Under Contract to March 16, 2039
WIXOM	City of Wixom	Under Contract to December 1, 2038
WOODHAVEN	City of Woodhaven	Under Contract to September 22, 2038
YPSILANTI (YCUA)	Augusta Twp, Pittsfield Twp, Superior Twp, York Twp, City of Ypsilanti, Ypsilanti Twp	Under Contract to May 19, 2038

Overall, DWSD has 92 percent of its annual recent water sales volume under agreements that extend beyond the planning period. Given the long term status of most contracts, detailed demand projections have been made for the current base of wholesale customers and retail customers.

It is possible that new customers will seek to join the regional water supply system during the planning period. It is also possible that one or more existing customers could choose to terminate

water services. Potential customer additions and losses, along with the impact on future water demand projections, are discussed in Chapter 4 Growth Scenarios and Demand Management.

3.4 Methodology for Water Demand Projections

TM-5 and TM-15 present detailed tabulations of water demand projections by wholesale and retail customer. **Table 3-4** below presents the methodology by which the numbers were developed and describes future scenarios that have been developed in addition the most probable projection.

Table 3-4: Water Demand Projection Methodology

Component of Demand	Identification	Basis of Present Day Water Use Estimate	Projection Methodology
Domestic – Indoor	Residential population served by DWSD water, based on maps and data provided by wholesale customers and SEMCOG. Population estimates were made for 2015, 2020, 2025, 2030 and 2035.	65 gallons per capita per day based on an analysis of metered indoor water in 4 communities from January to March 2013. Communities include Detroit, Warren, Novi and Rochester Hills.	65 GPCD held constant over the 20-year planning period, and then multiplied by the residential population in each 5-year interval. Scenarios are presented for future reduced per capita demands of 55 and 50 GPCD.
Outdoor Irrigation	Historic Maximum day and peak hour factors, after accounting for current and planned storage identified by wholesale customers.	Based on maximum day diurnal patterns documented in TM-3. Wholesale customers with existing or planned storage have a modified maximum day pattern to account for peak hour storage.	Present day diurnal pattern was held constant over the planning period, unless modified for the year where storage is proposed in the future. A scenario is presented for a lower system Maximum Day to Average Day ratio (1.75) in 2035 for selected supply alternatives.
Industrial, Commercial, and Institutional	Employment population by type of employment provided by SEMCOG for each wholesale customer service area and for the City of Detroit. Average water use by type of industry and institution based on data from AWWA and NAICS.	20 to 200 gallons per employee per day (GPED), depending on the mix of industrial, commercial and institutional businesses.	Present day GPED held constant over the planning period and then multiplied by the employment population in each 5-year interval A scenario is presented for Potential for new “Blue Water Economy” business water demands, particularly food and beverage.

Table 3-4: Water Demand Projection Methodology

Component of Demand	Identification	Basis of Present Day Water Use Estimate	Projection Methodology
Major Industrial Users	Individually identified as users of 25,000 GPD or more as provided by DWSD for FY2014 in the wastewater service area; and as identified by wholesale customer survey forms for areas outside of the wastewater service area.	Actual water use FY2013 or FY2014.	Held constant over the 20-year planning period. Scenarios for reduced water use by major industrial users are discussed in Chapter 4.
Fire Flow	Applicable for retail service area of Detroit and Highland Park. Wholesale customer maximum day and peak hour contract limits are intended to include fire flow requirements.	Insurance Service office standards.	Fire flows were not explicitly considered for the master planning scale of analysis.
Non-Revenue Water	Wholesale Customers: Comparison of three years of water sales to water purchases from DWSD. These data were available for 22 wholesale customers. <u>For Detroit, Dearborn and Highland Park:</u> based on findings of CS-1364 Water Audit.	In general, NRW for wholesale customers ranges from 10 to 20 percent, with an average of 15 percent. Used 10, 15, or 20 percent when data was not available. The higher or lower numbers were used based on relative age of water system.	Held constant over the planning period for wholesale customers. For the City of Detroit, average daily water loss is estimated to be approximately 90 MGD. Recommended practices to reduce this water loss were presented in the Product Plan Interim Report. For modeling, the current percentage of NRW was held constant for the planning period.

3.5 Domestic Demands

Domestic water demand is defined for this study as indoor household use associated with cooking, bathing, toilet flushing, and cleaning.

Domestic water demand in the United States has been declining for the last twenty years. This is due to the mandated use of water conserving plumbing fixtures and patterns of behavior to conserve water. Typical water use per household is approximately 200 gallons per household per day for residence of 2 to 3 persons.

In order to establish accurate per capita indoor domestic water use for the DWSD service area, a survey of four wholesale customers and three neighborhoods in Detroit was performed. Results of the

survey are described in TM-5. The range of domestic water use ranged from 58 to 72 gallons per capita per day (GPCD). An average of 65 GPCD was used for calculating future domestic demand projections.

3.6 Business Water Demands

3.6.1 Industrial, Commercial and Institutional

Industrial, commercial, and institutional water demands for Detroit and each wholesale customer were estimated based on projections of the number of employees and types of businesses in each municipality. SEMCOG maintains projections on numbers of employees and types of businesses for each municipality.

The North American Industrial Classification System (NAICS) provides a classification system for all types of business, which range from heavy industry, to retail commercial, professional services, schools, hospitals, among others. The NAICS classifications were used to establish an average daily water use per employee. Typical water demands for most businesses range from 20 gallons per employee per day to 200 gallons per employee per day, depending on the type of business. Lower numbers and higher numbers apply for certain types of businesses. Details of the methodology of developing these water demands are presented in TM-5.

3.6.2 Major Industrial Users

Major industrial users are defined as individual locations that use 25,000 gallons of water or more per day. Major industrial users are a subset of the industrial, commercial and institutional water demands described previously. However, because of the size of these individual water demands, these users were identified separately.

DWSD maintains a list of these users in its service area. The list is maintained by the Industrial Pretreatment Group in the Wastewater Division, and the list is called “Significant Industrial Users” (SIU). This list includes industry that meets the threshold for water use, as well as some industry with special wastewater characteristics, but lower daily water use.

For those wholesale customers that are not located in the DWSD wastewater service area, the major industrial users were identified by the customer on its Technical Data Request form. See Appendix TM-15 for additional information.

3.6.3 Blue Economy Initiatives

Appendix TM-7 describes economic initiatives to expand segments of businesses that are highly dependent on water in their business process. Examples include food and beverage industry, agriculture, mining, and certain types of manufacturing. These types of businesses currently represent less than 5 percent of DWSD’s average water demand. The goal of the Blue Economy Plan is to expand the number of water-dependent industries in the service area.

3.7 Outdoor Irrigation

Outdoor irrigation demands occur primarily in the summer months. Outdoor irrigation drives the maximum day and peak hour demand, which is typically in June, July or August.

Outdoor irrigation demand is weather dependent. The design maximum day demand of 1,014 MGD currently estimated for 2035 is a number that is expected to be reached about once every 3 to 5 years.

Table 3-5 shows maximum day and peak hour water use from the previous 10 years.

Table 3-5: Maximum Day and Peak Hour Demands 2005 to 2014

Year	Date	Maximum Day (MGD)	Peak Hour (MGD)
2005	June 27	1,104	1,347
2006	June 17	1,080	1,337
2007	August 3	1,128	1,388
2008	August 18	961	1,184
2009	August 5	804	989
2010	July 7	957	1,130
2011	July 21	1,000	1,205
2012	July 2	969	1,171
2013	August 26	761	914
2014	July 22	720	853

3.8 Non-Revenue Water

3.8.1 Trends in Nonrevenue Water

Nonrevenue water is the difference between the volume of water produced and the volume of metered or estimated sales. Water utilities generally have water production that exceeds water sales. This is due to meter inaccuracies, losses of water through leaks and breaks, and authorized unmetered public use of water for firefighting, water main construction and other purposes.

DWSD's rate setting process includes detailed reporting to wholesale and retail customers on revenues, prior period expenditures and proposed budgets for capital improvements, financing, and operation and maintenance. The information presented in the rates process documents the estimated volume of nonrevenue water. In the last seven years, nonrevenue water has been increasing in volume and as a percentage of total water production, as shown in **Table 3-6**.

Table 3-6: Trends in Nonrevenue Water

Period	Annual Volume of Nonrevenue Water (MGD)	Percentage of Total Water Production
July 2004 to June 2005	108.0	18.6
July 2005 to June 2006	106.2	17.7
July 2006 to June 2007	101.9	17.7
July 2007 to June 2008	139.2	23.2
July 2008 to June 2009	125.0	22.7
July 2009 to June 2010	113.6	22.0
July 2010 to June 2011	127.1	23.4
July 2011 to June 2012	145.5	26.2
July 2012 to June 2013	152.3	27.7
July 2013 to June 2014	165	30.7

3.8.2 Preliminary Water Balance for FY2012

The American Water Works Association (AWWA) has established terminology and procedures for analyzing and managing nonrevenue water. DWSD performed its first water audit in accordance with the AWWA procedures in 2006, and it performed a second water audit based on 2012 data. The purpose of these water audits is to analyze nonrevenue water and develop estimates for leakage, meter inaccuracy, and authorized, but un-metered public uses and develop strategies to reduce the NRW volume.

The first step in management of nonrevenue water is to establish best estimates of apparent causes of water loss. **Table 3-7** shows preliminary estimates prepared for this Water Master Plan Update based on results from DWSD's water audit program and its rate setting methodology.

Table 3-7: Preliminary Water Balance for FY 2012

Category	Estimated Volume (Million Gallons)	Approximate Percent of Total Production	Basis
Total Production	203,600		FY2013 recorded water production
Over-Registration at WTPs	10,000 to 20,000	5 to 10%	DWSD water audit 2006 pump tests and rate setting methodology
Net Production	183,600 to 193,600		
Water Sold to Suburbs	121,200		Actual sales to wholesale customers
Allowance for Transmission and Wholesale Meter Loss	10,000 to 20,000	5 to 10%	DWSD water audits and rate setting methodology
Water Sold to Dearborn	4,500		Actual sale to Dearborn
Allowance for NRW in Dearborn	800		DWSD water audit and rate setting methodology
Water Sold in Detroit	29,200		
Allowance for NRW in Detroit	20,000 to 30,000	10 to 15%	Rate setting methodology
Total Production	203,600		
Total Sales	150,400		
Nonrevenue Water (Total Production – Total Sales)	53,200	26.2%	Annual financial report

Source: CS 1396 2006 Draft Report; Sept 2013 BOWC Finance Committee Water Supply System Revenue Analysis; and Water Rate Methodology Table 14 Allocation of Non-Revenue Water.

Reducing non-revenue water is a demand management opportunity. Estimated leakage within the City of Detroit is 50 to 80 MGD, which is 10 to 15 percent of total water production.

Chapter 4 presents a discussion of future scenarios for water loss reduction.

THIS PAGE INTENTIONALLY LEFT BLANK.

4 Growth and Demand Management Scenarios

4.1 General

This chapter identifies future scenarios for growth in population and water sales, as well as scenarios for managing water demand. Collectively, these scenarios result in future water demands that range from approximately 5 percent higher to 10 percent lower than the baseline projections in Chapter 3.

Many water utilities face similar uncertainty when planning for 20 years into the future. In DWSD's situation, the significant loss of population in the service area since the year 2000, plus termination of water service agreements with the City of Flint and Genesee County have led to a close examination of future scenarios.

While there are recent events that have caused a decline in water sales, there is also data to suggest that water sales will increase over the planning period. Over 90 percent of future demand is associated with long term contracts; many of these customers expect growth in their service areas; there are inquiries to DWSD for new water service; and there are regional initiatives for economic growth.

The underlying factors that drive water consumption were analyzed, and from these factors a series of indicators were identified to track and report annually, and formally assess at 5-year intervals. These indicators will provide DWSD with the essential data to continue to regularly update this master plan and guide the capital improvement program.

The projections in this chapter present three scenarios for future growth: most probable, best case and worst case. In early 2015, it was decided to evaluate a fourth scenario: "worst worst" case. This fourth scenario was developed for sensitivity analysis for future financial projections, and all four scenarios are quantified in Chapter 10.

4.2 Major Factors and Trends

Six major factors that will influence water demands over the planning period were identified.

1. Residential Population
2. Domestic Water Use
3. Outdoor Irrigation
4. Industrial Water Use
5. Water Loss Reduction
6. Addition and Loss of Customers

Each of these factors and recent trends are discussed below.

4.2.1 Residential Population

Three projections were created for residential population: most probable, high and low. **Figure 4-1** shows the recent history of service area population from 2000 to 2015, along with projections to 2035. The black line is the most probable projection based on input from wholesale customers and SEMCOG. The light blue line is the high projection, and the darker blue line is the low projection, both based on SEMCOG’s range of future population projections.

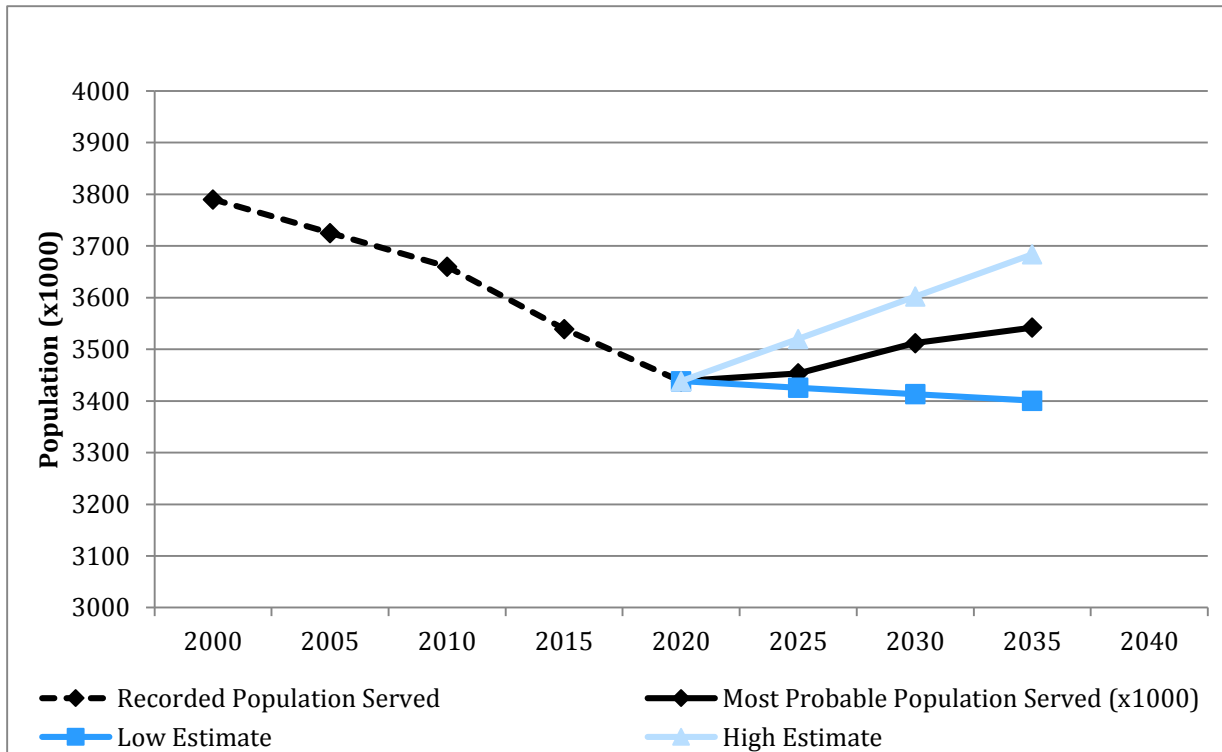


Figure 4-1: DWSD Water Service Population 2000-2035

The decline in water sales from the years 2000 to 2015 is consistent with national trends. The Michigan State University Institute for Public Utilities reports that annual declines of 1 to 3 percent per year are not uncommon in the United States during this period. The basis and development of these population projections and high and low scenarios are discussed in Appendix TM-2 and Appendix TM-15.

4.2.2 Domestic Water Use

The trend in household water use in the United States has been a decline that was first observed in the early 1980’s. Per capita domestic water use has fallen from 80 gallons per capita per day (GPCD) or more, to 60 to 70 GPCD in most recent studies. Projections by the Michigan State University Institute for Public Utilities point to domestic water use of 45 to 50 GPCD, with a theoretical lower limit of 35 GPCD. These lower numbers reflect new plumbing fixtures and water conserving appliances for household use, personal behavior dedicated to water conservation and reuse, changing attitudes and environmental ethic, and reduction in water use and waste in reaction to the increasing cost of water as a percentage of household expenses. Future regulatory action could also mandate or incentivize conservation.

Water conserving plumbing fixtures have been increasingly regulated through building and plumbing codes since 1982. Much of the impact of these regulations has already been experienced, but it can be expected to continue to impact per capita domestic water use through the planning period. Current estimates of domestic water use in the DWSD service area range from 58 GPCD to 77 GPCD. A range of 50 to 60 GPCD is possible by the end of the planning period.

4.2.3 Outdoor Irrigation

The Detroit metropolitan area has had outdoor irrigation regulations in various forms since the mid-1950's. Over half of DWSD's wholesale customers have enacted ordinances and/or public education programs to reduce outdoor irrigation peak demands on the regional system. A detailed evaluation of the quantitative impact of these regulations and public education programs has not been performed. However, peak hour water demand has fallen by more than 200 MGD in the last 10 years, and this can be attributed to a combination of peak hour storage, outdoor irrigation regulation and public education efforts.

Wholesale customers have constructed 95 million gallons of peak shaving storage at 30 locations. 15 million gallons of that storage has been built in the last 7 years and an additional 9 million gallons of storage is planned for the remainder of the planning period.

The impacts of this type of storage are significant in two ways:

1. First, this type of storage reduces the cost of water purchases for wholesale customers under the current rate structure.
2. Second, customer operated storage tanks reduce energy costs for pumping by DWSD. However, this reduction in energy cost is relatively small compared to other fixed costs for pumping and transmission facilities. The continuing reduction in revenue from water sales requires continued downsizing of the transmission system in some areas to align with current and future demands.

The City of Detroit's green infrastructure program could impact future trends in outdoor irrigation. This program, which DWSD is both encouraging and funding, has the goal of reducing current and future costs of managing wet weather runoff in the City of Detroit. Among the methods being promoted are rain barrels, cisterns, and stormwater retention basins, all of which can also be used to store rainwater and provide water for irrigation during dry periods. These efforts are intended to reduce peak summer water use for those customers who adopt the methods.

In some cases, DWSD is actively incentivizing large customers to store and reuse rainwater and stormwater. As an example, DWSD reports that a major manufacturer is utilizing a previously unused stormwater pond to retain and reuse runoff from its parking lot, up to a capacity of a few million gallons per storm. In return for holding this runoff and not releasing it into the sewer system, DWSD is providing the customer with a substantial reduction in their monthly drainage charge, which is billed to customers based on their acres of impervious are (e.g. paved lots and rooftops).

Currently DWSD plans to spend at least \$3 million per year on green infrastructure. It is likely that some of these activities will end up retaining rainwater for future irrigation use, and may reduce peak demands for water as well.

4.2.4 Industrial Water Use

Another trend is in water use by industry. DWSD Wastewater Division tracks the water consumption of its significant industrial users (SIUs) as part of its Industrial Pretreatment Program. SIUs are those industries which purchase at least 25,000 gallons per day of water. Water purchases by the SIUs have fallen from 45 MGD to 22 MGD from 2000 to 2014. Industrial water use has been declining due to increased attention to cost control, production efficiency, water reuse, and sustainability practices.

The Blue Economy Plan, and other economic initiatives, could begin to increase industrial and commercial water demand. A Blue Economy integrates water resources with jobs and development. On-going initiatives underway in Michigan's Blue Economy intend to grow businesses and jobs in water-enabled and water-related sectors by utilizing the region's abundant resources – water, academia, engineering and manufacturing workforces. Water-enabled and water-related industries are those portions of the economy that produce, implement, or are significantly affected by water research and innovation.

At present, Blue Economy water users represent less than 5 percent of DWSD's annual water sales. Food and beverage companies have particular potential for growth, and these represent half of the current Blue Economy water use. The proposed Blue Economy Plan has the potential for restoring some of the lost industrial water demand with new highly sustainable industrial and commercial water users. However, these changes are not expected to have a major impact on average daily demand, peak daily demand, or the many implementation actions proposed to reduce the system's capacity in order to lower fixed costs.

4.2.5 Water Loss Reduction

Many water utilities are implementing water loss reduction programs. These programs address the whole range of water loss, from water leaks, to metering improvements, and accounting and enforcement practices. Utilities are using these programs in reaction to aging infrastructure, limited water supply, or the need to reduce the cost of water purchases. Industry has responded with new technology for measuring, finding and rehabilitating water pipes. While DWSD has abundant supply, nonrevenue water has grown for the last 10 years. Reduction in real water loss would reduce the operating load on treatment plants, high lift and booster pump stations.

Chapter 3 identifies reduction in non-revenue water as a key priority. Real water loss through leakage in the City of Detroit is estimated to be 50 to 80 MGD. DWSD is vigorously applying a number of methods to locate and eliminate leakage. These efforts, in conjunction with replacement and rehabilitation of water distribution mains, are expected to produce significant reductions in water loss over the planning period. See Chapter 9 for recommended actions to reduce non-revenue water.

4.2.6 Addition or Loss of Wholesale Customers

Table 3-1 in Chapter 3 presents the water service contract status for current wholesale customers. The status of service contract was used as a basis for assessing the potential for loss of a customer. On the other hand, DWSD is receiving inquiries for water service from potential new customers. The following is a summary of ongoing or potential changes in the future service area:

- Wholesale customers in Genesee County are in transition. The City of Flint ended continuous water purchases in May 2014. Genesee County has provided notice to DWSD that it will end water purchases and the estimated termination date is in 2017.
- Lapeer County customers continue to purchase water from DWSD and are in discussion with DWSD regarding water supply beyond 2016.
- Some wholesale customers in the Downriver area have not signed a long term service contract and are considering water service from the City of Wyandotte.
- An examination of smaller water service providers to the west of the regional water service area was performed to document population growth, source water supply and cost of water service. Treatment and distribution regulations are becoming more stringent; this will tend to force consolidation of smaller water service providers. Potential new customers who may seek water service from DWSD in the future would likely be in one of the following three groups.
 - **Limited Groundwater Supply.** There is a significant groundwater aquifer to the west of the DWSD service area. This aquifer stretches from northwest Macomb County, through northwest Oakland County, and southeast Livingston County to northwest Washtenaw County. Some parts of this aquifer are impacted by regulated water quality contaminants. The location of the aquifer establishes a general boundary for cost-competitive water supply from DWSD. The aquifer delineates the western extent of DWSD's long term potential service area. As population growth continues, communities to the south and east of this aquifer could seek water service from DWSD in the future.
 - **St Clair River Water Service Providers.** Another group of potential customers include smaller water providers whose source water is the St. Clair River. As regulatory requirements and costs increase, some of these water providers could seek water service from DWSD in the future.
 - **Emergency Supply.** Larger water service providers, such as Monroe and Ann Arbor, could seek emergency supply agreements to address seasonal water supply. As noted earlier, the City of Flint has an emergency supply agreement with DWSD, and Genesee County is seeking an emergency supply agreement in 2016.
- DWSD began serving Highland Park on November 13, 2012. Before that time, Highland Park was a water service provider, but in 2012 it was not meeting water quality standards.
- There is an established trend for DWSD wholesale customers to sell water to adjacent communities. Most recently, Plymouth Township expanded its service area to include part of Salem Township.

4.3 Baseline Projections and High and Low Estimates

Each of the preceding factors was considered in reviewing projections for the major operating parameters of the treatment and transmission system. **Table 4-1** shows the factors in relation to the key parameters that they influence, including maximum day demand, peak hour demand, annual water sales, population served, real water loss, and industrial water sales. A baseline projection to the

year 2035 is presented, along with high and low estimates based on the trends discussed previously for each factor.

Table 4-1: Preliminary Projections , High and Low Ranges for 2035

Factor	Parameter	Last 5 Years Range	2035 Baseline	2035 High Bound	2035 Low Bound
Outdoor Irrigation	System Maximum Day Demand (MGD)	761 to 1,000	1,014	1,050	930
Outdoor Irrigation	System Peak Hour Demand (MGD)	914 to 1,200	1,100	1,200	1,020
Population, Domestic Demand, Industrial	Annual Water Sales (Million Gallons)	136,000 to 150,000	137,000	142,000	125,000
Service Area	Population Served (Millions)	3.5 to 3.7	3.54	3.68	3.40
Water Loss Reduction	Estimated Real Water Loss (MGD)	50 to 80	40	70	30
Industrial Demand	Food & Beverage Industries Water Sales (MGD)	2 to 3	5	10	3

4.4 Annual Tracking, Reporting and 5-Year Assessment

Table 4-2 presents a relationship between the factors that influence water use and measurable parameters that are indicators of the trend influence.

Table 4-2: Indicators for Factors Affecting Water Demand

Factor	Leading Indicators	Operating Parameter
Residential Population	Federal Census and annual municipal reporting of population, employment projections	Average Daily Demand and Annual Water Sales
Industrial Water Use	Industrial customer surveys	Average Daily Demand and Annual Water Sales
Domestic Water Use	Water use surveys based on billing records; new regulations	Average Daily Demand and Annual Water Sales
Outdoor Irrigation	Customer storage tank construction, projections discussed in contract re-opener discussions; new regulations	Maximum Day Demand
Water Loss Reduction	Leakage reduction measurements as described in AWWA Manual of Practice M36, Water Audits and Loss Control Program	Non-Revenue Water
Addition or Loss of Customers	Annual water revenue forecast, customer satisfaction surveys	Annual Water Sales

The indicators identified in **Table 4-2** provide a framework for collecting and maintaining data on the leading indicators that signal a possible impact the operating parameters of the regional water system.

It is proposed that DWSD and its customers develop procedures to collect data on leading indicators on an annual basis. Every 5 years, these data would be assessed with respect to changes observed in system operating parameters, and with respect to proposed improvements in this master plan and in DWSD's 5-Year Capital Improvement Plans

The forecast of growth, the range of scenarios, and 5-year monitoring of indicators provide a guide map that will allow DWSD to continue its focus on optimization of facilities, work force development, improving customer service and efficiency.

Assessing market indicators on a 5-year basis will allow DWSD to continue to make informed decisions for infrastructure investment. With respect to water treatment, all future scenarios confirm the need for a major reduction in water treatment capacity from 1,720 MGD to approximately 1,000 MGD.

Customers, both wholesale and retail, have demonstrated their ability to make relatively quick changes in their water use. However, it will take DWSD most of the planning period to optimize the treatment and transmission system to current and future demands. The infrastructure optimization process will be step-by-step and designed to address the range of potential forecasts for each operational zone of system. Each step of the optimization should be implemented based on the most current review of trends for the operational zone.

THIS PAGE INTENTIONALLY LEFT BLANK.

5 Source Protection and Water Quality

5.1 Overview of the Source Protection and Water Quality Plan

Customers of the DWSD share the Great Lakes watershed with approximately 35 million people in the United States and Canada. The Great Lakes are the world's largest source of fresh water, and its quality is exceptionally high relative to most fresh water sources. At the same time, the Great Lakes are subject to increasing pollution loads from agriculture, wet weather point and non-point source pollutants; chemical and oil spills, impacts of invasive species, and climate change.

DWSD is the largest water supplier to inhabitants of the Great Lakes watershed and it has a long and excellent reputation in water quality. DWSD supplies water to over 10 percent of the population of the watershed. (The City of Chicago supplies a larger population, but most of those served by Chicago live outside of the watershed). DWSD's significant role in the Great Lakes watershed creates a unique interest and responsibility for it to lead collaborative efforts for source water protection and water quality monitoring. The source protection and water quality plan proposes the following new leadership efforts by DWSD:

1. Update the Source Water Assessment, which was originally prepared in 2002.
2. Prepare Surface Water Intake Protection Plans for all three intakes.
3. Continue participation in Partnership for Safe Water, MiWARN, and the Huron to Erie Monitoring System.
4. Consider increasing the number of water quality staff to expand DWSD's role in these partnerships as needed, and to perform the new monitoring and special water quality studies recommended herein.
5. Update water quality goals for 2016 to 2025 with emphasis on repurposing and optimizing conventional filtration capacity at four plants, conversion to direct filtration at the Lake Huron plant, and maintaining regulatory compliance.
6. After 2025, update water quality goals again in response to new regulations that may take effect, and for new initiatives for multi-barrier disinfection and conversion from chlorine gas to sodium hypochlorite.

The plan proposes actions that in some cases go beyond existing minimum federal and state regulatory requirements, but are consistent with the best practices of major peer water utilities. The proposed actions will provide proactive information from which to make informed decisions on treatment plant upgrades and distribution system improvements required to protect public health and maintain compliance with water regulations quality in the future.

This chapter describes the detailed plan for source water protection and drinking water quality. The following three technical memoranda provide an in depth presentation on current and anticipated regulations and best practices to support the discussion in this chapter:

- Technical Memorandum 8: Watershed Management and Source Water Protection (TM-8)
- Technical Memorandum 9: Current and Future Drinking Water Regulations (TM-9)
- Technical Memorandum 10: Water Quality Monitoring (TM-10).

5.2 Regulations Governing Source Water

5.2.1 Introduction

This section outlines the current international, federal and state laws and regulations pertaining to source water quality that are relevant to DWSD. The regulatory information presented is based on current and historical literature from the US EPA, the MDEQ, and the Ontario Ministry for the Environment (OME). Additional detail is presented in TM-8 and TM-9 in the Appendix.

5.2.2 Bi-National Great Lakes Agreements

There are a series of Great Lakes water resources agreements spanning from 1995 to 2008, The Great Lakes Charter, originally published in 1995, established five principles for management of Great Lakes water resources:

1. Conserve the levels and flows of the Great Lakes and their tributary and connecting waters.
2. Protect and conserve the environmental balance of the Great Lakes Basin ecosystem.
3. Provide for cooperative programs and management of the water resources of the Great Lakes Basin by the signatory states of the United States and provinces of Canada.
4. Make secure and protect development within the region at the time of the agreement.
5. Provide a secure foundation for future investment and development within the region.

The Great Lakes Charter Annex is a supplementary agreement developed in 2001. The purpose of the Annex was for the Great Lakes Governors and Premiers to reaffirm their commitment to the five broad principles in the Great Lakes Charter. The Annex states: “The Governors and Premiers commit to further implementing the principles of the Charter by developing an enhanced water management system that is simple, durable, and efficient, retains and respects authority within the Basin, and, most importantly, protects, conserves, restores, and improves the waters and water-dependent natural resources of the Great Lakes Basin”.

In 2005, the Great Lakes Governors and Premiers signed the Great Lakes-St. Lawrence River Basin Sustainable Water Resources Agreement. At the same time, the governors endorsed the companion Great Lakes St. Lawrence River Basin Water Resources Compact, which became law in December 2008. These agreements reaffirm previous agreements and detail how the states and provinces will manage and protect the basin and provide a framework for each state and province to enact measures for water quality protection. The Council of Great Lakes Governors has a key role in approving actions

regulated under the Great Lakes agreements. The Council of Great Lakes Governors includes those of Illinois, Indiana, Michigan, Minnesota, New York, Ohio, Pennsylvania and Wisconsin. The Canadian counterparts are the premiers of Ontario and Quebec.

5.2.3 Canadian Source Water Protection Law and Regulation

The OME regulates surface water protection plans under the Ontario Clean Water Act of 2006.

Ontario requirements for source water protection are similar to those of the United States, but Ontario's requirements are mandatory, while those in the United States are mandatory for wells, but voluntary for surface waters. Ontario requires that each water utility:

- Establish an intake protection zone.
- Evaluate the vulnerable areas of the intake protection zone
- Identify existing and potential future land use activities that could be threats to source water quality.
- Evaluate water quality issues
- Evaluate water quantity for inland watersheds and groundwater systems

DWSD's Fighting Island intake lies within Canadian waters. Jurisdiction for development of a source water assessment and intake protection plan lies with MDEQ and DWSD. However, maintenance and repair activities need to be communicated and approved by the OME. Canadian land use activities have the potential to impact water drawn from this intake as well as the Belle Isle intake.

5.2.4 United States Source Water Protection Law and Regulation

The Safe Drinking Water Act (SDWA) was passed in 1974, and amended in 1986 and 1996. The SDWA gives the EPA the authority to establish and implement national drinking water standards and regulations. Public water suppliers have responsibility of meeting the standards set forth by the EPA and by their State regulatory agency when the State holds primacy as with Michigan. The 1996 amendments greatly enhanced the existing law by recognizing source water protection and public information as important components of safe drinking water. The 1996 amendments emphasize prevention through source water protection, enhanced watershed management and sustainable water use. Each state creates and implements its own program within the framework of the 1996 SDWA amendments. While the surface water protection program is voluntary, states have encouraged major water utilities to implement source water assessments.

5.2.5 MDEQ Source Water Assessment Program and DWSD Source Water Assessment

In 1998, Michigan amended its Safe Drinking Water Act to provide MDEQ with authority to regulate the source water assessment program. MDEQ established a voluntary program, and there are two principle parts:

1. Development of a Source Water Assessment (SWA)

2. Development of a Surface Water Intake Projection Plan (SWIPP)

MDEQ participated with the Great Lakes Protocol Workgroup to develop methodologies to perform the source water assessment. The group included representatives of the Great Lakes states, water utilities with intakes on the Great Lakes, USEPA Region 5 and other interested parties. A consensus based approach was used to develop protocols for the assessment, the elements of which include: identification of a critical assessment zone, source water protection areas, potential point and non-point contaminant sources, and criteria for high, medium and low sensitivities.

MDEQ contracted with the USGS in 2002 to prepare the source water assessments for DWSD. Summary level results of these assessments are shown on **Figures 5-1 and 5-2**. **Figure 5-1** shows the general location of DWSD’s Belle Isle and Fighting Island intakes, their critical assessment zones and applicable US and Canadian source water protection areas. **Figure 5-2** shows the general location of the Lake Huron intake and its source water protection area. Two important characteristics of source water assessment are “sensitivity” and “susceptibility”. Sensitivity is an indication of the natural protection afforded the source water by its natural setting. Susceptibility is based on factors within the source water protection area that may pose a risk to the water supply.

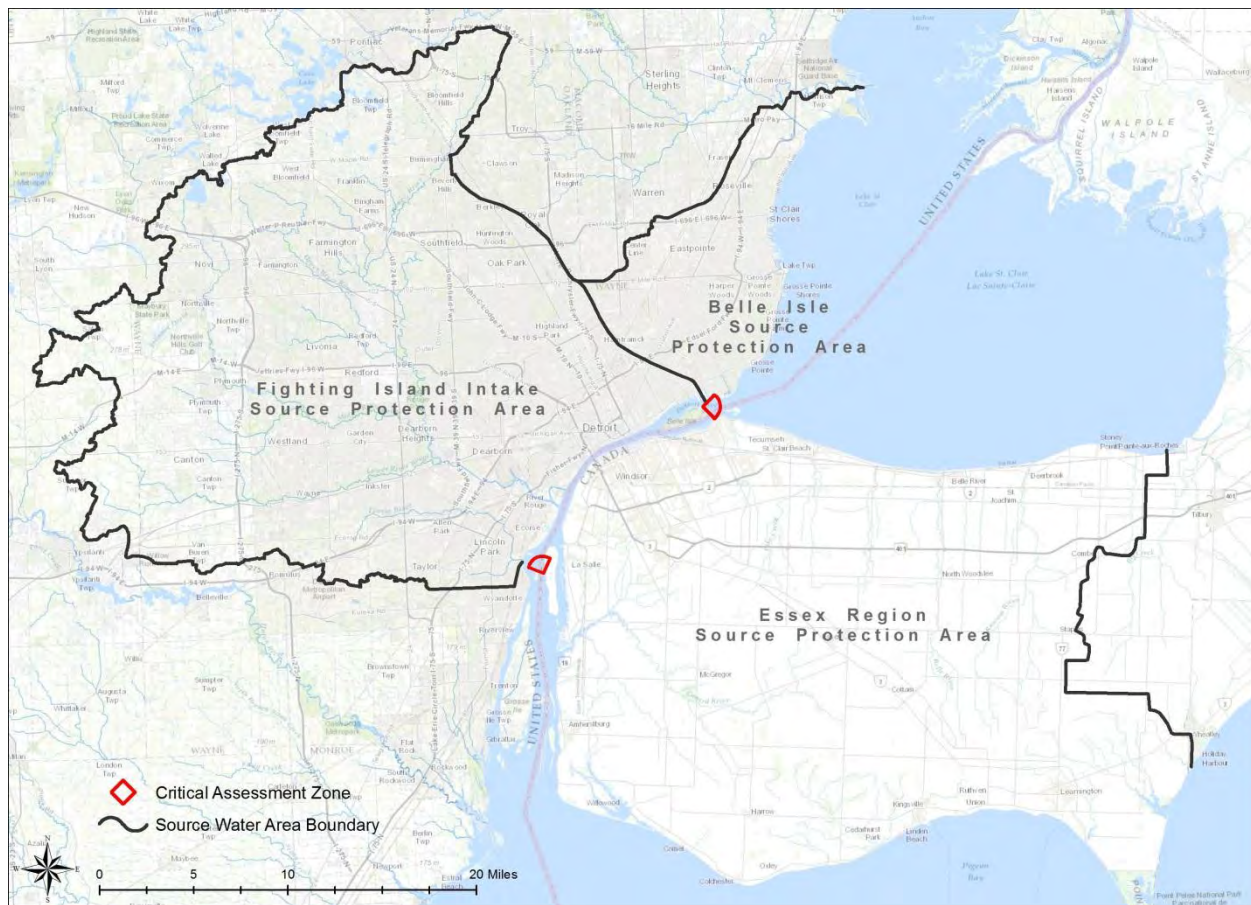


Figure 5-1: Belle Isle and Fighting Island Intake Source Water Assessment Areas

The Lake Huron source water was rated as moderate sensitivity and moderately low susceptibility based on the limited number of potential contaminant sources. The Fighting Island source water was rated as high sensitivity and high susceptibility based on the number of contaminant sources. The Belle Isle source water was rated as high sensitivity and high susceptibility based on the number of potential contaminant sources.

TM-8 provides additional mapping and information on the source water assessments.

5.3 Threats to Source Water Quality

5.3.1 Introduction

The source water assessments should be updated periodically to re-inventory contaminant sources and consider new factors. Going forward, a 5-year frequency of updating is recommended. This section discusses the key contaminant sources and influences that should be considered in each update.

5.3.2 Point and Non-Point Sources

Point sources include permitted wastewater treatment plant discharges and separate storm water discharges. Storm water discharges in Michigan are regulated by general storm water discharge permits for each municipality. This permit is met by implementing best management practices to address the following:

- Public education program on storm water impacts
- Public involvement and participation
- Illicit discharge elimination program
- Post-construction storm water management program for new development and redevelopment projects
- Construction storm water runoff control
- Pollution prevention and good housekeeping for municipal operations

There are many storm water discharges into the source water protection areas. The City of Detroit's storm water discharges are currently regulated by the jurisdictional general storm water discharge permit MIS04000 issued February 23, 2003. Other entities with storm water discharges have similar regulations and activities.

National Pollutant Discharge Elimination System (NPDES) permits provide water quality protection related to discharges from industrial facilities and wastewater treatment plants (WWTPs). These permits establish operating conditions and allowable discharge limits for a variety of parameters, including fecal coliform, nutrients and other pollutants. These types of discharges occur in all of the three intake source water protection areas. There are five WWTPs which discharge to the Detroit River (Detroit, Wayne County, Trenton, Grosse Ile Township and South Huron Valley Utility Authority). There are additional wastewater plants that discharge to Lake St. Clair and Lake Huron and tributary to the Detroit River.

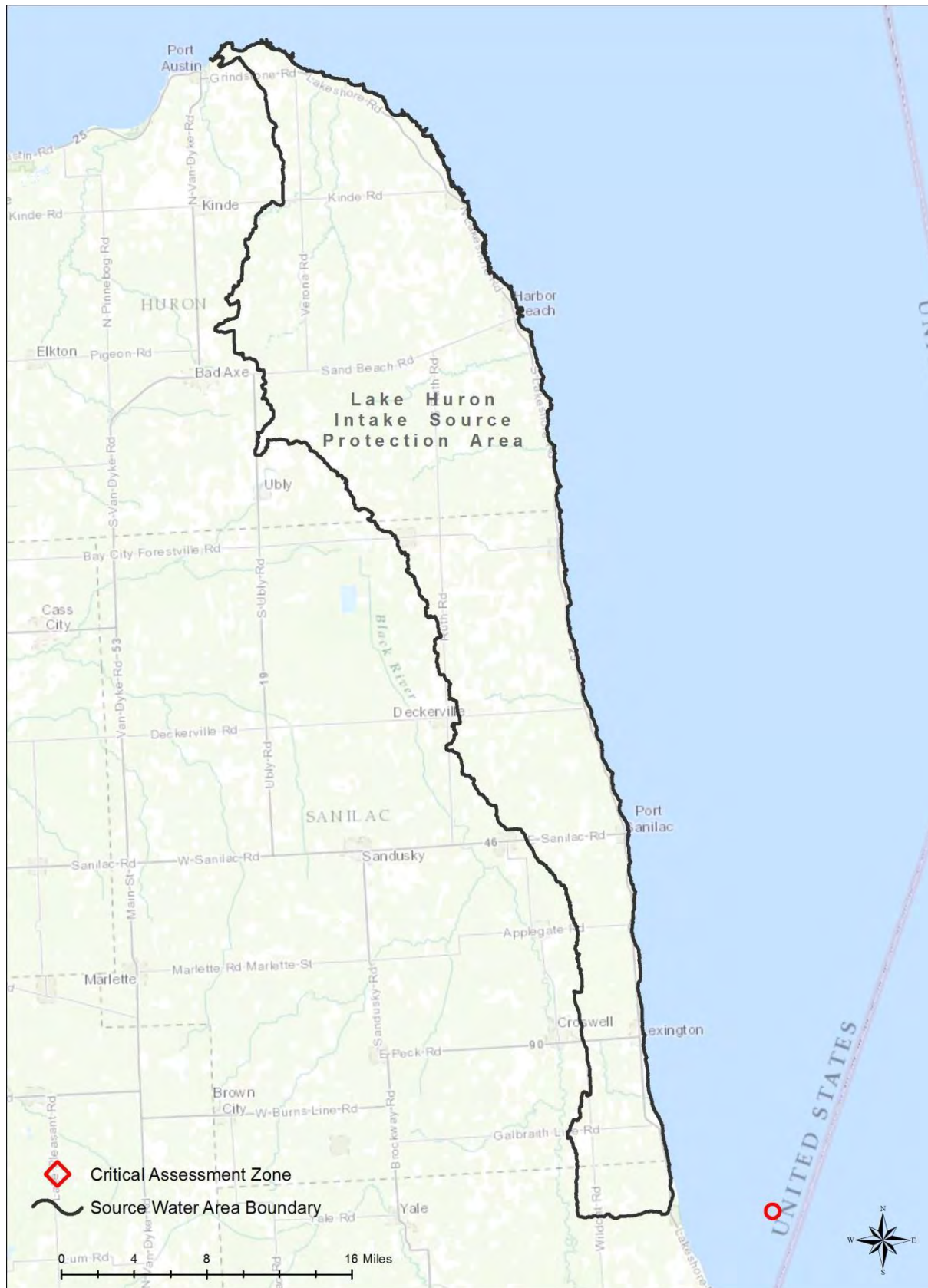


Figure 5-2: Lake Huron Intake Source Water Assessment Areas

Combined sewer overflow and sanitary sewer overflows (CSO/SSO) are typically associated with wet weather conditions and therefore occur intermittently. MDEQ tracks and reports CSO discharges on their website and issues annual reports on both CSO and SSO operations and discharges. There are many CSOs in the Lake Huron and Detroit River watershed and they generally discharge with a frequency of 4 to 20 times per year.

5.3.3 Spills

Spills of pollutants are a concern for source water protection. Spills may occur from:

- Industry/point source spills associated with local manufacturing or chemical storage. Both manufacturing and chemical storage are present in the watershed, particularly in the St. Clair and Detroit rivers.
- Transportation of substances can lead to spills on land or water. Such spills are unpredictable in duration, content and magnitude.
- Non-point sources typically impact water quality over a significant period of time. For example, seasonal accumulation of agricultural runoff.

The US Coast Guard maintains the spill reporting center under its authority as a National Reporting Center with the Department of Homeland Security. The State of Michigan and the Province of Ontario maintain respective spill alert systems. The State of Michigan Pollutant Emergency Alerting System (PEAS) was established in 1975, and the Ontario Spills Action Centre (OSAC) was established in 1985. Both organizations provide 24/7 operations to receive and disseminate information about spills.

From 2004 to 2013, there were 405 reported spills in Wayne County, 154 in Macomb County, 124 in St Clair County, and 11 in Sanilac County. The major spills involved oil, gasoline and polychlorinated biphenyls. There were over 1,000 spills reported for the province of Ontario in 2009. However, OME does not break out the spills by watershed.

5.3.4 Invasive Species

Zebra mussels and Quagga mussels have spread throughout the Great Lakes, rivers and inland lakes of the upper Midwest and Ontario. The mussels attach to any hard surface in the water. The ability of these mussels to attach in large clumps can create numerous problems, such as clogging water intake pipes and killing native mussels. The mussels are filter feeders and consume significant quantities of phytoplankton. The filter feeding results in improved lake clarity which in turn promotes macrophyte growth and extensive weed beds. The mussels' foul beaches, interfere with food webs, smother native mussels, impart taste and odor, and are linked to fish and wildlife die-offs. As such, they have a significant and typically negative impact on a water ecosystem. Quagga mussels are not currently as common as zebra mussels in the Great Lakes and often are found at greater depths.

Zebra mussels have been detected at DWSD intakes. Chlorination is practiced as a control measure at the Belle Isle intake but not at the other intakes. DWSD relies upon the large intake size as an approach to zebra mussel management. Blue-green algae blooms are another example of invasive species. These are discussed in the next section.

5.3.5 Climate Change

Climate change has multiple potential impacts on water quality and water quantity. Therefore, it is important to consider and plan for these impacts over the 20-year master planning horizon. In the Great Lakes area, reports of increased storm severity leading to rapidly fluctuating water quality and reports of increased cyanobacteria (blue-green algae) blooms leading to concerns over taste and odor and microcystin production. There is significant uncertainty associated with both the extent and timing of impacts so that it is difficult to predict the exact impacts over the next 20 years.

The USEPA anticipates that the Midwest could see increasing frequency and intensity of precipitation due to climate change. Excess runoff and snow melt could add to the extent of water quality changes. Among the potential impacts of climate change for DWSD are:

- Source water quality changes in concentrations and increasing fluctuations (pH, temperature, new pollutants)
- Increase in total organic carbon (TOC), pathogens, nutrients and cyanobacteria (microcystin production)
- Decrease in raw water alkalinity
- Elevated and fluctuating raw water turbidity
- Impacts of water quality changes on water treatment processes, distribution
- Changes in water demand due to changing precipitation patterns.

DWSD currently monitors most of these parameters and should continue to monitor and assess results at 5-year intervals. As discussed later in this plan, it is recommended that DWSD add to its monitoring program the occurrence of cyanobacteria, particularly at Fighting Island and at Belle Isle.

5.4 Plan for Source Water Protection

Based on the laws, regulations, and threats described above, four major actions are proposed over the planning period:

1. Update the source water assessment
2. Prepare surface water intake plans and perform associated monitoring
3. Expand involvement with Huron to Erie monitoring system.

These actions are described below.

5.4.1 Update of Source Water Assessment

DWSD's existing source water assessment was prepared in 2002. These type of assessments need to be updated periodically to consider results of water quality monitoring, changing land use, new regulations, and changes in the water quality threats described previously. DWSD should update the source water assessment within the first three years of the planning period. It is recommended that

DWSD engage both MDEQ and OME in planning and scope development for this work, and that a public involvement component be included. The updated source water assessment should update the contaminant inventory for each intake area, consider additional pathogen assessments, and provide recommendations on monitoring by the Huron to Erie Monitoring System (discussed below).

The AWWA provides guidance on source water assessment in its G-300 Source Water Protection Guidebook. The principle elements of AWWA's approach are:

1. Vision
2. Source Water Characterization
3. Program Goals
4. Implementation
5. Iterative Evaluation

The vision step should be done in conjunction with a public and stakeholder involvement program, including both internal and external stakeholders, including representation from the MDEQ and Province of Ontario.

Source water characterization should include an updated watershed delineation using newer GIS coverage information available since 2002. Hydrodynamic modeling tools and concurrent investigations by others can be used to update water quality and quantity and confirm or update previously identified key potential contaminants. Microtoxin should be added to list of potential contaminants.

Program goals should be developed around the risk assessment, susceptibility analysis and monitoring of potential contaminant sources, as well as history of compliance and regulatory requirements.

Implementation should include setting milestones for risk reduction, monitoring, emergency preparedness and response to specific, higher risk problems.

The iterative evaluation should be done on a 5- to 10- year frequency, depending on findings and specific goals of the assessment.

5.4.2 Prepare Surface Water Intake Protection Plans

DWSD has prepared and submitted applications to MDEQ for financial assistance to prepare Surface Water Intake Protection Plans (SWIPP) for all of its three intakes. The SWIPPs can be prepared in parallel with source water assessment, and should be completed within 3 years. DWSD began preparation of SWIPPs in January 2015. Critical issues for the SWIPPs include:

- Evaluate Zebra and Quagga mussel occurrence at all intakes.
- Establish chlorine feed system for mussels at Lake Huron. (Chlorine is currently fed at the Belle Isle intake.)

- In consultation with OME, improve routine access to the Fighting Island intake,
- After routine access is arranged, then establish chlorine feed system for mussel control at Fighting Island.
- Monitor cyanobacteria occurrence (microcystin production), particularly at Fighting Island/Southwest.
- Perform a historical water quality data review to determine if mussels have altered source water quality, algae types and concentrations, taste and odor, raw water pumping costs.

5.4.3 Huron to Erie Monitoring System

The Huron to Erie Monitoring System was established in 2007 as a means to provide early warning for chemical and petroleum spills in the St. Clair and Detroit river system. The network currently includes eight public water suppliers (shown in Table 5-1):

The operational goals of this group are:

- Install and maintain monitoring equipment at their water intakes
- Analyze water quality data every 15- to 30-minutes round the clock
- Share real time data with all participants
- Develop and use a water quality alarm system

The system is run as a collaborative effort among the utilities. Work is done on a part-time basis by one or more employees for each utility, or contracted by the respective utilities. **Table 5-1** shows the equipment installed in the system. Some equipment is newer, so historic data sets are incomplete.

Table 5-1. Existing Equipment for the Huron to Erie Monitoring System

Utility	Multi parameter sonde	TOC analyzer	Fluorometer	Microtox
Marysville WTP	X		X	
Marine City WTP	X			
Algonac WTP	X			
Ira WTP	X			
Mt. Clemens WTP	X	X		
Water Works Park II	X	X	X	X
Southwest WTP	X	X	X	X
Wyandotte WTP	X			
Monroe WTP	X	X	X	

The multi-parameter sonde has the capability to measure pH, temperature, conductivity, dissolved oxygen, oxidation reduction potential (ORP), turbidity and chlorophyll. The fluorometer is used to measure total hydrocarbons. Microtox is used to test for potential water contamination. It is based on

respiration of a strain of bioluminescent bacteria. Changes in bacterial respiration indicate a change in water quality.

The monitoring system provides high value with relatively low cost for public health protection. However, not all systems are fully functional or developed, maintenance and calibration are not regularly performed, and quality assurance and quality control procedures need to be improved. Some of the test equipment and procedures could be upgraded to newer and less expensive testing technology. An Event Detection System (EDS) should be added to the monitoring network in order to provide improved data analysis and alarm system.

More staff time and improved network capability are required to consistently meet the goals of the system. Given the value and importance of this monitoring system, it is recommended that DWSD consider ways to increase its participation in the operation of the system. Options include:

- Addition of one or more full-time staff;
- Extending DWSD's SCADA network to include the 8 monitoring sites; or
- DWSD and others providing additional funding for the network.

5.5 Regulations Governing Drinking Water

5.5.1 Introduction

Regulations that govern drinking water have evolved from laws governing public health in England shortly before the time that DWSD started to provide public water service in 1850. The Safe Drinking Water Act (SDWA) of 1974 established the first enforceable national standards in the United States. The 1974 Act was amended in 1986 and 1996, and both amendments broadened the scope of drinking water regulation. Public water suppliers have the responsibility of meeting the standards set by the EPA. The 1996 amendments greatly enhanced the existing law by recognizing source water protection, operator training, funding for water system improvements, and public information as important tools in maintaining safe drinking water.

Michigan passed the Michigan Safe Drinking Water Act 399 in 1976. The MDEQ holds primacy in the State of Michigan, and it adopts USEPA regulations largely unchanged.

5.5.2 Current Regulations

Current regulations are comprised of primary and secondary drinking water standards. The National Primary Drinking Water Regulations are legally enforceable standards that apply to all public water systems, although the regulatory details may vary with system size. These standards protect the drinking water quality by limiting the levels of specific contaminants that can adversely affect public health. There are current regulations for over 80 contaminants.

The National Secondary Drinking Water Regulations are federally non-enforceable guidelines regulating contaminants that may cause cosmetic effects or aesthetic effects in drinking water. EPA does not require public water supply systems to comply with secondary standards, but states may choose to adopt them as enforceable. MDEQ recommends but does not enforce secondary standards.

TM-9 presents a complete review of current regulations in the following categories:

- Surface Water
- Systems Relying on Purchased Water Sources
- Water Treatment Facilities and Distribution System

DWSD is in current compliance with all existing drinking water regulations.

5.5.3 Anticipated Regulations Before 2020

Upcoming regulations could present challenges to many water utilities across the United States because of potential new requirements for multi-barrier disinfection, conversion from chlorine gas to safer forms of chlorination, protection from exposure to lead, emerging contaminants and minimization of carcinogenic volatile organic chemicals, perchlorate and hexavalent chromium. **Table 5-2** provides a summary of anticipated new regulations. TM9 provides additional detail on each regulation.

Table 5-2: Proposed and Pending USEPA Regulatory Actions

Regulatory Action	Topic Addressed	Anticipated Impact on DWSD	Regulatory Date	Final/Compliance Date
Long Term 2 Enhanced Surface Water Treatment Rule (LT2ESWTR) sampling	Cryptosporidium control	Increased monitoring	Jan 2006 Final	Second round of sampling by 2015
CFATS (Chemical Facilities Anti-Terrorism Act)	Gas chlorine use	Conversion from Chlorine gas to sodium hypochlorite	2008 Draft	Unknown
Long-Term Lead and Copper Rule (LT-LCR) Revisions	Lead exposure	Corrosion control, lead water service removal	2014 Draft	2015 Final
Perchlorate	Perchlorate minimization	Increased monitoring	2014 Draft	18 months after draft rule
Carcinogenic VOCs (cVOCs)	Carcinogens minimization	Increased monitoring	2014 Draft	2016 Final
Hexavalent Chromium (Cr-6)	Chromium minimization	Increased monitoring	2017 or 2018 Draft	2019 or 2020 Final
Third Six-Year Review (SY3)	Regulatory process	Unknown at this time	2015 Draft	2016 final
Unregulated Contaminant Monitoring (UCMR3)	Regulatory process	Unknown at this time	May 2012 Final	2014 Sampling
Third Regulatory Determination (RD3)	Regulatory process	Unknown at this time	2013 Draft	2015 Final
Anticipated RD3 Regulations	Regulatory process	Unknown at this time	2016 or 2017 Draft	2018 or 2019 Final

5.5.4 Anticipated Regulations Beyond 2020

Anticipated regulations beyond 2020 could include new requirements for emerging contaminants, improved scientific methods, and updates to existing regulations. Further research into health effects of known chemicals or drinking water additives will spur further drinking water regulatory actions in the future. Potential regulatory actions may include those listed in **Table 5-3**.

Table 5-3: Potential Future Regulations

Regulatory Action	Topic Addressed	Potential Impact on DWSD
Fluoride	Lower MCL and recommended dose, cost savings	Reduction in fluoride dose
MTBE	New monitoring and treatment requirements	New monitoring requirements
CCL4/UCMR4	Development of next UCMR and CCL cycle	New monitoring requirements with potential for treatment of new contaminants
D/DBP3	Lower MCLs, Individual DBP regulations	New DBP mitigation strategies
Non-Regulated DBPs	New regulated compounds	New DBP mitigation strategies
LT3ESWTR	New pathogens monitoring and new microbial movement requirements, lower combined filter effluent turbidity	Improved filter operations and new monitoring requirements, need for multiple barrier approach to pathogen removal
EDCs and PPCPs	New regulated compounds	New monitoring requirements and potential for treatment of new contaminants
Regulatory approach	Consideration of sensitive subpopulations in any regulation	New public education requirements and enhanced treatment processes

5.6 Plan for Drinking Water Quality

Based on preceding review of current and anticipated regulations, a series of actions are proposed to manage the cost of regulatory compliance over the planning period. Most of the proposed actions relate to work processes. Therefore, these are organized in a way that will merge with the DWSD's ongoing optimization effort as follows:

1. Goals
2. People
3. Technology
4. Processes

5.6.1 Goal Setting

Water utilities establish operational goals for several reasons. First, operating goals provide a means to measure and optimize treatment process performance and efficiency. Distribution system operating goals are essential to the provision of high quality water to all customers. Second, operating goals are set higher than the minimum regulatory requirements so that monitoring results fall within compliance levels. Third, operational goals are used to manage any particular situations with respect to source water, treatment technology, storage and distribution.

Table 5-4 shows the relationship between water quality goals and treatment process/distribution system upgrades for DWSD. The table establishes 8 major groups of water quality characteristics, describes the impact on respective treatment processes, and then establishes goals for the first half and second half of the planning period. Goals are established for the first and second half of the planning period for reasons related to regulatory compliance:

1. DWSD is in compliance with current regulations, which are expected to remain in effect for the next ten years or beyond. During this 10-year period, DWSD will be reducing treatment capacity and optimizing.
2. New regulations are anticipated by 2020, and these may require the addition of new treatment process for multi-barrier disinfection and corrosion control.

DWSD has established goals for over 35 water quality constituents. Many of these goals go beyond minimum standards and meet best practices of the municipal water treatment industry. For example: DWSD's turbidity goal is to maintain finished water at less than 0.1 NTU (Nephelometric Turbidity Unit), while the regulatory standard is less than 0.3 NTU. There is no regulatory requirement for Taste and Odor, but DWSD operates with a goal of no objectionable taste or odor.

These goals guide DWSD's drinking water treatment and transmission and distribution system performance. **Table 5-4** is a thematic summary of the goals, while Table 1-1 in TM-10 presents the actual goals.

Table 5-4: Relationship of Water Quality Goals and Capital Improvements

Water Quality Characteristic	Goals Drive Changes to These Processes and Operations	Direction for Water Quality Goals and Capital Improvements	
		FY2016 to FY2025	FY2026 to FY2035
Particles and Turbidity	<p>Particle removal by rapid mix, flocculation, settling, and filtration.</p> <p>Membrane technology removes particles and certain pathogens.</p>	<p>Review and update goals based on current regulations and industry practice. No major changes are expected in current goals.</p> <p>Continued performance in accordance with current or more stringent goals will allow for conventional water filtration process to be maintained at all plants.</p> <p>Perform pilot testing and plan for change to direct filtration at Lake Huron to reduce operating cost.</p>	<p>Review goals based on monitoring results, new and anticipated regulations.</p> <p>Continued performance in accordance with current or more stringent goals will allow for conventional water filtration process to be maintained at all plants.</p> <p>Implement direct filtration at Lake Huron.</p>
Pathogens and Microbials	<p>Disinfection by chlorine, ozone, or multi-barrier disinfection processes; more effective chlorine contact time; shorter water age in the transmission system.</p>	<p>Review and update goals based on current regulations and industry practice.</p> <p>Plan for multi-barrier disinfection protection at Springwells.</p> <p>Plan for potential chlorine gas conversion after 2025.</p>	<p>Review and update goals based on monitoring results, new and anticipated regulations.</p> <p>Add multi-barrier disinfection protection at Springwells, plan for multi-barrier protection at all plants. Begin phased chlorine gas conversion program.</p>
Disinfection Byproducts	<p>Source water is naturally low in the organic precursors for these byproducts.</p>	<p>Review and update goals based on current regulations and industry practice. No major changes are expected in current goals.</p>	<p>Review and update goals based on new and anticipated regulations. Plan process upgrades, if necessary.</p>
Taste and Odor	<p>Operational performance of all processes; adding ozone assists in taste and odor control.</p>	<p>Review and update goals based on current regulations and industry practice. No major changes are expected in current goals.</p>	<p>Review and update goals based on new and anticipated regulations.</p>
Inorganics	<p>Coagulation, polymer addition, and corrosion control, pH adjustment, addition of orthophosphate, reduction in lead</p>	<p>Review and update goals based on current regulations, monitoring of lead, and industry practice.</p>	<p>Review and update goals based on new and anticipated regulations.</p>

Table 5-4: Relationship of Water Quality Goals and Capital Improvements

Water Quality Characteristic	Goals Drive Changes to These Processes and Operations	Direction for Water Quality Goals and Capital Improvements	
		FY2016 to FY2025	FY2026 to FY2035
Chlorine Residual and Water Age	Operational practice for reservoir turn-over; re-chlorination at distant points in the transmission system.	Establish goals based on current practice for reservoir operations and ultimate “treatment to tap” water age. Perform pilot testing to assess need for re-chlorination in the 72-inch main to Lapeer and Genesee County.	Review and update goals based on monitoring results and new and anticipated regulations.
Emerging Contaminants	Optimization of existing processes, or new processes to remove contaminants such as personal care products, microcystin, and others.	Special studies and monitoring programs as proposed in this plan.	Review and update goals based on monitoring results and new and anticipated regulations.

5.6.2 People

A 2012 study completed for Arlington County, Virginia established a relationship between the number of water quality staff positions in water utilities and total population served by the utility. DWSD performs all of the water quality analyses for the treatment plants. DWSD also performs water quality analyses for the distribution systems in Detroit. Wholesale customers perform the water quality analyses in their respective distribution systems, with the exception that DWSD performs total coliform rule (TCR) samples, lead and copper (LCR) samples, and customer complaint samples for most customers. Based on DWSD's responsibilities, the 2012 study indicates that water quality staffing should total between 30 to 34 full time staff. The current staffing level is approximately 22 full time staff. Therefore, current staffing should be considered and analyzed to ensure water quality compliance.

These new staff will enable DWSD to fulfill the recommendations in this chapter for regulatory driven monitoring, source water assessment updating, special studies for source water, treatment process changes and distribution system water quality. The organization of new staff should be performed within the context of DWSD's ongoing operational optimization program and the proposed consolidation and repurposing of water treatment plants as discussed in Chapter 6.

Staff Training and Responsibilities. DWSD should provide continuing staff support and training to facilitate its ongoing work to meet water quality goals, maintain laboratory certification, maintain regulatory compliance, and perform special studies as well as planning for the future. Specific recommendations include:

1. Familiarize staff and prepare a plan for Level 1 assessment requirements under revised total coliform rule (RTCR).
2. Continue to proactively track and plan for potential regulatory changes.
3. Review water quality goals and update to be more comprehensive of regulations and aesthetics. Consider more global approach that includes wholesale systems or water quality at the points of entry.
4. Implement the monitoring recommendations provided in Table 2-1 of TM-10 Water Quality Monitoring.
5. Develop and implement a plan to review and trend data routinely. Establish and utilize control limits for key parameters.
6. Document all sampling plans and update annually
7. Review current sampling plans per recommendations.

5.6.3 Technology

Technology changes proposed in this master plan include evaluation and pilot testing of the direct filtration process at the Lake Huron water treatment plant, upgrading and consolidation of laboratory facilities, and new monitoring technology.

Direct Filtration at Lake Huron. The Lake Huron water treatment plant has been approved by the MDEQ to operate with a “modified direct filtration” process. With the potential reduction of treatment capacity at Lake Huron and other plants (see Chapter 6), it is possible that a full direct filtration process could be used. The advantage of direct filtration is lower cost for chemicals, and elimination of rapid mix, flocculation sedimentation basins and associated operating costs.

Direct filtration appears to be feasible at the Lake Huron plant due to the low turbidity in the source water. The major steps to evaluating the feasibility of this process are:

1. Meet with the MDEQ to review any pilot or demonstration work requirements and collect any additional data required. While new water treatment plants are required to conduct pilot studies per 10-States Standards, converting an existing treatment plant to direct filtration may only require in-plant demonstration studies.
2. Review existing treatment processes relative to typical direct filtration design parameters. These include:
 - a. Filter design parameters: media type, media configuration, filter loading rate, maximum available filtering head and historical filter treatment performance when operated in conventional and direct filtration modes including filter loading rate, filter run time, unit filter run volumes, backwash frequency, and head loss accumulation rates. Review of these parameters will be important to determine what upgrades will be required, if any, to convert to year-round direct filtration. .
 - b. Design and operational information as mentioned above can be compared against industry best practices for direct filtration. For example, experience has shown that a deep-bed coarse mono media filter should have greater capacity to cope with algal and diatom blooms with reasonable filter runs than traditional dual-media beds with 1.0 mm effective size anthracite.
 - c. Conduct pilot and/or full-scale evaluations if required to validate year-round direct filtration process at Lake Huron. Bench-scale tests should also be performed to screen alternative coagulants and polymers for cold water coagulation to produce a filterable floc. Polyaluminum chloride (PACL) may be a promising coagulant for cold water direct filtration application. Filter aids should also be considered for year-round application at low doses.
 - d. Complete survey of other direct filtration plants treating Lake Huron water such as the Lake Huron plant referenced in Foley, 1981 (“Experience with Direct Filtration at Ontario’s Lake Huron Treatment Plant,” Journal AWWA, 1981).
 - e. Based on the outcomes of the above steps, prepare process design criteria and cost estimates for direct filtration upgrades for Lake Huron WTP.

Laboratory Facilities. Most of DWSD’s laboratory facilities are 25 or more years old. Recommended upgrades are listed below:

1. Maintain central laboratory facility and capacity at Water Works Park II.
2. Rehabilitate the laboratory at Springwells plant as part of the existing SP-563 construction contract.
3. Begin to phase out operations at the Northeast plant starting in 2016, and eliminate this laboratory by 2020 as part of the repurposing plan outlined in Chapter 6.
4. Review laboratory operations between the Springwells and Southwest plants, and determine if it is more cost-effective to perform more water quality analysis at Springwells and less at Southwest.
5. Plan longer term laboratory improvements at Lake Huron, to be constructed after 2025.
6. Acquire and implement a Laboratory Information Management System (LIMS). Such a system will streamline compliance reporting to MDEQ, improve QA/QC processes within DWSD and enhance data sharing among DWSD facilities and wholesale communities.

New Monitoring Technology. New monitoring technology will enhance compliance with new regulations and improve operational control of the treatment and transmission systems.

1. Evaluate data needs and gaps in existing online instrumentation in the source water, treatment plants and transmission system. Add online instruments as determined per this study.
2. Re-evaluate the potential for using installed distribution system sampling stations at select locations in the distribution system. The difficulty in establishing satisfactory transmission system sample points is an ongoing challenge for DWSD and therefore dedicated owned stations may be an option. This type of monitoring technology should be explored by DWSD and its wholesale customers.

5.6.4 Processes

Key recommendations for new processes offered relating to water quality monitoring and assessment are presented below.

5.6.4.1 Water Quality Monitoring

DWSD tests source waters, finished waters and the distribution system for a wide variety of parameters (see **Table 2-1 in TM-10 Water Quality Monitoring**). In addition, special studies are conducted through Water Research Foundation participation, consultants, the DWSD Water Quality Group and regulatory requirements such as the Unregulated Contaminant Monitoring Rule.

In addition to testing DWSD source and finished waters, DWSD tests the City of Detroit distribution system. DWSD collects and analyzes samples for retail customers. DWSD also provides total coliform

sample collection and testing for 84 of the 127 communities. While this water quality monitoring program is robust, some detailed improvements are recommended as shown in **Table 2-1 in TM-10**.

5.6.4.2 Maintaining Water Quality in the Transmission and Distribution Systems

DWSD does an excellent job in maintaining water quality in its transmission and distribution systems. The AWWA Manual of Practice Water Quality Control in Distribution Systems outlines standard procedures which should continue to be followed. Minimum pressures are monitored closely, reservoir pumps are automated to turn-over reservoir volume within 3 days, chlorine residuals are monitored throughout the transmission system, and water main repairs are flushed and chlorinated before returning a main to service.

DWSD tracks customer complaints in the retail distribution system. DWSD receives customer inquiries on topics such as rusty water, lead, odor, and others. In the past, odor had been a frequent complaint related to algal blooms and zebra mussels, but the frequency has declined in recent years. Rusty water complaints can occur in areas with high water age and old unlined cast iron pipe. All water quality data complaints are captured by type, location and time in a database. Hydrant flushing is the primary approach employed to improve customer concerns. Onsite investigation and sampling are conducted when deemed appropriate.

DWSD should consider developing a database of wholesale customer complaints within the wholesale customer distribution systems.

Changes in future water use patterns in the transmission system and distribution system should be watched. DWD is already assessing options for satellite chlorination in the 72-inch water main that was constructed to originally serve Flint and Genesee County, but now serves Genesee County and the Greater Lapeer Utility Authority. Within the City of Detroit, many water distribution mains serve a greatly reduced residential population in over 25 percent of the City. These water mains could be subject to increasing water quality problems due to long water age.

5.6.4.3 Special Studies and Projects

A number of special studies and water quality projects are recommended to enhance the understanding of both current and future water quality challenges. These special studies should be performed within the next 5 to 10 years in order to have new knowledge pertinent to anticipated future regulations, and to refine water quality goals as described earlier. These special studies include:

1. (Re)assess perchlorate concentrations in source water
2. (Re) assess MTBE concentrations in source water
3. Develop an inventory of lead pipe and lead connections in distribution system (in progress)
4. Assess strontium in source water and finished water (2014 UCMR3)
5. Assess hexavalent chromium in distribution system (2014 UCMR3)
6. Continue monitoring of EDCs (endocrine disrupting compounds) and PPCPs (pharmaceuticals and personal care products) in source water

7. Assess HAA9(Haloacetic acids) concentrations
8. Deliver CCR (consumer confidence report) via provision of URL on postcard and/or water bill (ongoing)
9. Develop design criteria for conversion from gas chlorine to hypochlorite
10. Assess zebra mussel impact on water quality
11. Evaluate aluminum occurrence and approaches for minimization
12. Monitor microcystin, MIB and geosmin when algae blooms occur
13. Perform a study of UV254 absorbance and UV transmittance prior to any UV disinfection planning or design.

TM-8, TM-9 and TM-10 provide detail on the scope and objective of each special study and water quality project.

5.6.4.4 Emergency Response Planning

The Emergency Response Plan Technical Memorandum was submitted separately to DWSD in November 2013. The Master Plan Update team performed a review of DWSD's existing Emergency Response Plan, and made recommendations. The DWSD's Emergency Response Plan and the review performed under Master Plan Update are confidential documents for reasons of security.

DWSD is currently updating its Emergency Response Plan, and this plan includes procedures for responding to water quality emergencies.

5.6.4.5 Work Groups and Partnerships

DWSD has facilitated a robust customer involvement program for drinking water customers for over 10 years. This program has demonstrated the value of dialogue and facilitated discussion among DWSD staff, wholesale and retail customers, regulatory agencies, industry partners and other stakeholders. Customer involvement work groups and partnerships with other water utilities and agencies should continue throughout the planning period and specific objectives should evolve to address the challenges of the regional utility. While DWSD and its customers have several work groups and discussion topics underway, potential new topics include the following:

1. Meeting water quality regulations in the transmission and distribution system can best be achieved by close collaboration between the water quality staff of DWSD and its wholesale customers. It is recommended that a team of water quality personnel from DWSD and its wholesale customers form a water quality group to facilitate discussion and coordination of activities.
2. To meet finished water quality objectives, a coordinated team that includes representatives from each plant and the water quality group should be implemented

3. Develop a program and costs for providing lab services to wholesale customers.
4. Continue participation in Partnership for Safe Water and MIWARN. Consider participation in the AWWA Distribution System Optimization programs.

6 Water Treatment

6.1 Summary of the Water Treatment Plan

DWSD owns and operates 5 water treatment plants. These plants have a design capacity of 1,720 MGD, they are in compliance with current regulatory requirements, and they produce high quality water for the service area.

Chapter 4 identifies a potential range of required water treatment capacity in 2035 from 930 to 1,100 MGD, based on different scenarios for population growth, service area expansion, and per capita water use. Therefore, DWSD has significant excess capacity for water treatment relative to projected water demands.

A needs assessment for all water treatment plants was performed in order to determine the most cost-effective approach to reduce excess capacity and maintain or improve the level of service to customers throughout the planning period. Improvements were identified for regulatory compliance, safety, energy and operational efficiency. With the exception of the newest plant, Water Works Park, the other 4 plants were built or acquired in the period 1930 to 1970. The plants are in compliance with current regulations for water treatment. While there is no imminent change in regulatory requirements, by the end of the planning period DWSD can expect to address conversion from chlorine gas to sodium hypochlorite and extension of multi-barrier disinfection. The projected capital needs for all 5 plants to maintain current regulatory requirements and current rated capacities are approximately \$981 million over the planning period.

In addition to the needs assessment, a life cycle cost analysis was performed to consider O&M as well as capital needs, the cost to decommission and repurpose plants, the cost to build new transmission mains for water supply, and the cost to assure a level of redundancy typical of large water utilities.

Figure 6-1 summarizes the life cycle cost analysis and shows that repurposing the Northeast and Southwest plants is the alternative with the lowest life cycle cost. A detailed review of the costs of repurposing Northeast and Southwest plants shows that under some economic forecasts, the most cost-effective alternative is to repurpose only the Northeast plant and continue to operate the Southwest plant. Therefore, it is proposed that DWSD move forward with repurposing the Northeast water treatment plant by 2020, and then re-examine the cost effectiveness of continuing to operate the Southwest plant or repurposing it.

DWSD has already taken the initial steps in its FY2016 CIP to reduce the capacity of its water treatment plants. The FY2016 CIP was developed with substantial input from the Water Master Plan Update.

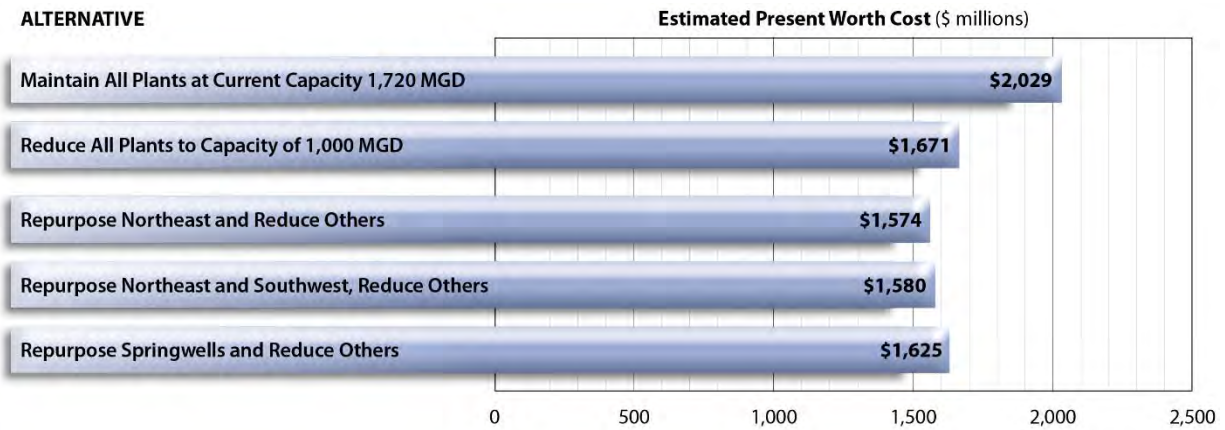


Figure 6-1: Summary of Life Cycle Cost Analysis¹

The water treatment consolidation and repurposing program outlined in this chapter builds on the plan for regulatory compliance and source water protection outlined in Chapter 5. The new consolidation and repurposing program will reduce DWSD’s costs for operation and maintenance, as well as reduce its immediate capital costs for plant upgrades and long term costs for regulatory compliance. This program builds on DWSD’s ongoing initiatives for optimization, asset management, and customer service, and it maintains high level of service standards in drinking water quality, pressure and volume.

6.2 Existing Water Treatment Facilities

DWSD owns and operates five water treatment facilities which provide water to 3.54 million customers in Southeastern Michigan. The Springwells, Northeast, Southwest, Lake Huron, and Water Works Park Water Treatment Plants have a maximum rated treatment capacity of 1,720 million gallons per day and firm high service pumping capacity of 2,400 million gallons per day. The high service pumping capacity exceeds the rated treatment capacity to assist in meeting peak hourly demands from finished water storage. Applicable treatment and pumping capacities and other data for each plant are shown in **Table 6-1**.

¹ The costs in Figure 6-1 include allowances for financing costs. Financing costs were not explicitly included in the life-cycle cost analysis presented in TM-6. Therefore, the values in Figure 6-1 differ slightly from the values in TM-6 and Table 6-6.

Table 6-1: Water Treatment Plant Capacity, Finished Water Storage and Areas Served Summary

Facility	Year Placed in Service	Rated Treatment Capacity (MGD)	Firm High Service Pumping Capacity (MGD)	Finished Water Storage Volume (MG)	Areas Served
Springwells WTP	1931 First Train; 1958 Second Train	540(1)	260 Intermediate Pressure District 450 High Pressure District	60	Detroit and Northern Wayne County, Eastern Washtenaw County, Oakland County, Southeastern Macomb County, Western Wayne County
Northeast WTP	1956	300	400	30	Northeast Detroit/Wayne County, Southern Macomb County, Southeast Oakland County
Southwest WTP	1964	240	310	30	Southern Wayne County, Northern Monroe County, Eastern Washtenaw County
Lake Huron WTP	1974	400	420	44	Genesee County, Lapeer County, St. Clair County, Macomb County, Oakland County
Water Works Park WTP	2003	240	560	28	Eastside of Detroit, Eastern Wayne County
System Totals:		1,720	2,400	192	

1 - Filter upgrades at Springwells limit plant capacity to 350 MGD until construction is complete.

Four of the five plants (Northeast, Springwells, Southwest and Water Works Park) are conventional treatment facilities with the following process trains: rapid mix, coagulation, flocculation, sedimentation, granular media filtration, and disinfection. Lake Huron is the only facility which is operated as a “modified direct filtration” plant, which means the sedimentation basins do not require a minimum detention time of 4 hours. In addition, Water Works Park is the only plant which employs intermediate ozonation for primary disinfection control. All five plants use the same chemical systems including alum for coagulation, chlorine for pre-oxidation and primary disinfection (excluding Water Works Park), powdered activated carbon (PAC) for taste and odor (T&O) control, phosphoric acid for corrosion control, and fluoride for dental health protection. Polymers are also added at several facilities to enhance coagulation and filtration as well as for thickening and dewatering of alum residuals.

Two of the five plants, Southwest and Water Works Park, employ automated residuals removal from the sedimentations basins. The residuals are thickened and dewatered on site along with backwash wastewater, and disposed of at landfills. Lake Huron’s basins are cleaned manually on an annual basis

and the sludge is discharged to the sludge drying lagoons. The lagoons also receive thickened solids from the waste wash water treatment facility which processes filter backwash wastewater. The Springwells and Northeast plants do not have automated alum residuals collection in the sedimentation basins or a thickening treatment process on site for alum residuals or backwash wastewater. At both facilities, the basins have been manually cleaned on an annual or biannual basis and the solids discharged to the wastewater collection system; backwash wastewater is also discharged to the wastewater collection system. Over the last two years, DWSD has assessed the impact of the water treatment residuals on its wastewater treatment plant, and is seeking solutions to minimize the residuals loading. A pilot is planned at Northeast in which the mud valves in the sedimentation basins will be opened approximately every two months to discharge sedimentation basin solids to the collection system. This process will be adopted at Springwells if the pilot test at Northeast is successful.

A more complete description of treatment processes is provided in TM-13 in the Appendix. A general description of each plant is presented below.

6.2.1 Springwells Water Treatment Plant

The Springwells WTP is the oldest of the DWSD water treatment facilities. The first train was constructed in 1930 and has a maximum rated capacity of 340 MGD and the second train constructed in 1958 has a maximum rated capacity of 200 MGD, for a total capacity of 540 MGD. Like Northeast, the Springwells plant receives its raw water from the Belle Isle Intake. The raw water influent is screened, chlorinated and fluoridated at Water Works Park before it is conveyed to Springwells. The low lift pumps lift the raw water for treatment through the process trains, which operate independently. The 1930 train provides hydraulic mixing through a baffled chamber for rapid mixing while the 1958 train has mechanical rapid mixers. Both trains have flocculation, sedimentation and filtration treatment units. **Figure 6-2** shows a schematic diagram of the Springwells Water Treatment Plant.

A major project to upgrade the Springwells plant, SP 583, was underway during the preparation of this Master Plan Update. This project includes a complete replacement of the 1958 filters and a limited replacement of some of the 1930 filters. A laboratory upgrade, yard piping and other site improvements, and electrical improvements are also included in this project.

6.2.2 Northeast Water Treatment Plant

The Northeast Water Treatment Plant was constructed in 1956 to serve growing suburban populations east and north of Detroit. The source of raw water is the Belle Isle intake, located in the Detroit River, which also serves Springwells and Water Works Park. The raw water is chlorinated, fluoridated, and screened at WWP before it flows to Northeast by gravity. Low lift pumps lift the raw water to the process trains, which operate in parallel. With a maximum rated capacity of 300 MGD, the plant process trains consist of rapid mix, flocculation, sedimentation, and dual-media gravity filtration. **Figure 6-3** shows a schematic diagram of the Northeast Water Treatment Plant.

6.2.3 Southwest Water Treatment Plant

The Southwest Water Treatment Plant was constructed in 1963 at which time it was owned and operated by Wayne County. Through an agreement with Wayne County, DWSD purchased this plant to regionalize water services in Southeast Michigan. Raw water for Southwest flows by gravity from

the Detroit River through an intake at Fighting Island. The plant has a rated capacity of 240 MGD. The original plant was designed with the ability to be upgraded to 320 MGD via equipment replacement. There are also spare raw water conduits which can accommodate an expansion up to 480 MGD. The low lift pumps lift the raw water for treatment through the process trains which operate in parallel. The Southwest Water Treatment Plant also has a Residuals Handling Facility to treat filter backwash wastewater and alum sludge residuals. **Figure 6-4** shows a schematic diagram of the Southwest Water Treatment Plant.

6.2.4 Lake Huron Water Treatment Plant

Lake Huron was constructed in 1974, initially designed as a conventional water treatment facility. In 2004, after completion of a pilot study along with various upgrades to the process trains, the MDEQ rated the maximum capacity of Lake Huron at 400 MGD. Lake Huron is the only DWSD facility which is operated in “modified” direct filtration mode. The sedimentation basins do not meet 10-State standards and thus are not considered to be true settling basins by the MDEQ. The raw water source for the plant is Lake Huron. The raw water tunnel is designed for a maximum capacity of 1200 MGD and 800 MGD during cold weather. The plant was constructed with provisions to increase the capacity by adding additional process trains and pumping units to obtain the maximum production capacity of 1200 MGD.

In the early 2000’s a variety of process treatment improvements were constructed at the Lake Huron WTP. These improvements included new high lift and backwash water pumps (including discharge piping and valves), rehabilitation of two clear wells and the high service suction well, filtration capacity improvements, pretreatment improvements and filter control modification, and a new treatment facility for filter backwash wastewater. **Figure 6-5** shows a schematic diagram of the Lake Huron Water Treatment Plant.

6.2.5 Water Works Park Water Treatment Plant

Water Works Park II began operating in 2003 as a conventional surface water treatment plant. The original Water Works Park water treatment plant was razed and a new facility was constructed on the same site. The raw water source for the plant is the Belle Isle intake on the Detroit River. The plant has a maximum rated capacity of 240 MGD and is DWSD’s first facility with ozone disinfection facilities as well as a Residuals Handling Facility to treat filter backwash wastewater and alum sludge residuals. The plant was designed to use independent process trains - a minimum of two process units are provided for each treatment process. In addition, all conveyance facilities such as pipelines, junction chambers, channels, and wet wells are configured to provide a minimum of two treatment pathways. **Figure 6-6** shows a schematic diagram of the Water Works Park Water Treatment Plant.

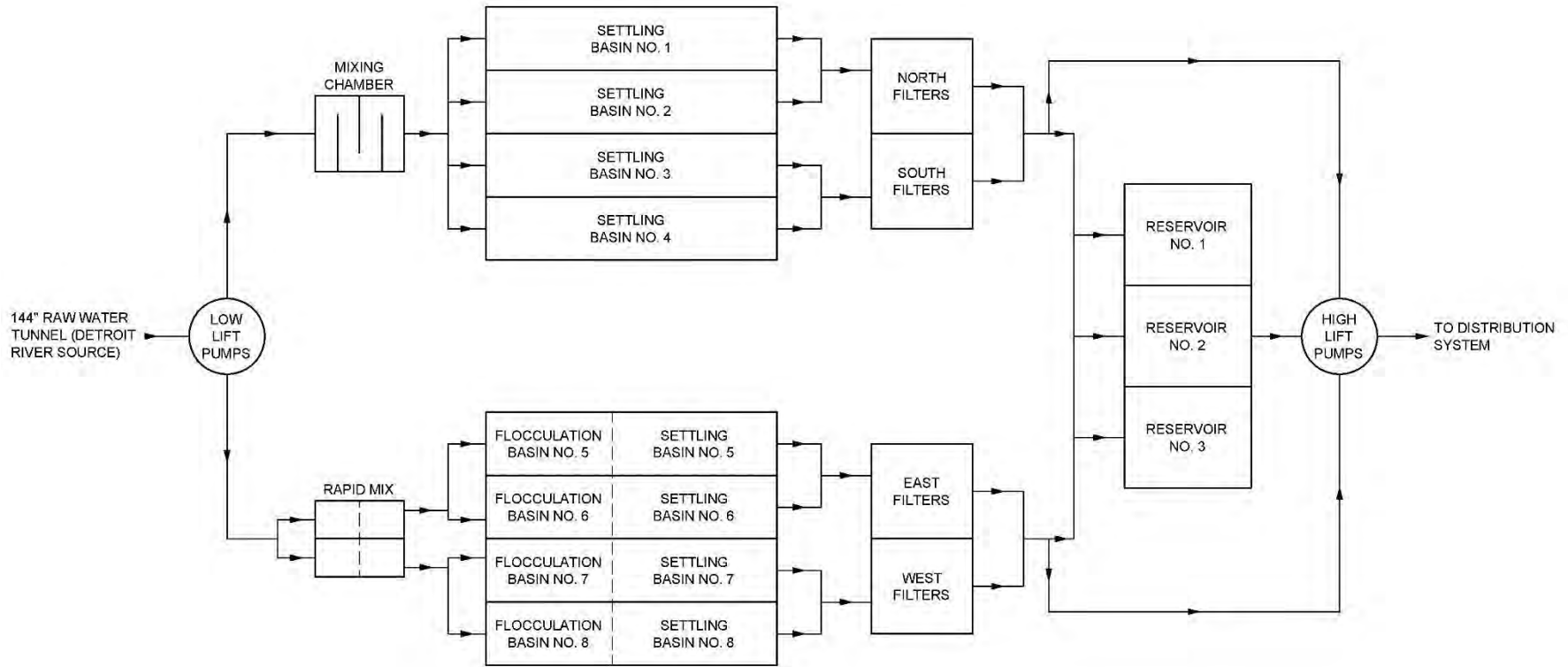


Figure 6-2: Springwells Water Treatment Plant

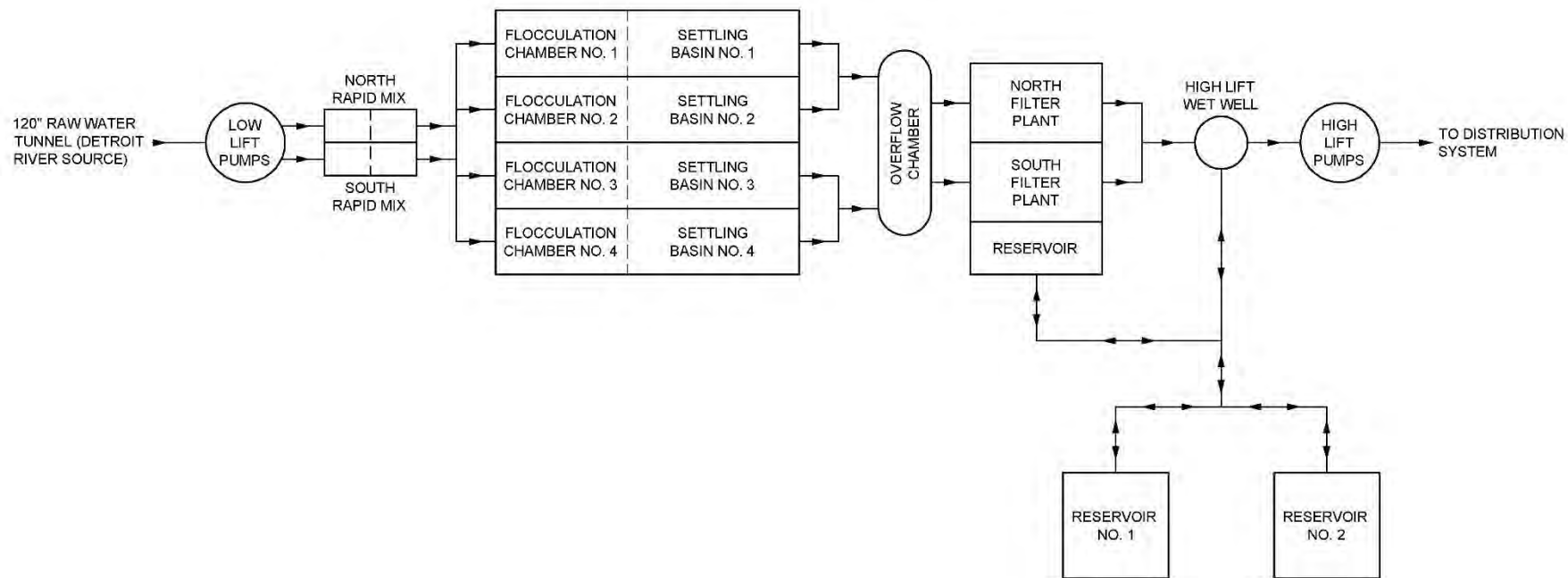


Figure 6-3: Northeast Water Treatment Plant

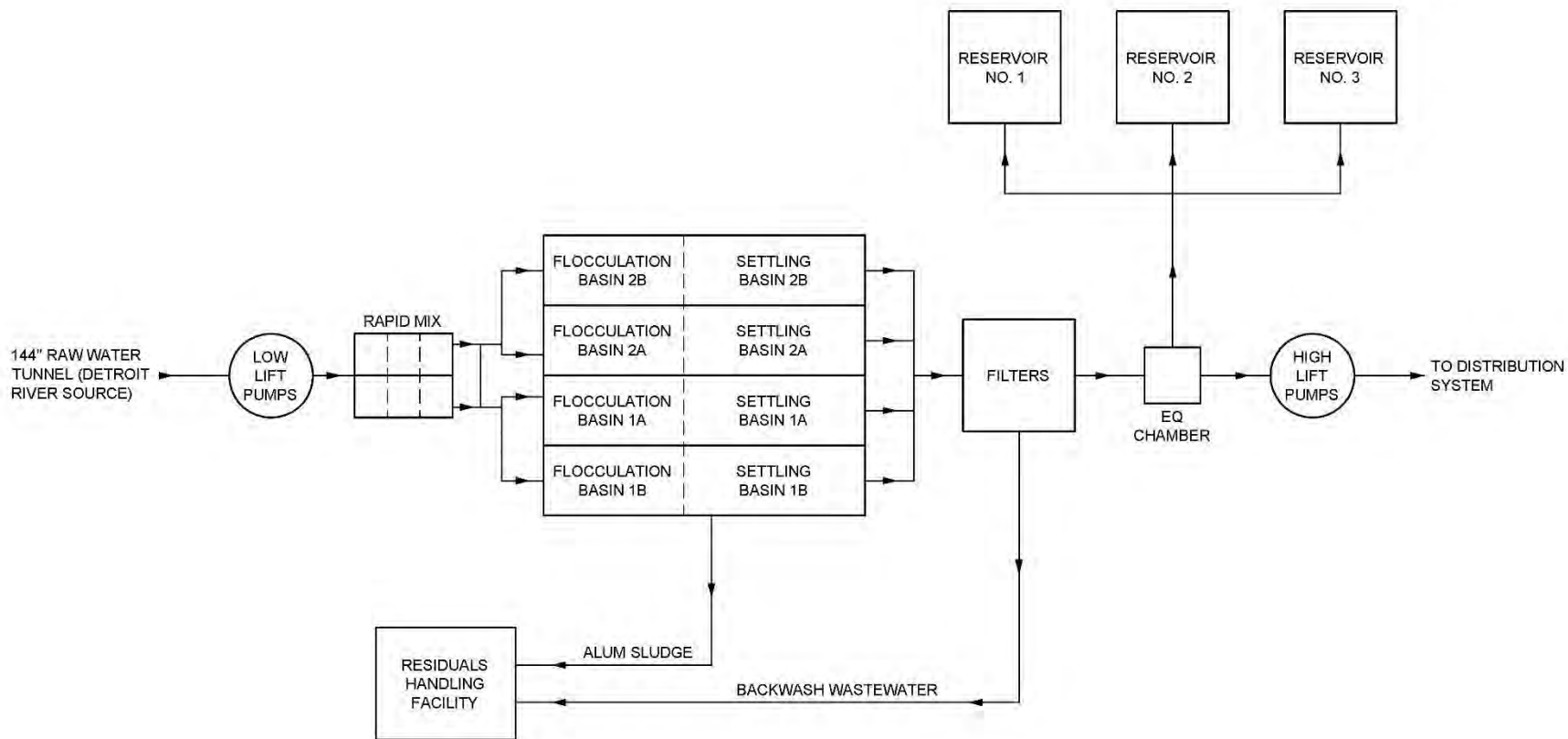


Figure 6-4: Southwest Water Treatment Plant

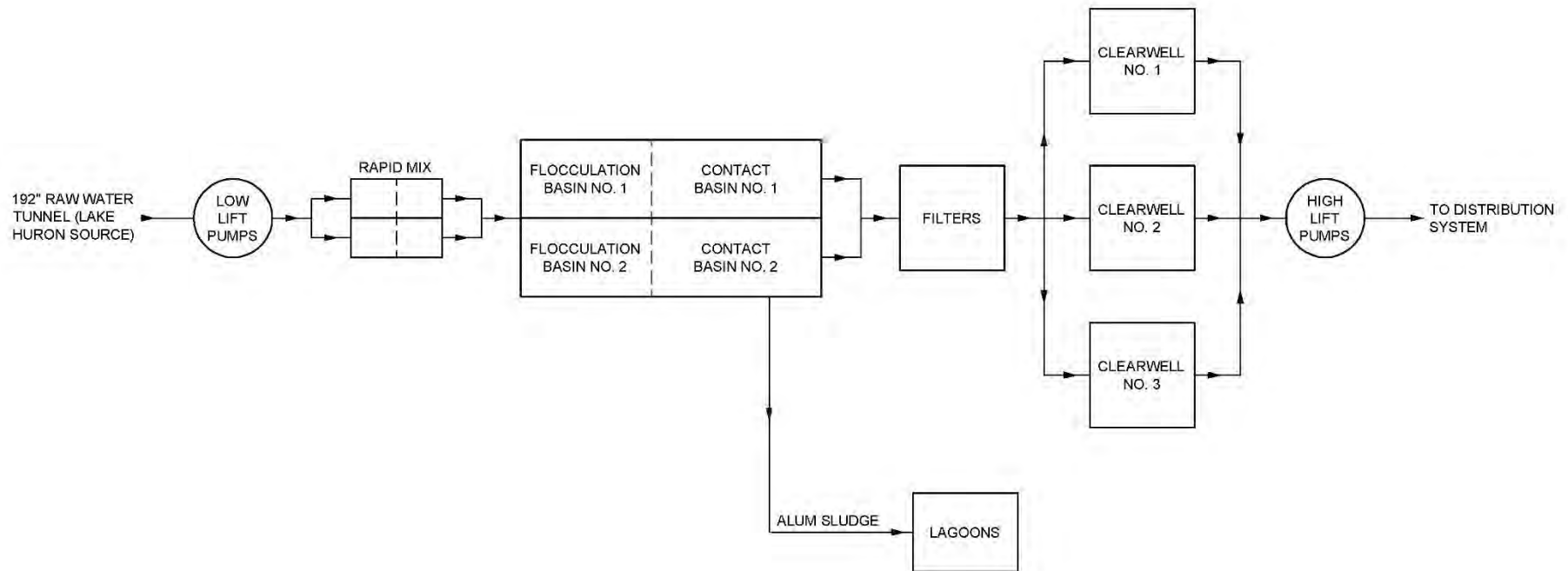


Figure 6-5: Lake Huron Water Treatment Plant

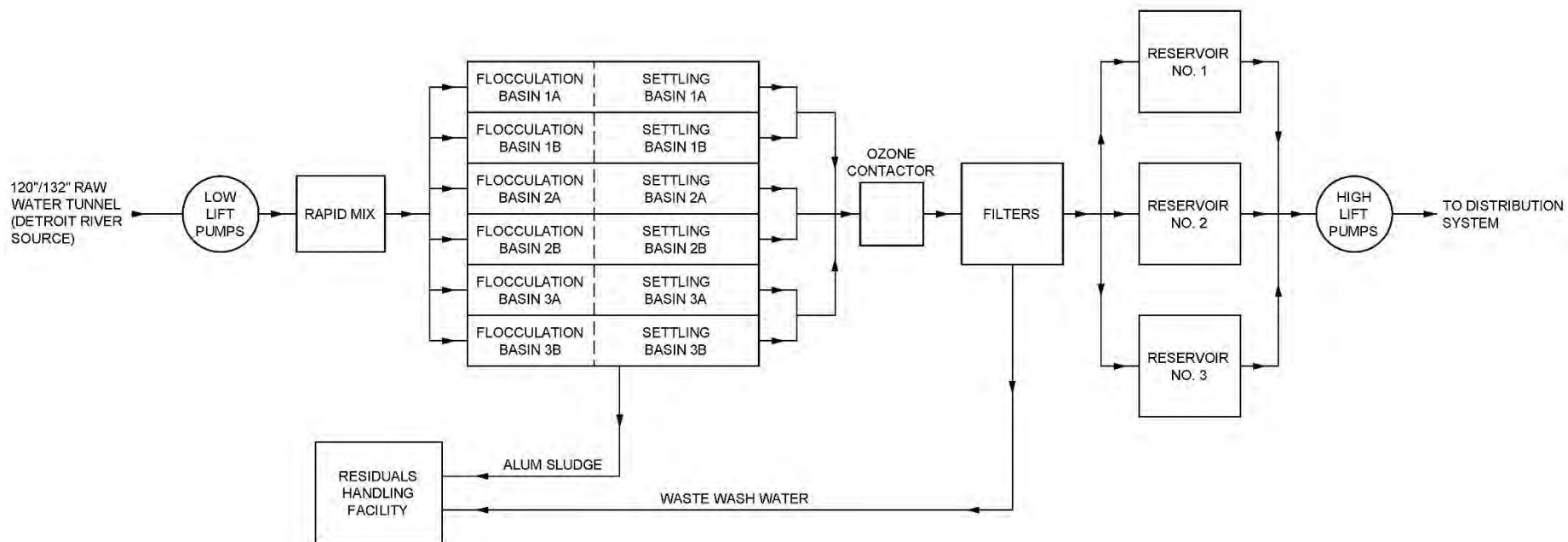


Figure 6-6: Water Works Park Water Treatment Plant

6.3 Needs Assessment

An assessment of the capital improvement needs for all five plants was performed. This work included a review and update of previous needs assessment projects in the years 2002 and 2003. At that time, DWSD performed a series of detailed inspections and engineering studies at its three oldest water treatment plants – Springwells, Northeast, and Southwest. The update was prepared in a series of five steps:

1. Detailed review of 2002 and 2003 Needs Assessment reports.
2. Identification of projects completed since 2003.
3. Interviews with DWSD engineering staff, plant operators and general plant inspections in September 2013.
4. Integration with the evaluation performed for the Independent Evaluation of DWSD's 10-Year Capital Improvement Program in September 2013.
5. Review of the current DWSD capital improvement plans for the period July 2014 to June 2020.

In updating the previous needs assessments, any previously proposed improvements to add treatment capacity were excluded from the forecast for this master plan. For example, in the previous 2002 Needs Assessment for the Northeast Water Treatment Plant, there was a recommendation to increase the plant capacity from 300 MGD to 340 MGD. Costs for this capacity expansion are not included in the updated needs assessment for this master plan.

Highlights of the updated needs assessment for each plant are presented below. Additional detail and project lists are provided in TM-13 in the Appendix. The proposed improvements address identified needs for mechanical equipment upgrades, regulatory requirements, safety and reliability, and efficiency in operation, and are intended to bring the plants to the levels of reliability and redundancy established in TM-4 Planning Criteria.

6.3.1 Springwells Water Treatment Plant Needs Assessment

The Needs Assessment for Springwells was completed in November of 2002. This report identified process, mechanical, electrical, instrumentation, HVAC, structural and architectural system deficiencies. All of these deficiencies were grouped into projects and a 10-year capital improvement plan was developed to address the necessary improvements. Some of the major process improvements identified were completed or are under construction at Springwells. The largest project is SPW 583, which is the rehabilitation of the 1958 filters.

6.3.2 Northeast Water Treatment Plant Needs Assessment

The Needs Assessment for the Northeast Water Treatment Plant was completed in April 2002. This project was conducted to identify process, mechanical, electrical, instrumentation, HVAC, structural and architectural deficiencies and develop a 10-year capital improvement program to address these needs. The needs assessment team found that, due to the length of service of much of the plant's equipment and the frequency of preventive and corrective maintenance activities, most of the process equipment was in need of replacement.

Most of the projects recommended have not been constructed and several design projects for recommended improvements are now on hold, pending the findings from this update of the water master plan. DWSD staff have performed very well to maintain high water quality at the Northeast plant without the previously recommended improvements. Recent and proposed short-term projects are designed to maintain minimum plant operations until the plant is repurposed.

6.3.3 Southwest Water Treatment Plant Needs Assessment

The Needs Assessment report for the Southwest Water Treatment Plant was completed in May of 2003. This report identified process, mechanical, electrical, instrumentation, HVAC, structural and architectural system deficiencies. All of these deficiencies were grouped into projects and a 10-year capital improvement plan was developed to address the necessary improvements. Several of the projects have been completed in the last ten years. During 2011 to 2013, capacity at the Southwest plant was limited to 120 MGD while project SW-548 (Sludge Treatment and Waste Wash Water Treatment Facilities) was being completed.

6.3.4 Lake Huron Water Treatment Plant

Capital improvements needs for the Lake Huron Water Treatment Plant were included in the Comprehensive Water Master Plan in 2004. Several projects were identified to increase the treatment capacity of Lake Huron to match future demand projections of the DWSD water distribution system in the northern and northwestern suburbs. Given decreasing water demands in recent years, only a number of smaller projects have been implemented.

6.3.5 Water Works Park Water Treatment Plant

The Water Works Park water treatment plant was new at the time that the 2004 Comprehensive Water Master Plan Team was prepared. Consequently, the 2004 master plan only recommended the addition of a new UV disinfection system. The yard piping improvements project for Water Works Park is included in DWSD's current capital improvement plan, and this is a time-critical project that must be completed before the Northeast plant can be decommissioned and repurposed. Other sizeable projects for Water Works Park include a new ozone generator, Ovation system upgrades, and replacement of certain mechanical equipment subject to high wear.

6.4 Discussion of Particular Types of Improvements

Three types of identified future needs have significant capital and O&M cost impact and are discussed below.

6.4.1 Residuals Handling

Mechanical systems to handle residuals in sedimentation basins have been installed in the Water Works Park and Southwest water treatment plants. DWSD is currently testing a new procedure to flush small quantities of sedimentation basin residuals to the wastewater collection system on a regular schedule. If this procedure is successful, then systems to handle residuals may be less costly at Northeast and Springwells. The cost for full mechanical residuals handling has been maintained in TM13, but ultimate implementation could be achieved at a lower cost than presented.

6.4.2 Advanced Disinfection

The 2002 and 2003 needs assessment reports and the 2004 Comprehensive Water Master Plan included the addition of ultraviolet light (UV) disinfection at all five water plants and the addition of ozone at the Southwest, Northeast, Lake Huron and Springwells plants. Ozone is currently installed at Water Works Park.

The addition of ozone and UV to enhance DWSD's ability to remove giardia and cryptosporidium was proposed in the 2002 and 2003 needs assessment reports. The US EPA Interim Enhanced Surface Water Treatment requires 2-log removal of *Cryptosporidium*. This is accomplished by filtration performance which is achievable by well operated water plants. Additional removal may be required depending on the concentrations of *Cryptosporidium* detected in the source water(s). If additional *Cryptosporidium* removal is required based upon sampling results, then a utility has multiple options for providing that removal, including but not limited to, ozone, UV, enhanced filtration, source water protection, and other approaches. Michigan utilities, including DWSD, are not required to achieve additional *Cryptosporidium* removal because of source water quality.

6.4.3 Chlorine Gas Conversion

In 2007-2008, MDEQ conducted a project to meet with all water and wastewater utilities that use gas chlorine in Michigan to discuss the potential to convert to non-gas alternative (such as hypochlorite). A few utilities, such as Wyoming, have since made the conversion. The MDEQ continues to advocate for reduction in gas chlorine use. If a capital project at a treatment plant impacts the disinfection system, that utility will be required to convert or to justify continued gas chlorine use. In addition, the US Congress has discussed including water and wastewater utilities in the CFATS (Chemical Facilities Anti-Terrorism Standard). After much deliberation, water and wastewater utilities have remained exempt, and this regulation is currently not being actively pursued. Both MDEQ and Congress justify the reduction in use of gas chlorine based on risks from both accidental releases and terrorism acts.

Several major water utilities in the United States and Canada are currently implementing projects to convert from chlorine gas to on-site generation of sodium hypochlorite. On-site generation of sodium hypochlorite for water treatment plants in the range of 180 MGD to 540 MGD would use technology to convert brine to a 12 percent solution of sodium hypochlorite. New facilities are needed for salt receiving, handling and storage; brine generation, heat exchangers, chillers, and water softening; storage tanks for brine and 12 percent sodium hypochlorite; and new equipment for adding and mixing sodium hypochlorite in the finished water. Based on the cost of installations in other water utilities, costs for DWSD's water plants could be \$20 to \$40 million per plant, or higher.

Chlorine gas conversion was not included in previous needs assessment reports. It has been included in life cycle cost analysis in TM-6 as part of the potential costs of regulatory compliance. Further discussion will be held with DWSD during development of the Service Management Plan in March 2015 to address chlorine gas conversion in long term capital planning.

6.4.4 Summary

Table 6-2 presents a summary of the needs assessment in TM-13. The cost estimates show needs based on current regulatory requirements to rehabilitate the plants to their current rated capacities. The work includes the installation of mechanical system for residuals as described above.

Table 6-2: Summary of Needs Assessment for 2015 to 2035 Based on Current Regulatory Requirements

Plant	Needs Based on Current Regulations
Lake Huron	\$ 176,800,000
Northeast	\$ 211,200,000
Water Works Park	\$ 82,800,000
Springwells	\$ 446,000,000
Southwest	\$ 64,200,000
Total	\$ 981,000,000

Notes:

See TM-6 and TM-13 for documentation of these numbers.

Values are rounded to nearest hundred thousand dollars.

Cost estimates are based on prevailing construction prices for March 2014.

6.5 Screening and Evaluation of Alternatives

A two-phase process was conducted to screen a range of alternatives for water treatment plant consolidation and repurposing. A detailed presentation of the screening and evaluation of alternatives is presented in TM-6 in the Appendix. Highlights are presented in this chapter.

In the first phase, eight alternatives were analyzed. Based on the conclusions from the screening process, four alternatives were given additional consideration. The process was conducted with a high degree of involvement from DWSD staff, wholesale customers, and the Board of Water Commissioners. Four meetings and three workshops devoted to this issue were held between March and August of 2014.

6.5.1 Screening of Alternatives

In the first phase, the following alternatives were analyzed:

1. Maintain all 5 plants at their current capacity
2. Repurpose the Northeast plant
3. Repurpose Southwest plant
4. Repurpose Northeast and Southwest plants
5. Repurpose the Springwells plant
6. Increase Capacity at Lake Huron

- 7. Reduce Capacity Across all plants
- 8. Repurpose Lake Huron plant

In this analysis “repurposing” a water treatment plant means maintaining the high lift pumping and reservoirs, and then building a new main or mains to supply water to the high lift pumping. The existing reservoirs will be supplied with water from other plants, and the existing high lift pumps will operate in manner similar to other booster pumping stations in the system.

An important factor to consider in planning a reduction in the number of water treatment plants is the limited number of days each year when the plants operate at or near their peak capacity. Currently, the five plants operate at less than 35% percent of design capacity for 9 months of the year. **Figure 6-7** compares rated water treatment plant capacity to projected water demand. Water treatment capacity is based on the maximum day demand. The maximum day demand, in turn, is based on the summer time outdoor irrigation pattern. The maximum day demand pattern is approximately twice the average day demand when outdoor irrigation is at its maximum. However, due to the variability of weather, outdoor irrigation demands fluctuate from year to year, and the design maximum day demand may be reached every 3 to 5 years. 2013 and 2014 were cool, wet summers, and the maximum day demands were 761 and 748 MGD, respectively. 2012 was exceptionally dry and warm, and the maximum day demand was 969 MGD. **Figure 6-7** shows capacity requirements in terms of days per year.

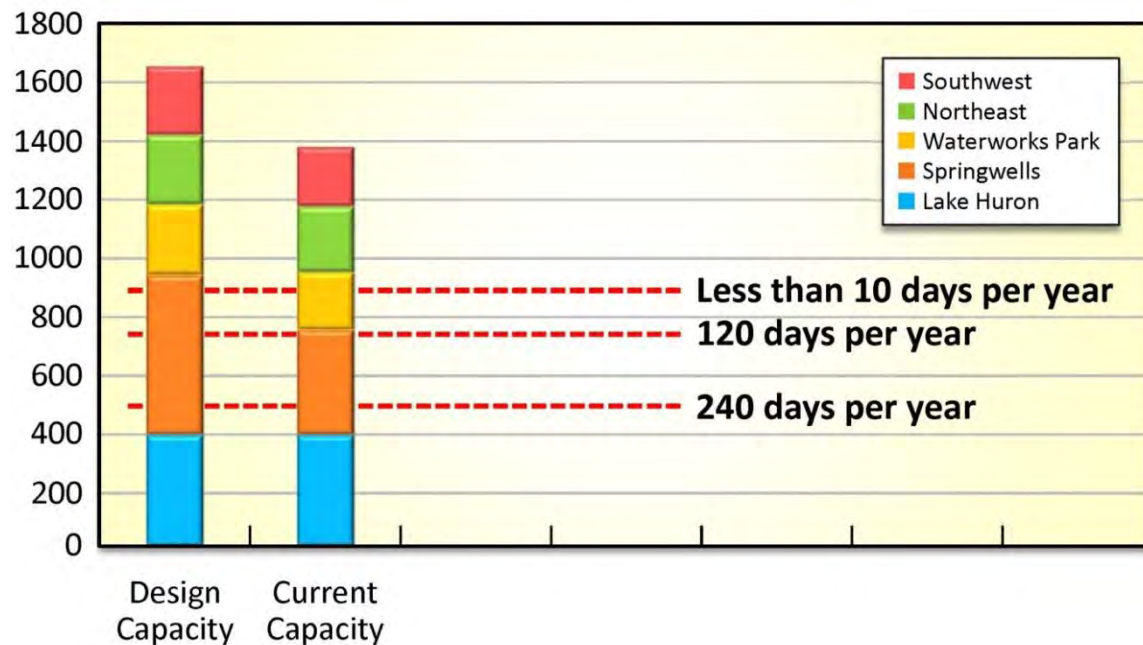


Figure 6-7: Comparison of Plant Capacity and Projected Water Demand (dotted red lines)

The alternatives are described below. All alternatives were defined so that they met the standards for redundancy established in TM-4. In order to meet these standards, any alternative that includes decommissioning a plant includes new transmission system improvements to allow for maximum day demand to be conveyed from other remaining plants.

1. Maintain all Plants at Current MDEQ Rated Capacity. This is the baseline alternative of continuing to upgrade and operate plants at current permitted capacity. In this alternative, all plants would be upgraded to the levels summarized in **Table 6-1** to meet current regulatory requirements. The Water Works Park Yard Piping Project, WW-536, would be completed so that the high lift pumps can move 240 MGD from the plant site. The work of upgrading the plants would be performed over the 20-year period 2015 to 2035. For purposes of general planning of the construction work, the following phases were established in **Table 6-3**.

Table 6-3: Construction Phasing Plan for Life Cycle Cost Analysis

Period	Type of Upgrade Activity	Approximate Percent of Total Construction Cost
2015-2019	Safety, Water Quality	10
2020-2024	Efficiency, Water Quality	25
2025-2029	Efficiency, Water Quality	30
2030-2034	Service Life Renewal	35

2. Repurpose the Northeast Water Treatment Plant. This is the same as the baseline alternative, except the Northeast plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Northeast reservoirs to be filled from the high lift pumps at Water Works Park and the North Service Center.
3. Repurpose the Southwest Water Treatment Plant. This is the same as the baseline alternative, except the Southwest plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Southwest reservoirs to be filled from the high lift pumps at Springwells.
4. Repurpose the Northeast and Southwest Water Treatment Plants. This is the same as the baseline alternative, except the Northeast and Southwest plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Northeast reservoirs to be filled from the high lift pumps at Water Works Park. North Service Center valve operations would be changed to allow more water from Lake Huron into the Northeast service area. Other improvements would allow the Southwest reservoirs to be filled from the high lift pumps at Springwells.
5. Repurpose the Springwells Water Treatment Plant. This is the same as the baseline alternative, except the Springwells plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission

system improvements would be made to allow the Springwells reservoirs to be filled from the high lift pumps at Water Works Park and Northeast.

6. Expand the Lake Huron Plant and Continue to use Water Works Park. This alternative would expand the Lake Huron plant to use direct filtration treatment technology. Direct filtration is a less costly treatment process, because it does not use sedimentation ahead of filtration. The excellent source water quality in Lake Huron makes this process theoretically feasible. Pilot testing would be needed to establish design criteria and obtain approval from MDEQ. The Water Works Park plant would be upgraded and operated at its current rated capacity. Other plants would be decommissioned. The high lift pumps and finished water reservoirs of the other plants would continue in operation, and would function like other booster pump stations in the transmission system. Significant transmission system improvements would be required to bring more water from the Lake Huron plant to the service area. In concept, this would require the second feed from the Lake Huron Plant to the North Service Center, plus additional transmission improvements south of that point.
7. Upgrade All Plants at a Reduced Rated Capacity. This alternative would maintain all plants, but the total capacity would be equal to the projected maximum day demand in 2035, roughly 1000 MGD. There are many ways to downsize the plants to equal 1000 MGD. The scenario shown in **Table 6-4** was used for the life-cycle cost evaluation:

Table 6-4: Scenario for Future Reduced Capacity

Plant	Scenario for Future Rated Capacity	Rationale
Water Works Park	240	New plant, maintain existing rated capacity
Northeast	150	High cost to upgrade, reduce capacity by half
Southwest	180	Reduce capacity 25 percent with one less flocculation/sedimentation train
Springwells	200	Capacity of 1958 plant, currently being upgraded
Lake Huron	300	Reduce because most water customers are far south of this plant
Total	1,070 ²	

Transmission improvements similar to those included in Alternative 5 would be required to assure that finished water could be routed among the Northeast, Southwest, Water Works Park and Springwells plants, and they could collectively meet maximum day demands.

8. Repurpose Lake Huron Water Treatment Plant. This alternative was suggested for consideration by one of DWSD's wholesale customers. It is the same as the baseline alternative, except the Lake Huron plant treatment facilities would be decommissioned early in the planning period. The reason for considering this is that the energy cost for pumping water are higher from Lake Huron than other plants, and the plant could be offered for sale to KWA. Due to the

² The capacity of 1,070 MGD used in the life-cycle cost evaluation was later refined to 1,040 MGD for the recommended plan. This footnote also applies to Table 6-5 and Table 6-6.

distance of this plant, the high lift pumps and finished water reservoirs would not continue in operation. Transmission system improvements would be made to provide a second transmission main from the North Service Center to customers in northern Macomb County and northern Oakland County as a redundant source of supply should a service disruption take place on the 96-inch transmission main.

TM-6 in the Appendix presents the details of the life cycle cost evaluation for the Screening of Alternatives. The following conclusions were drawn:

- It is cost effective to reduce the number of treatment plants in the future. In the face of stable water demands, DWSD can avoid capital costs by upgrading fewer plants.
- Given the potential for future regulatory requirements for ozone, ultraviolet disinfection, and chlorine gas conversion, the cost savings of decommissioning two plants could be 15 to 20 percent greater than shown in Table 6-2. Therefore, potential future regulatory requirements reinforce the cost-effectiveness of reducing the number of DWSD's water treatment facilities. DWSD would reduce fixed costs and achieve economies of scale by making regulatory-driven improvements at fewer facilities in the future.
- The alternative for direct filtration at Lake Huron has potential long term merit. This alternative could be more cost-effective in the future, depending on adding new customers in the northern part of the service area, and depending on future regulations.
- It is cost-effective to decommission the Northeast water treatment plant early in the planning period. During the 1970s and again in 1985, the Northeast plant was temporarily closed, and there is operational experience on filling its reservoirs from the transmission system. Transmission system upgrades totaling approximately \$68 million are required, but capital costs avoided by not upgrading the Northeast plant are \$211 million based on current regulations and potentially much higher if ozone, UV disinfection and chlorine gas conversion are required in the future.
- It is potentially cost effective to decommission the Southwest water treatment plant later in the planning period. Improvements currently underway at the Springwells 1958 plant need to be completed, and future improvements to assure the reliability of Springwells high lift and intermediate pumps need to be completed.
- Also, further evaluation of nonrevenue water and reduction in non-revenue water is recommended. Over \$60 million in new capital improvements have been made at the Southwest plant in the last 7 years. Delaying the closure of Southwest allows DWSD to gain more benefit from those investments while rehabilitation of Springwells continues. However, if a future decision or event triggers the need for new capital investment at Southwest, then that would be the time to consider decommissioning.
- Part of the ongoing upgrade project at Springwells includes limited upgrades to the Springwells 1930 filter plant. That work could determine that the cost to upgrade all of the 1930 filters is greater than what has been estimated in the updated needs assessment. If that is the case, then

DWSD could consider decommissioning the 1930 filter plant at Springwells, but maintain the 1958 plant and the Southwest plant.

6.5.2 Evaluation of Selected Alternatives

The preceding findings and conclusions were reviewed with DWSD and wholesale customers in a series of workshops from May to August 2014. Based on the results of the workshops, the following four alternatives were selected for further evaluation:

1. Reduce all plants to a total capacity of 1,070 MGD.
2. Repurpose the Northeast plant, and reduce others for a total of 1,070 MGD.
3. Repurpose the Northeast and Southwest plants, and reduce others for a total of 1,070 MGD.
4. Repurpose the Springwells plant, and reduce others for a total of 1,070 MGD.

In the evaluation of the selected four alternatives, some of the earlier ground rules used in the screening of alternatives were changed. The changes included the addition of the following costs to the evaluation:

1. Costs for complying with potential future drinking water regulations;
2. Costs for warehousing and inventory of water treatment plant spare parts, equipment and materials;
3. Annual costs for FY2014 were considered in addition to those previously used for FY2013; and
4. Costs to upgrade all critical inter-plant transmission mains and to maintain three water intakes.

TM-6 in the Appendix presents the details of the evaluation of selected alternatives. The non-monetary considerations for the evaluation are listed in **Table 6-7**. Highlights are summarized in this section. The four alternatives are described below.

1. Upgrade All Plants at a Reduced Rated Capacity. This alternative would maintain all plants, but the total capacity would be equal to the projected maximum day demand in 2035, roughly 1000 MGD. There are many ways to downsize the plants to equal 1000 MGD. Further hydraulic modeling and optimization of each operating zone to meet the future needs of the City of Detroit and wholesale customers will establish the planning period capacities for each plant. The following scenario was used for the economic evaluation for this Product Plan Interim Report:

Table 6-5: Scenario for Future Reduced Capacity for Selected Alternatives

Plant	Scenario for Future Rated Capacity	Rationale
Water Works Park	240	New plant, maintain existing rated capacity
Northeast	150	High cost to upgrade, reduce capacity by half
Southwest	180	Reduce capacity 25 percent with one less flocculation/sedimentation train
Springwells	200	Capacity of 1958 plant, currently being upgraded
Lake Huron	300	Reduce to the capacity of the 96-inch main from Imlay Station to North Service Center
Total	1,070 ¹	

2. Repurpose the Northeast Water Treatment Plant. This is similar to the baseline alternative, except the Northeast plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Northeast reservoirs to be filled from the high lift pumps at Water Works Park and the North Service Center.
3. Repurpose the Northeast and Southwest Water Treatment Plants. This is similar to the baseline alternative, except the Northeast and Southwest plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Northeast reservoirs to be filled from the high lift pumps at Water Works Park. North Service Center valve operations would be changed to allow more water from Lake Huron into the Northeast service area. Other improvements will allow the Southwest reservoirs to be filled from the high lift pumps at Springwells.
4. Repurpose the Springwells Water Treatment Plant. This is similar to baseline alternative, except the Springwells plant treatment facilities would be decommissioned early in the planning period. The high lift pumps and finished water reservoirs would continue in operation, and would function like other booster pump stations in the transmission system. Transmission system improvements would be made to allow the Springwells reservoirs to be filled from the high lift pumps at Water Works Park and Northeast.

Table 6-6 shows a summary of present worth and annual operating and maintenance costs for the four selected alternatives compared to the projected cost of maintaining past practice with operation of all five plants at their current rated capacity.

The least cost alternative remains repurposing Northeast and Southwest. However, the costs are very close, and an equally attractive alternative is to maintain Southwest but not upgrade Springwells beyond the capacity of the current filter upgrade project. Filters could be rerated from 200 to 240, DWSD could establish that new capacity for Springwells and avoid higher costs up upgrading the 1930 filter plant.

Table 6-6: Cost Comparison of Selected Alternatives to Past Practice Alternative¹

Alternative	Present Worth (\$ 1,000)	Annual O&M Cost (\$ 1,000)
Past Practice: Maintain All 5 Plants at Current Rated Capacity of 1,720 MGD	\$2,029,000	\$54,870
New Baseline: Maintain All 5 Plants, but Reduced Total Rated Capacity of 1,070 MGD	\$1,530,000	\$48,570
Repurpose the Northeast Plant and Reduce other Plants to a Total of 1,070 MGD	\$1,444,000	\$47,190
Repurpose the Northeast and Southwest Plants, Adjust Other Plants to a Total of 1,070 MGD	\$1,439,000	\$44,210
Repurpose the Springwells Plant, Adjust Other Plants to a Total of 1,070 MGD	\$1,488,000	\$49,420

6.6 Summary of Cost and Non-Monetary Factors

Table 6-7 presents a summary of capital and operating cost estimates and non-monetary factors for the selected alternatives and the past practice of maintaining five plants at their current rated capacity. The cost factors and the identified non-monetary factors in **Table 6-7** support a plan for reducing treatment plant capacity and for reducing the number of plants. There is sufficient justification for repurposing the Northeast plant to begin that process in FY2016. Further consideration of repurposing the Southwest plant should be made following the assessment of certain critical information that will be available by 2020: completion of work at the Springwells 1958 plant and the estimated cost of upgrading the Springwells 1930 filters, additional progress on non-revenue water reduction, and the results of source water assessment and surface water intake protection planning for all three intakes.

THIS PAGE INTENTIONALLY LEFT BLANK.

Table 6-7: Summary of Cost and Non-Monetary Factors

Alternative	Major Benefits	Major Disadvantages	Present Worth (\$ Thousands)	Annual O&M Cost (\$ Thousands)	Supply, Treatment and Transmission Redundancy	Short-Term CIP and Stranded Costs Issues
Past Practice: Maintain All 5 Plants at Current Rated Capacity of 1,720 MGD	Maintains ability to expand the service area and respond to potential higher than forecast population growth. Does not require change in existing water rate elevation and distance formula.	Highest cost; capacity greatly exceeds the known 20-year potential for new customers and population growth; maintaining 5 plants is counter to goals of the staff optimization effort	\$2,029,000	\$54,870	Three supply intakes; treatment redundancy exceeds MDEQ and Ten States standards; condition assessment and rehabilitation or replacement of critical transmission mains will be required over the planning period. 96-inch transmission main will need division valves to facilitate preventive maintenance (common to all alternatives)	Requires over \$700 million in expenditures for capital improvements to treatment plants in the first half of the planning period
New Baseline: Maintain All 5 Plants, but Reduced Total Rated Capacity of 1,000 MGD	Capacity is based on projected population forecasts; can accommodate expansion of service area for high probability new customers. Greatly reduces capital improvement costs compared to Past Practice.	Leaves five plants operating; exposes DWSD to higher costs for labor and future regulatory compliance; does not allow fulfillment of staff optimization goals; most plants will continue to operate at less than 40 percent capacity for most months of the year.	\$1,530,000	\$48,570	Three supply intakes; treatment capacity redundancy meets MDEQ and Ten States standards; condition assessment and rehabilitation or replacement of critical transmission mains will be required over the planning period.	Requires over \$400 million in expenditures for capital improvements to treatment plants in the first half of the planning period
Repurpose the Northeast Plant and Reduce other Plants to a Total of 1,000 MGD	Avoids short term cost of previously postponed plant upgrade estimated at over \$200 million; plant location has least impact on maintaining level of service to customers now supplied by this plant.	Requires immediate implementation of two capital projects totaling up to \$90 million that are required during the planning period, but must be done at the start of the planning period.	\$1,444,000	\$47,190	Three supply intakes; treatment capacity redundancy meets MDEQ and Ten States standards; condition assessment and rehabilitation or replacement of critical transmission mains will be required over the planning period.	Least impact on short-term CIP; relative to other repurposing alternatives, this alternative has the lowest capital cost for new inter-plant water transmission mains.
Repurpose the Northeast and Southwest Plants, Adjust Other Plants to a Total of 1,000 MGD	Minimizes the number of treatment locations, therefore reduces cost of future regulatory-driven improvements and annual cost for labor and preventive maintenance.	Southwest has had recent upgrades and it has its own intake; preserving three intakes requires additional capital cost; largest number of customers is affected by any potential change in elevation and distance formula for water rates.	\$1,439,000	\$44,210	Requires new raw water transmission main from Southwest to Springwells to maintain three supply intakes; treatment capacity redundancy meets MDEQ and Ten States standards; condition assessment and rehabilitation or replacement of critical transmission mains will be required over the planning period.	Southwest plant has had over \$60 million in residuals management facilities since 2004; these new facilities have remaining service life.
Repurpose the Springwells Plant, Adjust Other Plants to a Total of 1,000 MGD	Springwells WTP has highest capital cost, if the full plant capacity is needed. By repurposing, the potential complexities and cost of construction can be avoided.	Existing water supply tunnel and plant is at a strategic point for service to western suburbs; extensive new transmission mains required to replace the existing supply.	\$1,488,000	\$49,420	Three supply intakes; treatment capacity redundancy meets MDEQ and Ten States standards; condition assessment and rehabilitation or replacement of critical transmission mains will be required over the planning period; major new transmission mains are required to supply finished water to the Springwells high and intermediate lift stations.	Ongoing construction for project SP 563. 1958 Filter Rehabilitation, would have stranded costs of \$87 million.

THIS PAGE INTENTIONALLY LEFT BLANK.

6.7 Proposed Treatment Programs

Previous sections of this chapter and report have established the basic needs for treatment capacity, operational efficiency, and regulatory compliance. This section identifies the major programs required to meet these needs. In the context of this report, a “program” is a set of CIP projects and annual O&M initiatives that collectively achieve an objective of this master plan. Four programs are identified for treatment:

- Program to Reduce Treatment Capacity
- Program to Repurpose the Northeast Plant
- Program for Regulatory Compliance
- Program for Renewal, Reliability and Energy Management

Accompanying this master plan report is a 20-year Capital Improvement Plan (CIP) spreadsheet that lists approximately 350 CIP projects. These projects are organized by program, including the four programs listed above for treatment, plus other programs for transmission, distribution, and metering and non-revenue water.

6.8 Program to Reduce Treatment Plant Capacity

The estimated maximum day treatment capacity for 2035 is estimated to be 1,040 MGD. This value is based on subsequent refinement of 2035 water demands and hydraulic modeling subsequent to the life-cycle cost evaluation. This number includes an allowance for emergency service for Flint and Genesee County. There is no allowance for new customers joining the service area, but there is projected growth in demand of existing customers. Trends in water use should be tracked annually and assessed every 5 years, along with requests for service from new wholesale customers. The projected 1,040 MGD maximum day capacity total should be reviewed again in the year 2020.

Table 6-8 presents the current rated capacities and optimized future capacities for treatment and high lift pumping at each treatment plant.

Table 6-8: Future Optimized Treatment Capacity and High Lift Pumping Capacity

Plant	Existing Rated Treatment Capacity (MGD)	Existing High Lift Pumping Capacity (MGD, Firm)	April 2015 Operational Capacity (MGD)	Future Optimized Treatment Capacity (MGD)	Future Optimized Pumping Capacity (MGD, Firm)
Lake Huron	400	420	340	320	360
Northeast	300	556	240	0	260
Springwells	540	150 intermediate 520 high	350	360	100 intermediate 350 high
Southwest	240	250	240	120	165
Water Works Park	240	560	80	240	240
Total	1,720	2,456	1,250*	1,040	1,475

*Note: Operational capacity is determined by either construction, treatment or transmission constraints in a given month.

DWSD should initiate actions in 2015 to achieve the optimized plant and pump station capacities by the year 2020. This schedule is based on the completion dates of the 1958 filter rehabilitation project at Springwells and the anticipated duration of new treatment process and production metering projects at other plants. A description of the actions required at each plant and pump station is presented below.

6.8.1 Lake Huron Treatment

Substantial upgrades are planned in DWSD's current CIP for FY2016 to FY2020. Ongoing construction there could limit operational capacity. In the direct filtration mode of operation, all filters will be required, but the rapid mix, flocculation and contact basins would be modified or eliminated from the process. The piloting work for direct filtration is scheduled to start in 2025. Until that time, any improvements for the rapid mix, flocculation, and contact basin should be made on the basis of a service life of 10 years or less.

6.8.2 Lake Huron High Lift

The existing firm capacity of the Lake Huron high lift pumping station is 420 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are eight pumps rated at 60 MGD each. The future optimized firm capacity is 360 MGD. The reduction in firm capacity can be achieved by removing from service one pump when the pump or motor require major repair.

6.8.3 Northeast Treatment

This plant is proposed for repurposing; its high lift pumping and reservoir functions would continue with finished water supplied from Water Works Park. A schedule and set of projects for repurposing is presented later in this section. Repurposing can be achieved by 2020 or earlier. Between now and the time of repurposing, the operating capacity of the Northeast WTP could be reduced to 225 MGD. This reduction would be accomplished by a shut-down of one of the 4 parallel flocculation and filtration process trains. As other prerequisite transmission projects for repurposing are completed, the operating capacity at Northeast could be reduced to 150 MGD with a shut-down of two process trains.

6.8.4 Northeast High Lift

The existing firm capacity of the Northeast high lift pumping station is 556 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are twelve pumps rated at capacities of 49 to 52 MGD each. The future optimized firm capacity is approximately 260 MGD. The reduction in firm capacity can be achieved by removing from service up to six pumps when the pumps or motors require major repair.

6.8.5 Springwells Treatment

Due to reconstruction of the 1958 filters, today's operational capacity at Springwells is close to the future optimized capacity. The reconstruction of the 1958 filters is scheduled to be completed by 2018. Some 1930 filters were rehabilitated during this project, and the 1930 filters were found to be in good condition, or were cost-effectively reconstructed. Another review should be conducted in 2020 regarding the repurposing the Southwest plant. If the decision at that time is to maintain the Southwest plant, then DWSD could replace additional 1930 filters and perform other upgrades at

Springwells to reach a rated capacity of 360 MGD. If it is decided to repurpose the Southwest plant, then upgrades to Springwells would need a total capacity of 480 MGD. In this situation, a study should be performed to determine the cost-effectiveness of a new 240 MGD plant at the Springwells location to supplement the rehabilitated 1958 plant, relative to the cost of rehabilitating the 1930 plant.

6.8.6 Springwells High Lift

The existing firm capacity of the Springwells high lift pumping station is 520 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are eleven pumps: four with capacity of 40 MGD, one with a capacity of 50 MGD, and six with a capacity of 60 MGD. The future optimized firm capacity is approximately 350 MGD. The reduction in firm capacity can be achieved by removing from service three or four pumps when the pumps or motors require major repair.

6.8.7 Springwells Intermediate Lift

The existing firm capacity of the Springwells intermediate lift pumping station is 150 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are four pumps: three with capacity of 50 MGD and one with a capacity of 60 MGD. The future optimized firm capacity is approximately 100 MGD. The reduction in firm capacity can be achieved by removing from service one pump when the pump or motor requires major repair.

Within the last few years, minimum nightly demand at the Springwells intermediate lift station has dropped below the capacity of the smallest pump, and significant throttling of pumps is required at night. New smaller pumps are required for minimum flows of approximately 20 MGD. Preliminary analysis performed as part of this master plan indicates that two 20 MGD pumps could be installed within the space of the one larger pump that is no longer needed. Alternatively, VFDs could be considered on the existing pumps.

6.8.8 Southwest Treatment

The optimized future capacity for the Southwest water treatment plant is 120 MGD, or one-half of its current rated capacity. The optimized capacity can be achieved by only scheduling new improvements for two of the four parallel process trains. After review of the repurposing in 2020, which would largely hinge on an updated forecast of new customers who seek to join the service area, then two process trains could be shut down.

6.8.9 Southwest High Lift

The existing firm capacity of the Southwest high lift pumping station is 250 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are seven pumps: four with capacity of 55 MGD, one with a capacity of 25 MGD, and two with a capacity of 30 MGD. The future optimized firm capacity is approximately 165 MGD. The reduction in firm capacity can be achieved by removing from service one 55 MGD pump and one 30 MGD pump when the pumps or motors require major repair.

6.8.10 Water Works Park Treatment

The optimized future capacity for the Water Works Park water treatment plant is 240 MGD, which is its current rated capacity. The optimized capacity can be achieved by eliminating the restrictions in the transmission mains from the plant, and this is discussed later in the section on repurposing.

6.8.11 Water Works Park High Lift

The existing firm capacity of the Water Works Park high lift pumping station is 560 MGD. Planning criteria for this Master Plan Update have established firm capacity (largest unit out of service) as the basis for capacity measure. There are eleven pumps: three with capacity of 80 MGD, four with a capacity of 60 MGD, and four with a capacity of 40 MGD. The future optimized firm capacity is approximately 280 MGD. The reduction in firm capacity can be achieved by removing from service the three 80 MGD pumps and one of the 60 MGD pumps when the pumps or motors require major repair.

Within the last few years, minimum nightly demand at the Water Works Park high lift station pumping has dropped below the capacity of the smallest pump, and significant throttling of pumps is required at night. New pumping units are required for minimum flows of 20 to 60 MGD. Preliminary analysis performed for this master plan indicates that two existing 40 MGD pumps could be replaced with two 20 MGD pumps and one variable frequency drive.

6.8.12 Summary of Plant and Pump Capacity Reduction

The proposed capacity reduction program will reduce treatment plant capacity by approximately 700 MGD and high lift pumping capacity by approximately 1,000 MGD. DWSD will benefit in the short term from reduced costs for preventive maintenance and potential for reduced electrical demand charges, and in the long term from reduced capital improvement costs. Implementation of the program for reducing plant capacity is expected to be carried out through a combination of funding from: a) the O&M budget; b) Small Capital Projects funding; and c) new CIP projects to replacing pumps at Water Works Park and Springwells to cost-effectively supply water during minimum demand periods.

6.9 Program to Repurpose the Northeast Water Treatment Plant

Changing the treatment and transmission system is complex and needs to be performed in a carefully planned and sequenced approach.

The capital improvements required for repurposing the Northeast plant include the construction to increase the capacity for finished water transmission main from Water Works Park to the Northeast reservoir tanks. Also required is a change in valve positions at the North Service Center so that the service area of the Lake Huron water treatment plant can be extended to the south. The capital improvements and operational changes are described below.

6.9.1 Alternatives for Transmission from Water Works Park to the Northeast Plant

Three alternatives to convey finished water from Water Works Park the Northeast plant have been identified. These alternatives vary in their transmission capacity and cost. This section describes the alternatives outlines options, data collection underway, and a process to make a decision on one alternative and start design first quarter of 2016.

6.9.2 Use the Existing Garland Main with New State Fair Valve

The Garland main is generally 48-inch in diameter and constructed of riveted steel pipe. It was installed between 1918 and 1921. A portion of the main (approximately two miles) from Concord and 6 Mile Road to the Northeast WTP is 54-inch steel pipe installed in 1946. This main has been reliable. There has only been one break reported on the main within the last eight years and that occurred in 2009. Based on modeling analysis, the average day demand through the main is approximately 12 MGD with an average of 21 MGD during a maximum day demand. The maximum capacity of the main, based on hydraulic modeling, is estimated to be 58 MGD.

Since the Garland main would be a critical pipe for the Northeast WTP repurposing scenario, it is recommended that an assessment of this pipe be performed prior to implementing this option. This could include performing a leak detection survey using acoustical sounding equipment or helium injection as well as removing small sections of the pipe to investigate the structural integrity. The estimated cost of the condition assessment is \$2,700,000.

Water would also be supplied from Springwells. Records show a PRV with short connection pipe of 36-inch diameter located at the intersection of 8 Mile Road and Marx Road, one block west of Dequindre Road. That PRV is used to control the water flow between Northeast and Springwells districts. In order to allow Springwells to supply water through this PRV, it will be necessary to increase the diameter of the 36-inch short connector and replace with a new pipe 54-inch diameter, the same size as its west or east connection pipes along 8 Mile Rd. The estimated cost is \$2,500,000. A new emergency plan for valve openings would need to be prepared.

The existing State Fair Valve, located immediately to the west of the Northeast water treatment plant has a capacity for 17.5 MGD. A new valve with capacity for approximately 60 MGD is required, along with associate yard piping improvements at the Northeast plant. The State Fair Valve and Northeast yard piping project is DWSD's approved CIP for FY2016 with a cost of \$800,000.

Figures 6-8 to 6-10 show additional transmission system improvements that are prerequisite to repurposing the Northeast Water Treatment Plant. **Figures 6-8 and 6-9** depict two options for the State Fair Valve and other yard piping improvements at the Northeast plant. One option requires improvement to the State Fair Valve, the other option relies on existing yard piping. The option with the State Fair Valve provides more flexibility for filling the reservoirs from the Water Works Park high lift station. **Figure 6-10** shows proposed changes to existing division valves at the North Service Center.

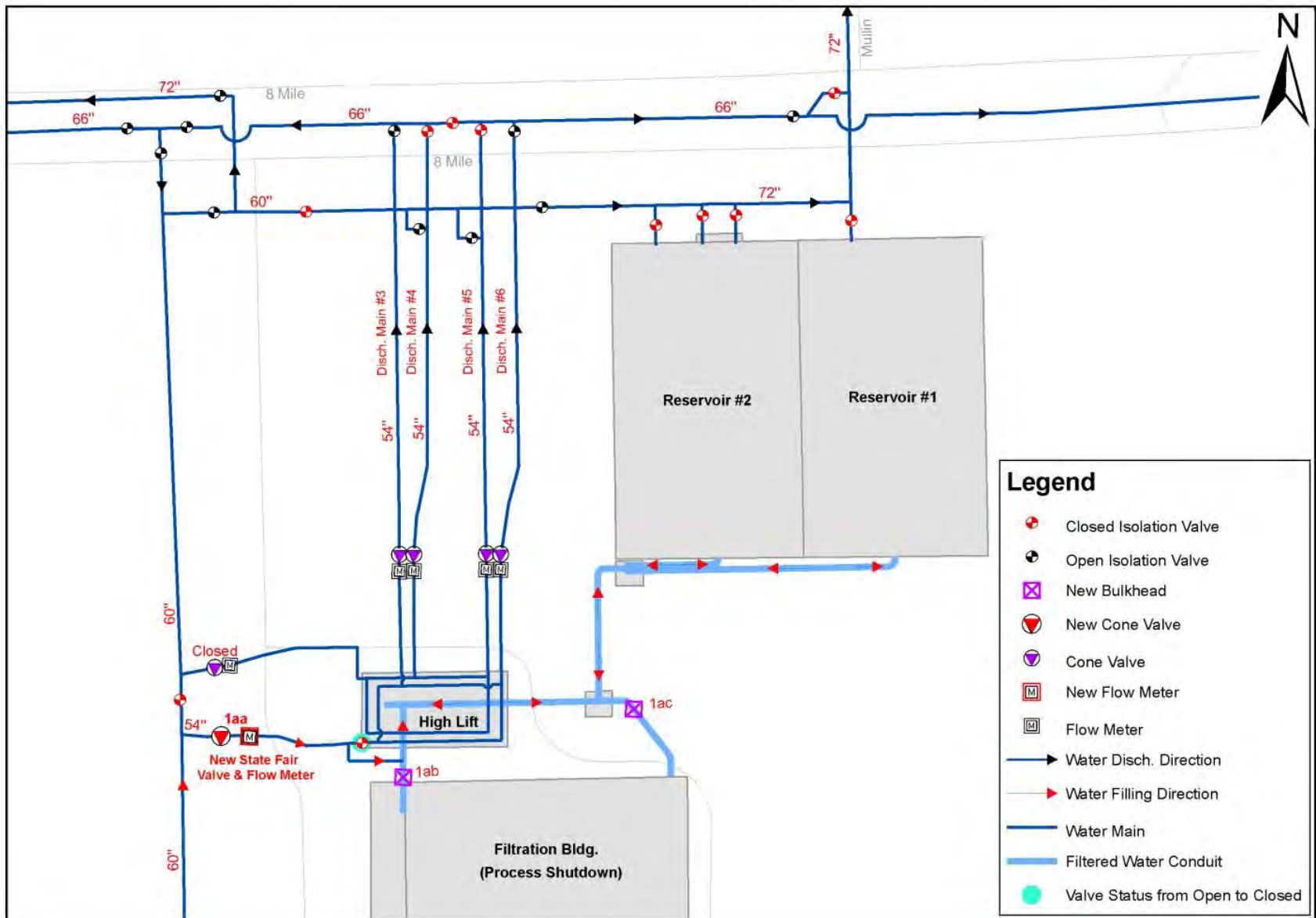


Figure 6-8: Northeast WTP Treatment Closure – Required Yard Piping Improvements, Option No. 1

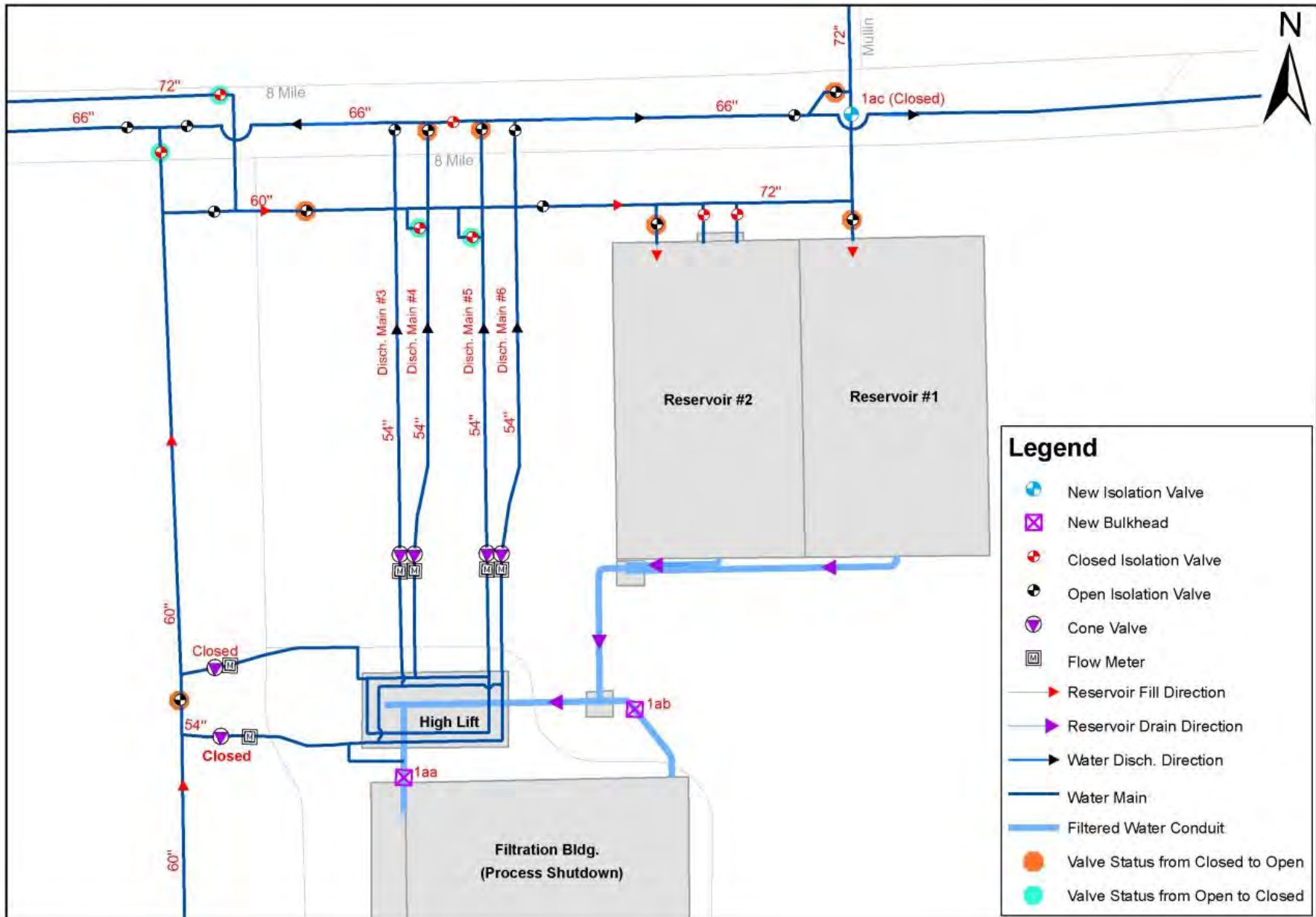


Figure 6-9: Northeast WTP Treatment Closure – Required Yard Piping Improvements, Option No. 2

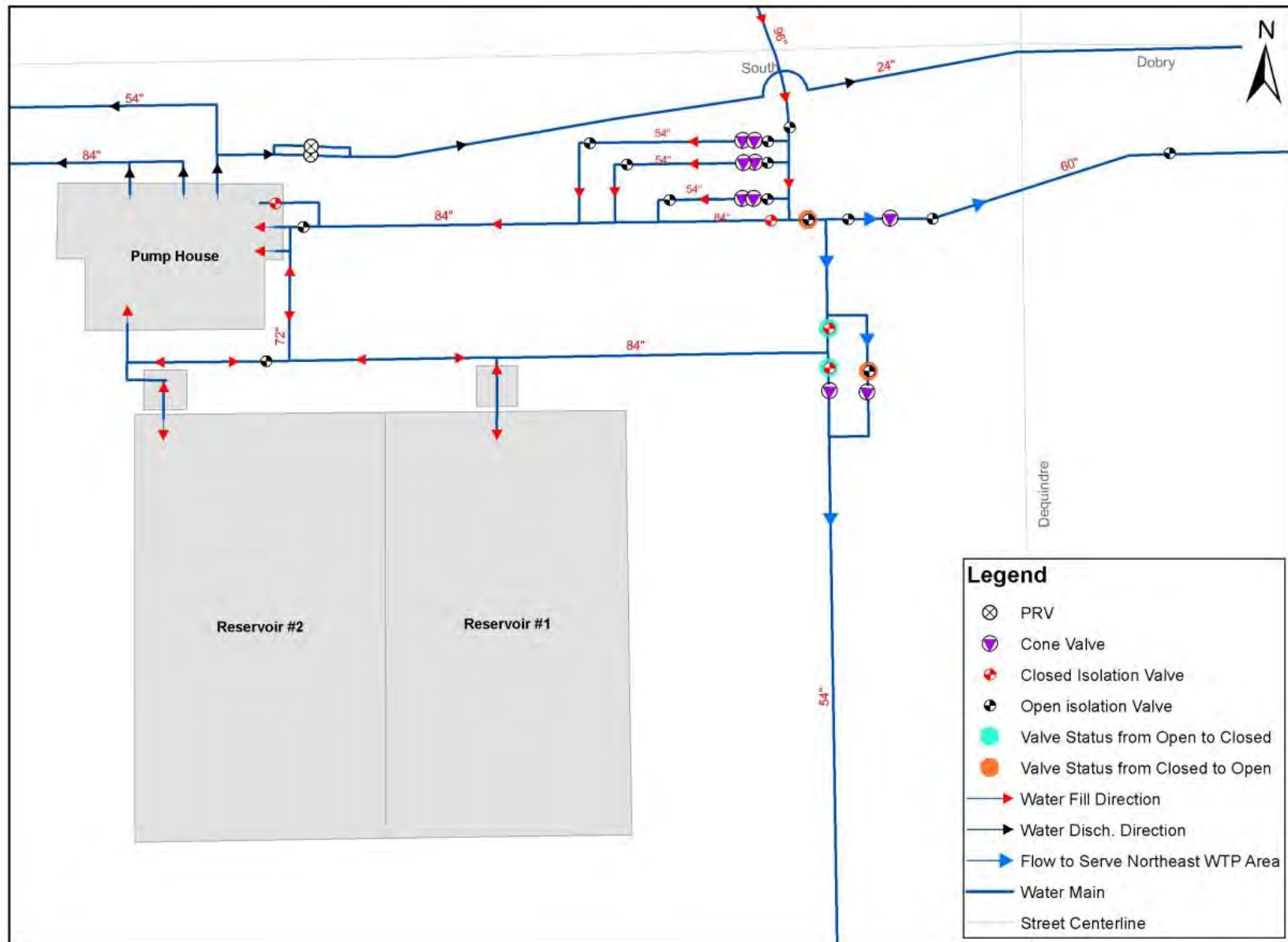


Figure 6-10: North Service Center – Required Isolation Valve Status Change

Project WW-536, Yard Piping Improvements at Water Works Park would need to be completed, in whole or in part, to allow 58 MGD to be conveyed to the Garland Main. The estimated cost for that project is \$35,000,000. This project is discussed in more detail in Section 6.10.7. The total estimated cost for this alternative is \$41,000,000 and the capacity is 58 MGD, assuming that the condition assessment shows that 58 MGD flow can be delivered reliably.

6.9.3 Rebuild Garland Main with New 60-inch Pipe

This alternative is based on the possibility that the condition assessment of the Garland Main would find the main in need of rehabilitation or replacement. Hydraulic modeling shows that a new 60-inch main would have a capacity of approximately 80 MGD. The new main would be approximately 43,500 feet long and would follow the route of the existing Garland Main. The construction cost of this new main is estimated to be \$53,000,000.

The total estimated cost for this alternative includes the condition assessment and the State Fair Valve replacement noted above, which yields a total of \$56,500,000, and the capacity would be 80 MGD.

6.9.4 Construct New 84-Inch Main in the Existing Raw Water Tunnel

This alternative provides for a larger main to be built by lining the existing raw water tunnel for the Northeast plant.

The existing Northeast raw water tunnel was constructed in the early 1950's. It was constructed as a branch from the tunnel that connects Water Works Park to the Springwells plant. The Northeast raw water tunnel has a finished interior diameter of 10-feet. It is constructed with a primary liner of circular concrete masonry units and a secondary liner of unreinforced concrete. In 1985, ground subsidence on 7-Mile Road lead to an investigation and repairs to 250 to 500 feet of the tunnel with new reinforced concrete.

The primary components of this alternative include:

1. Revision to the Water Works Park high lift station to optimize pumping capacity for conveying finished water to Northeast. Revision may include replacement of select high lift pumping units dedicated to this conveyance alternative if existing pumps cannot be modified to accommodate new system curve envelope.
2. New 84-inch diameter pipe from the Water Works Park high lift station generally along Pennsylvania Avenue to the existing junction chamber at Forest and Pennsylvania.
3. New 84-inch diameter liner pipe installed within the existing raw water tunnel from the junction chamber to the Northeast low lift station.
4. New 84-inch diameter pipe through the yard of the Northeast plant from the low lift station to the reservoirs.
5. New pressure sustaining valve system to maintain minimum of 35 psi in new finished water pipeline. [Further system analysis required to confirm.]
6. New connections to the Northeast reservoirs and addition of baffles in the reservoirs.

7. This alternative would have capacity for approximately 150 MGD finished water transmission. The finished water would be supplied to the Northeast reservoirs at a relatively even base flow rate of 100 MGD and a minimum pressure of 35-psi in the direct bury portion of route.

Construction of this alternative is anticipated to be as follows:

1. Revision to the Water Works Park high lift station to optimize pumping capacity for conveying finished water to Northeast would include new pumps, or possibly reconditioning of existing suitable pumps from Water Works Park or another booster pump station, plus new pump motors.
2. New 72-inch diameter pipe from the Water Works Park high lift station generally along Pennsylvania Avenue to the existing junction chamber at Forest and Pennsylvania would be approximately 10,000 feet in length. It would be constructed by traditional cut and cover excavation, and either steel pipe or PCCP pipe would be used.
3. New 84-inch diameter liner pipe installed within the existing raw water tunnel from the junction chamber to the Northeast low lift station would be approximately 26,800 feet in length. The tunnel would be dewatered for construction of the new liner pipe. In 2015, a new project has been started to inspect this tunnel and all other raw water tunnels and intakes. Results from this inspection would be used to confirm the feasibility, construction techniques, and cost estimate for this alternative. The new tunnel liner pipe would be installed through two or more access shafts into the tunnel. The annular space between the new pipe and the existing tunnel would be filled with grout.
4. New 72-inch diameter pipe through the low lift pump station and grounds of the Northeast plant from the low lift station to the reservoirs would be roughly 2,200 feet in length. It would be constructed by traditional cut and cover excavation.
5. New pressure sustaining valve system within an existing building of the Northeast plant to maintain minimum of 35 psi in new finished water pipeline. If a suitable building for this application is not available along pipeline route a new building would be constructed.
6. DWSD is planning improvements within the Northeast reservoirs to improve water circulation and avoid short-circuiting. Design of those improvements would be coordinated with the design of the new 72-inch feed to the reservoirs.

Figure 6-11 shows a plan view of this alternative and **Figure 6-12** shows a profile view. The estimated construction cost of this alternative is \$85,000,000, and the capacity is 150 MGD.

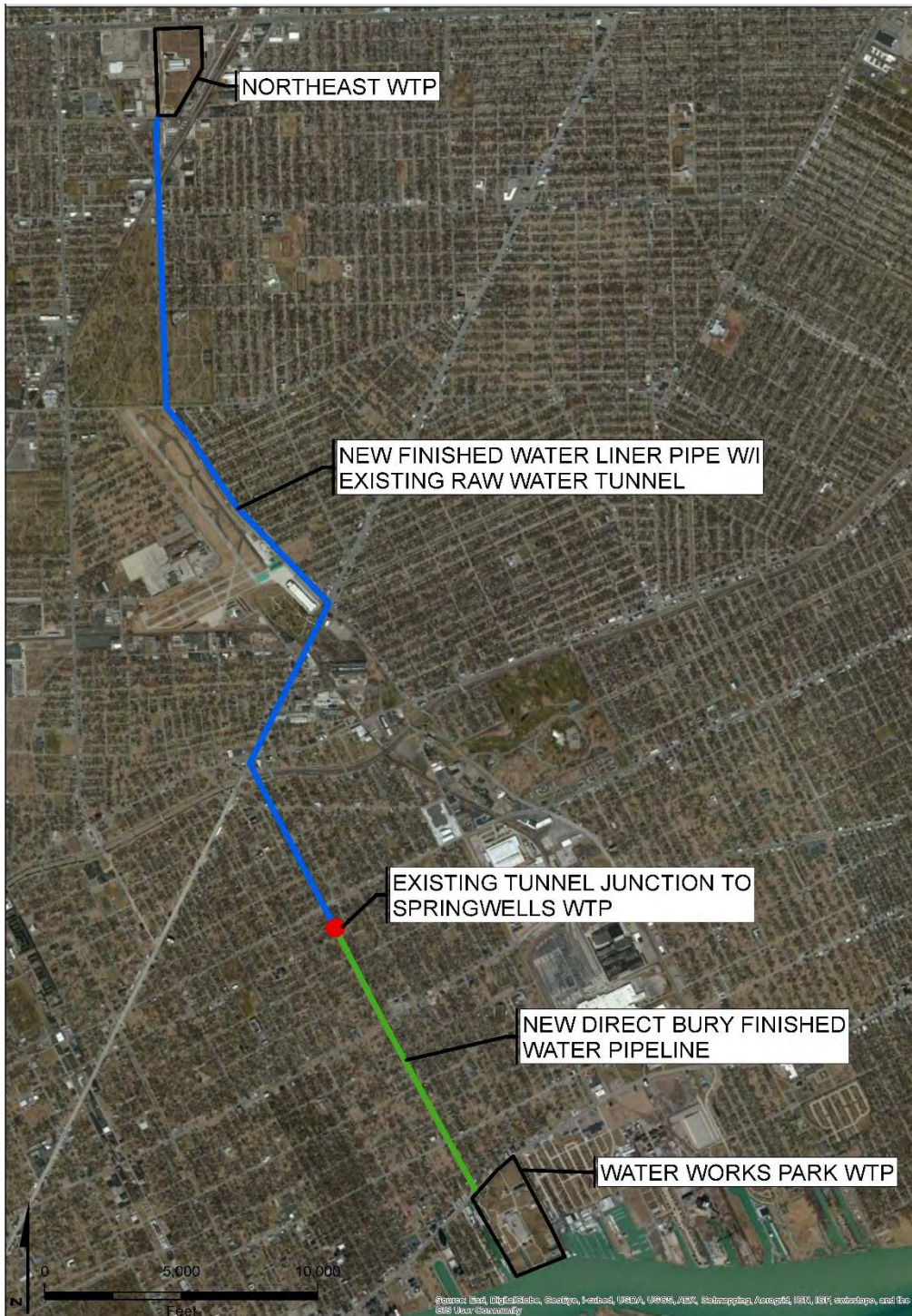


Figure 6-11: Plan View of Alternative for Re-Use of Northeast Raw Water Tunnel

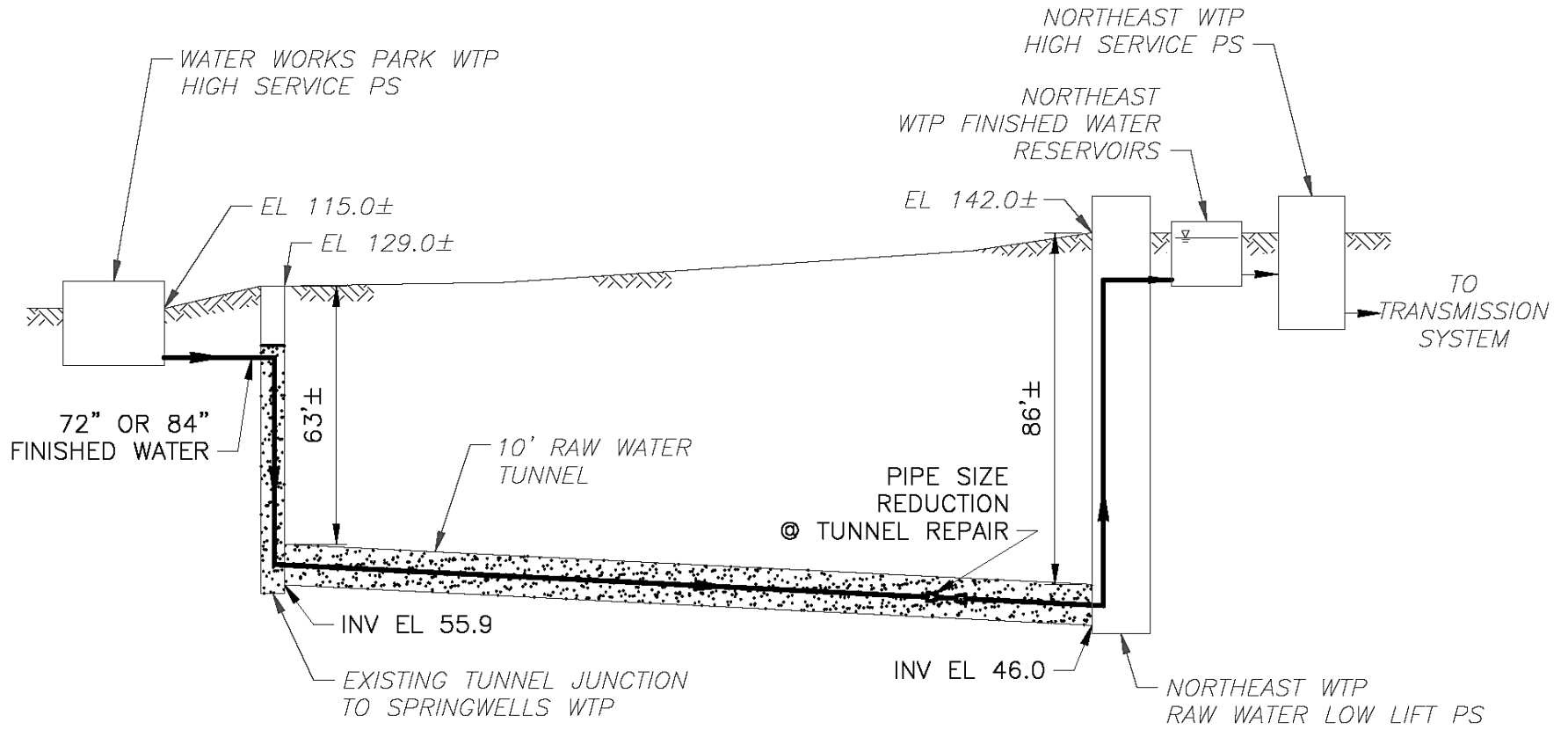


Figure 6-12: Profile View of Re-Use of Northeast Raw Water Tunnel

6.9.5 Comparison of Alternatives.

Table 6-9 presents a comparison of the alternatives.

Reuse of the existing Garland main is not seen as a long term alternative, given the age and diameter. The existing Garland main could be used for a short duration during construction of the tunnel reuse alternative.

The advantages of the alternative for lining the tunnel relative to the replacement of the Garland Main are as follows:

1. Re-use of the tunnel provides a higher capacity than re-use of the existing Garland Main alternative.
2. In the emergency situation of a break on the 96-inch main near Romeo, when supply from Lake Huron is cut-off, there would be sufficient water supply from Water Works Park to serve average day demands for all customers south of Romeo.
3. The previously designed Yard Piping and metering project for Water Works Park could be reduced in scope. The Venturi meters at the Northeast plant are being rehabilitated, so those meters can be used for the 150 MGD of finished water from Water Works Park. Additional meters at Water Works would only need 90 to 120 MGD capacity. Also, fewer new transmission mains in the Water Works Park yard would need to be replaced.
4. The future cost of upgrades to the Water Works Park high lift station would be reduced, because 150 MGD capacity would be devoted to supplying Northeast at a relatively low pressure, while 90 MGD to 120 MGD capacity would be devoted to the Water Works Park City of Detroit service area.
5. The alternatives provide a range of transmission capacity from 50 MGD to 150 MGD. A capacity greater than 150 MGD is not feasible because 90 MGD of the Water Works Park plant capacity is required for maximum day in the plant's current service area.

The 150 MGD capacity alternative has the following advantages:

- a. It is less costly to pump finished water from Water Works Park than from Lake Huron due to the shorter distance, a lower static head, and the need for Lake Huron to use seasonal intermediate pumping at Imlay City. On an annual average basis, the cost to pump from Lake Huron is approximately \$117/million gallons, while the cost to pump from Water Works Park is approximately \$40/million gallons.
- b. The estimated annual cost savings to pump 100 MGD from Water Works Park instead of Lake Huron is approximately \$ 3 million. Due to this cost savings, the payback period is less than 12 years for the additional cost to install a pipeline and pumping for 150 MGD relative to the cost to install a system for 60 MGD.

- c. With 150 MGD transmission capacity from Water Works Park to the Northeast High Lift Station, the following emergency capacity will exist:
 - Capacity to supply Oakland, Macomb, and Lapeer County customers with average day demand if there is break on the 120-inch main
 - Capacity to supply Oakland and Macomb customers with average day demand if there is a break on the 96-inch main.”

Table 6-9: Comparison of Alternatives for Transmission from WWP to Northeast

Alternative	Capacity	Requirements	Water Works Park Yard Piping Project	Estimated Construction Cost	\$/MGD Capacity to Supply Northeast High Lift
Use Existing 48-inch Garland Main	58	Condition Assessment, Improvement to State Fair Valve	Required	\$41,000,000	\$0.71/MGD
Rebuild Garland Main with New 60-inch pipe	80	Condition Assessment, Improvement to State Fair Valve	Required	\$56,500,000	\$0.71/MGD
Build new 72 to 84-inch main within existing Northeast raw water tunnel	150	Condition assessment of Garland main and raw water tunnel; Improvement to State Fair Valve (required for the short-term use of the Garland main during construction in the raw water tunnel)	Partial required	\$85,000,000	\$0.57/MGD

6.9.6 Feasibility of Using the Existing Garland Main Prior to Condition Assessment

Then feasibility of decommissioning the Northeast Plant immediately and relying on the existing Garland Main to supply finished water from Water Works Park treatment plant to the Northeast high lift pumping station was considered. The advantage of this approach would be an immediate savings in operation and maintenance costs, as well as savings in Small Capital Program capital costs to keep equipment working at the Northeast plant over the short term. Annual savings could be \$3,000,000 to \$5,000,000 annually. In order to use the existing Garland Main, the new State Fair valve project would need to be designed and constructed at an estimated cost of \$875,000. Also, some portion of the previously designed Water Works Park Yard Piping Project may need to be constructed. Therefore, it would take at least 18 months to complete that work.

There are risks to immediately decommissioning the Northeast plant, prior to completing a new finished water transmission pipeline from Water Works Park to Northeast. The risks are due to the

unknown condition of the existing Garland main, and the limited number of isolation valves on the 96-inch main, as described below. The capacity of the existing Garland Main is estimated to be approximately 60 MGD. However, the main has not been tested at this flow rate and pressure for many years. Currently, the Northeast plant provides an average of 100 MGD water supply to its service area. Therefore, the Lake Huron plant could provide approximately 50 MGD and the Water Works Park plant could provide approximately 50 MGD for average daily demand. In the summer, more water would need to be supplied from the Lake Huron plant.

The existing service area of the Northeast plant would be highly dependent on the reliability of both the Garland Main and the 96-inch Main. The 96-inch Main experienced a leak on an air valve in March 2015. The repair was temporary and difficult due to the limited number of isolation gates on this main. (Recommendations for 5 additional isolation gates are present in Section 7.10, and recommendations for condition assessment of transmission mains are presented in Section 7.11).

Due to the age and unknown condition of the Garland Main, plus the recent leak on the 96-inch Main and the known difficulty of repairing that main, it is likely that one or the other main could have a service disruption of 1 to 2 weeks within the next few years. A shut down of either main for 1 or 2 weeks would require water use restrictions and potentially a boil water advisory for up to 1 million people. With a water main shut down, the inconvenience to the public, economic hardship on businesses, and public health risks for customers for even 1 week could, if quantified, exceed the annual savings that would result from an early decommissioning of the plant.

The proposed 42-month schedule for condition assessment, installation of new isolation gates on the 96-inch main, fast track construction of a new 84-inch main in the Northeast raw water tunnel, pump rehabilitation at Water Works Park high lift station, coupled with an immediate down-sizing of treatment operations at the Northeast plant provides a sequence of construction that minimizes risk, increases long term benefits, and reduces short term treatment costs.

6.9.7 Modifications to the Water Works Park High Lift Station

This project will replace or recondition pumps number 2, 3, 4 and 5 so that each new unit can pump 50 MGD to the Northeast plant. Firm capacity would be 150 MGD. This project will also replace pumps number 11 and 12 with two new 20 MGD pumps for low demand periods at night. One pump would be equipped with a VFD. All pump replacements, or pump reconditioning, can be accomplished with minimal changes to existing piping and pump and motor structural bases. All pumps would operate at lower speeds than the existing pumps, and new motors would be provided.

Figure 6-13 shows a conceptual system head curve for the new pipeline to Northeast that was used to estimate the configuration and capacity of the new 50 MGD pumps. **Figure 6-14** shows a proposed layout of the new pumps.

6.9.8 Ongoing Investigation of the Raw Water Tunnel

DWSD has initiated work on the inspection of water intakes and tunnels under project CS-1623. The inspection of the Northeast raw water tunnel is expected to be complete by the end of 2015. This work is expected to confirm the feasibility and constructability of building a new transmission main in the tunnel.

The Garland Main condition assessment should be performed concurrently during 2015.

By the first quarter of 2016, there would be enough condition information available to make a decision on which alternative to choose. At that time, DWSD should begin design for the selected project.

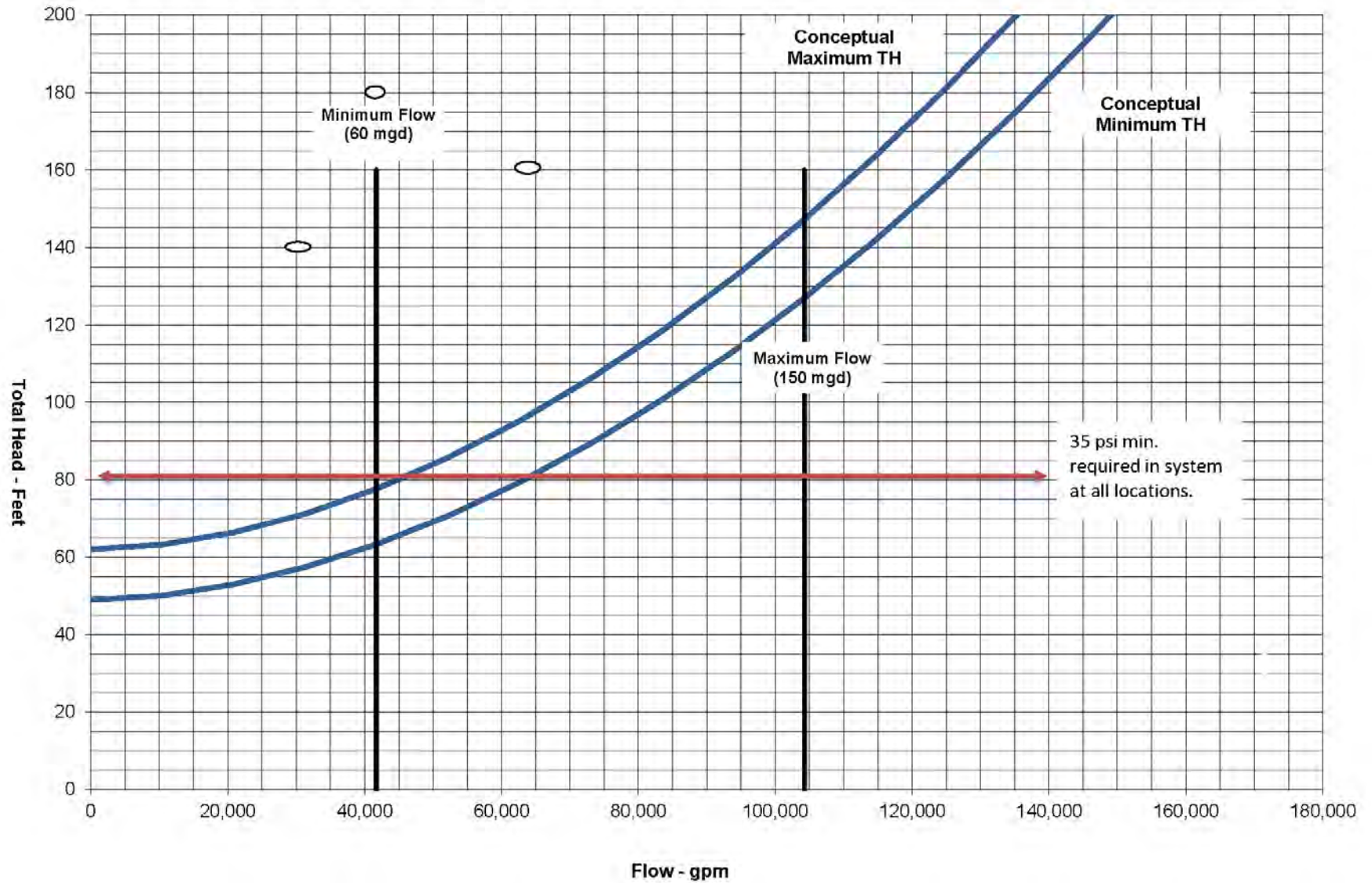


Figure 6-13: Conceptual System Head Curve for New Pipeline within Northeast Raw Water Tunnel

6.9.9 Water Works Park Yard Piping

Project WW-536 was designed in 2009 and DWSD received bids in March 2011. This is a multi-faceted project to improve transmission mains within the Water Works Park yard to make improvements to drainage, roadways, electrical, distribution, water metering, and to the Belle Isle intake.

There were 11 bids received ranging from approximately \$31 million to \$49 million. Six bids were in excess of \$40 million. DWSD currently plans to review the design, some of which is over 8 years old. This master plan recommends that the review have the following objectives:

1. Review number and location of new transmission mains from the site. The original design provided for water transmission to the historic service area of Water Works Park, but the current and projected demands in the Downtown are lower than in previous decades. Most water will be conveyed to the Northeast plant, which will simplify the improvements within the yard for transmission to the Downtown.
2. Review the production metering design. DWSD will soon begin a contract for Venturi meter rehabilitation at Northeast, Springwells, and Southwest. The new meters at the Northeast plant will measure the finished water supplied from Water Works Park. New or refurbished metering at the Water Works Park plant should consider whether the full plant flow should be measured, or just the flow not measured at the Northeast plant.
3. Re-prioritize work unrelated to water transmission and metering. Much of this work could be postponed for several years.

The cost estimates for the Garland Main Replacement and the reuse of the Northeast raw water tunnel reuse already include the cost of new transmission from the WWP high lift station to the NE reservoirs, so WW-536 is not required to implement either of those alternatives.

6.9.10 Additional Design Considerations

The series of projects described in Sections 6.9.1 to 6.9.9 should be developed further through advanced planning, or concurrently coordinated to the 30-percent design stage. Coordination of the project designs through 30-percent will allow for optimization and value engineering that will provide the most cost-effective and implementable construction projects.

Discussions with DWSD engineering staff late in the master planning process identified these considerations and clarifications for design:

- CIP Projects in TM-17 for the Northeast reservoir inlet outlet and outlet piping separation and baffle project needs to be constructed with the repurposing program improvement. Also, the PRV at Marx Street and 8-Mile Road needs inspection and testing, and a short reach of its adjoining main needs an increase in diameter from 36-inches to 54-inches.
- Final design of the pipeline with discharge to the reservoirs at Northeast should consider reservoir fill time, and end of pipe pressure control as the pipeline enters the reservoir.

- There is significant pipe capacity in place in the corridor from Water Works Park to Northeast. While cost estimates for the master plan have allowed for a new 72-inch to 84-inch main, equivalent capacity could be achieved with a combination of the 48-inch Garland Main plus an additional 42-inch main in the area.
- Route options for use of the Northeast raw water tunnel versus open cut excavation from Forest and Pennsylvania to Northeast should be fully explored. Recent tunnel investigation shows the need for some tunnel repair. Also, given the 10-foot diameter, a significant volume of grout is needed, which adds to the cost of the tunnel route. At the same, use of the tunnel is faster, requires fewer utility relocations and easements, and avoids the cost of abandoning the tunnel (estimated to be up to \$500 per foot, or \$10 million).
- Isolation valves for the 96-inch main, are urgently needed. These will be costly, and may need to be phased with the relocation of the 96-inch main in project WS-622 at 24-Mile and Dequinder. Therefore the proposed 20-year CIP shows 2 phases of the isolation valve project. First phase to install at least 2 more valves, second phase to add up to 3, for a total of 5.
- Springwells raw water tunnel repair should proceed over the next 5 years, but it is not deemed necessary to complete this ahead of the repurposing project.

The alternative for constructing a second transmission main from the Lake Huron WTP was considered in the Phase 1 portion of this master plan update, but it was not carried forward for further evaluation in Phase 2. A modified concept of a second feed from Lake Huron was discussed with DWSD engineering staff. The modified concept would include a new transmission main generally parallel to US-94 from the Lake Huron WTP to 24-Mile Road in Chesterfield. This modified concept would create a new transmission loop north of 24-Mile Road, and would rely on existing transmission loops south of 24-Mile Road. The new transmission main parallel to US-94 would be an alternative to building a new transmission main from Water Works Park to the Northeast WTP.

Further evaluation of this new alternative should consider the following advantages and disadvantages:

- **Advantages:** The second feed would provide redundancy to the Lake Huron plant supplying the communities in Oakland and Macomb Counties, and it could eliminate the need for a future expansion of the Rochester pump station. Furthermore, the second feed would allow for the re-routing of the 96-inch main that is currently under a landfill and an opportunity to reduce the number of new isolation valves on the 96-inch main. The construction of a new 6-mile transmission main from Water Work Park to Northeast would also not be required. Depending on the results of a condition assessment study, rehabilitation of the Garland Main early in the planning period may not be required.
- **Disadvantages:** Extensive planning, design, and easement acquisition would be required for a new 40 mile transmission main. There is a potential need for the proposed Chesterfield pump station to boost pressure from the new main, so the new main would not necessarily eliminate the proposed pumping station. There is a higher annual cost for pumping finished water approximately 60 miles from Lake Huron WTP versus 6 miles from Water Works Park WTP, however the pumping strategy, including a potential reduced reliance on the Imlay City booster

station, may reduce the cost differential. The Water Works Park plant would continue to operate below its rated capacity due to limited demand in its service area.

6.9.11 Treatment Decommissioning

Two events in the operating history of the Northeast WTP provide background for a decommissioning plan.

1. During the early 1970's when total summer system flow often exceeded one billion gallons per day, finished water from Water Works Park was directed to the reservoirs at Northeast on a regular basis. After improvements were made to Northeast, including the addition of the third raw water conduit from the low lift to the filter building, the regular practice of filling the reservoirs from Water Works Park was discontinued.
2. In 1985, the Northeast plant was temporarily shut-down to repair a reach of the raw water tunnel near Outer Drive and Greiner Street, approximately one mile south of the plant. The shutdown lasted approximately six months, and the plant was operated as a booster station. Drinking water was supplied to the high lift from Water Works Park through the State Fair valve just south of the plant perimeter and the Lake Huron Water Plant through the reservoir fill valve on the north boundary of the plant.

The temporary shut-down in 1985 provides a basis for a future decommissioning plan for the Northeast WTP.

One or more workshops should be held with DWSD personnel to review the decommissioning plan. Additional studies may be needed to confirm certain aspects. After transmission improvements are made, these should be fully tested under summer and non-summer demand conditions.

Prior to decommissioning, the transmission improvements described earlier need to be completed.

Once the transmission improvements are complete, the sequence of steps within the Northeast water treatment plant would be as follows:

1. The optimum time frame to initiate the process would be after the peak summer flows. In general that would be after September 15th.
2. The new State Fair valve must be fully operational and Water Works Park must have the ability to deliver the required supply of water. Both of these capabilities need to be tested prior to beginning the decommissioning.
3. Depending on how the Northeast yard piping improvements are made, it may be necessary for the Lake Huron Water Plant Fill Valve to be exercised to verify it is operational.
4. Coordination with all the plants and System Control is essential. There will be a reversal of the usual flow of water in parts of the system. This will probably require additional valve positioning at Water Works Park and the suburbs north of Northeast.

5. There is the possibility of upsetting any deposits in the mains with resulting water quality complaints. There is also the issue of additional stresses on existing water mains near Northeast unaccustomed to potential higher system pressures.
6. Strictly follow all safety procedures such as lockout/tag out and confined space entry.
7. The shutdown of Northeast can be undertaken in one operation. Start with full reservoirs at Northeast.
8. Shut down and lock out the low lift pumps and remove all the filters from service. Cease operation of all chemical feed systems.
9. Gradually open the State Fair valve and initiate valve operations at NSC. There should be enough water in the reservoirs to last four to six hours.
10. During this time if the process is not working properly just return the low lift pumps, chemical feed and filters to service.
11. When all DWSD parties are satisfied with the operation, proceed to the next steps. This could be hours or days. The following steps involve dewatering the plant.
12. Wash each of the forty-eight filters at least twice. All filters should be left in the off position with the drain valves open. The effluent valve from each filter can be operated to remove all the remaining water from the filter. This will prevent icing of the filter during the winter.
13. Dewater the north and south applied water conduits. There are valves on the south side of the filter plant that allow these conduits to be emptied.
14. Close the four influent gates to the settling basins.
15. Dewater the rapid mix tanks from each settling basin. There are valves in the chemical building that will drain these chambers.
16. Dewater the four settling basins that can then be sequentially emptied and cleaned. Follow the normal plant procedures for draining and cleaning.
17. Drain the raw water conduits from the low lift to the filter building.
18. Until buildings are re-used, turn down heat and minimize electrical use.
19. The raw water tunnel from Water Works Park to the NEWTP should be kept full to protect its integrity. The tunnel should be inspected at appropriate intervals, based on initial inspection, and subsequent inspection results.
20. Install bulkhead at filter effluent tunnel.

6.10 Program for Regulatory Compliance

The program for regulatory compliance includes regulatory driven projects from the needs assessment in TM-13, the goals of Chapter 5, plus the change to direct filtration for Lake Huron, as discussed in TM-10.

There are 12 projects in the preliminary CIP driven by regulatory compliance objectives, and this work spans the whole planning period.

6.10.1 Chlorine Conversion

A major potential regulatory compliance project is conversion from chlorine gas as disinfectant. This section discusses the status of chlorine gas conversion in the United States and a conceptual basis of design for future chlorine conversion for DWSD.

6.10.1.1 Status of Chlorine Gas Conversion in the United States

Nationwide, the Department of Homeland Security (DHS) is advocating for inherently safer technologies and potentially requiring water/wastewater utilities to eliminate chlorine gas. Many plant operators prefer the simplicity and lower cost of chlorine gas for disinfection. The following is a snapshot in 2015 regarding the status of chlorine conversion. In general, larger water utilities in the midwest and northeast have not convert from chlorine gas.

West: Most water plants in California have converted from chlorine gas following a statewide initiative in the early 1990's. Denver completed chlorine conversion. The Trinity River Water Authority serving Dallas and Ft Worth has performed planning for conversion from chlorine gas.

Southeast: Many plants in the Southeast have converted from chlorine gas to sodium hypochlorite, or on-site generation. Construction to convert from chlorine gas is underway in Knoxville, Tennessee. New Orleans and Charlotte, North Carolina have not yet converted.

Midwest: Many small to mid-size water plants in the Midwest near schools, parks, etc. have made the conversion to sodium hypochlorite. Indianapolis has converted its four water treatment plants. Among major cities that have not converted from chlorine gas are: Chicago, Evanston, IL, Fort Wayne, IN, Hammond, IN, and Minneapolis.

Northeast: Most water suppliers in New England have converted to sodium hypochlorite. Hartford, CT has converted one plant and continues to use chlorine gas at another. Larger New England cities that have not converted are Providence, RI, Springfield, MA, and Worcester, MA. New York City has not converted from chlorine gas.

6.10.1.2 Concept Design for On Site Generation of Sodium Hypochlorite

A preliminary concept design for chlorine conversion for DWSD was developed around on-site generation of sodium hypochlorite.

Salt in the form of brine would be purchased and stored at each plant. There would be two or three brine tanks per facility, and each would be 12-foot diameter and 26-foot tall. Brine storage on site would need to be sufficient to meet requirements for 30 days of chemical storage for average day production.

Reactors to generate sodium hypochlorite are as large as 2,000 pounds per day. Up to four reactors would be needed per plant, including an allowance for one reactor out of service. Sizing of reactors was based on an average 2 mg/L chlorine dose. Electrical costs are estimated at 1.8 Kw-hours per pound of chlorine.

Storage of liquid sodium hypochlorite on site would be in tanks of similar size to the brine tanks.

Ancillary equipment would include pumps, chillers, heaters, containment dikes, instrumentation and control.

Concept level cost estimates for installations at all four plants are as follows:

Construction cost.....	\$50,000,000
Annual costs for brine, electricity, operation and maintenance.....	\$5,500,000

6.11 Program for Renewal, Reliability and Energy Management

The program for treatment plant renewal, reliability and energy management includes the replacement and upgrade of equipment for service life or process improvement and operational efficiency.

There are approximately 74 projects in the preliminary CIP under this program. The scope and schedule of these projects should be refined through asset management evaluations.

7 Transmission

The evaluation of the transmission system was performed in two phases. The first phase focused on priorities for the first five years of the planning period and examined three locations where needs for transmission improvements have been previously identified between DWSD and the respective wholesale customers. These locations are Adams Branch in Oakland County, 24-Mile Road in Macomb County, and the Downriver area of Wayne County.

The second phase of evaluation of the transmission system involved extensive hydraulic modeling and was a comprehensive examination of pipeline, booster pump station, reservoir needs to the year 2035. Phase 1 findings are presented below. Phase 2 findings begin in Section 7.9.

7.1 Adams Branch

The Adams Branch is the area north of South Boulevard fed from the Adams Pump Station. This branch is a single 42-inch water main that supplies Auburn Hills and Orion and parts of Pontiac and Rochester Hills. This area experienced periods of low pressure during summer months prior to 2008. Since 2008, peak water demands have been reduced or mitigated with new storage facilities, resulting in fewer pressure concerns.

The Analytical Work Group held a series of discussions with customers and DWSD from December 2011 to September 2013 on water service for the Adams Branch area. A summary of those discussions is presented in **Table 7-1**.

Table 7-1: Summary of AWG Discussion of Adams Branch

Date	Discussion and Findings
December 2011	The Technical Advisory Committee (TAC) Analytical Workgroup (AWG) identified the Adams Branch as the number one topic of discussion for 2012.
January 2012	A table of options was developed with long-term and short-term options and then reviewed by DWSD.
February 2012	Based on input from the DWSD Director, the options were revised with a new midterm option. The decision was made for the Adams Branch issue to continue to be assessed during break-out discussions that would follow the regular AWG Meetings.
March to May 2012	Adams Branch break-out meetings were held in March, April and May. Short term options for addressing the issue, including modifying the Auburn Hill tank filling schedule, were implemented.
July 2012	DWSD issued a report on July 16 presenting a hydraulic analysis of options for improving the pressure along the Adams Branch. The DWSD report concluded that the most cost effective method to improve pressure in the Adams Branch is to install an additional 17.5 mgd pump at the Orion Pump Station. The AWG recommended that additional options be considered as part of the Master Plan Update.

Table 7-1: Summary of AWG Discussion of Adams Branch

Date	Discussion and Findings
August 2012	Orion Township placed a new storage tank in service during August 2012 with the expectation to ease the pressure on the Adams Branch.
September 2013	OCWRC representatives presented a proposal for a North Oakland County Water Authority (NOCWA) representing a potential agreement among the Adams Branch customers to form a water authority similar to SOCWA. This proposal would allow for the customers to complete interconnections among their respective systems and maximize the use of existing customer and DWSD storage and transmission facilities.

Storage facilities have been installed along the Adams Branch area since 1963. In addition to the Orion Township storage tank noted in **Table 7.1**, Auburn Hills placed 1.5 million gallon storage tank in service in 2005. The DWSD Adams Road pump station includes a 10 million gallon tank that was placed in service in 1998. The one other reservoir on the Adams Branch is the 2 million gallon elevated tank for Pontiac that “floats” on the hydraulic grade and has been in service since approximately 1963.

Regarding low pressures prior to 2008, the daily, concurrent, outdoor irrigation water demands in the early morning and evening appeared to be the root of this area’s previous pressure problems.

In April 2013, the AWG offered the following guidance for the statement of work for the Master Plan Update:

1. DWSD was able to provide adequate pressure and flow during the 2012 summer season. This was done by utilizing the full capacity of the Orion Pump station.
2. Upgrades to the Orion Pump Station should be further evaluated by the Master Plan Update to assure that it can reliably supply water at its firm capacity.
3. Master Plan Team’s evaluation, including discussion with Oakland Township, should determine the desired parameters for DWSD water supply.
4. DWSD customers supplied from the Adams Branch should continue to work to develop a long term plan to supply DWSD water to the Adams Branch area.
5. Any additional studies of the North Adams Branch should be conducted on both peak hour demand and middle-of-the-night conditions using data acquired after the Orion Township tank was placed into service on August 24, 2012.
6. The issue of system redundancy should be addressed by the Water Master Plan update project.

Water demands for the Adams Branch area were reviewed based on results received to date from customers responding to the Technical Data Request. These customers have provided estimates of

growth similar to those projected by SEMCOG. At the same time, demand management initiatives are under way, most importantly the more efficient use of 12 million gallons of storage in the City of Pontiac. See Section 7.4 for conclusions and recommendations.

Figure 7-1 shows the proposed NOCWA system.

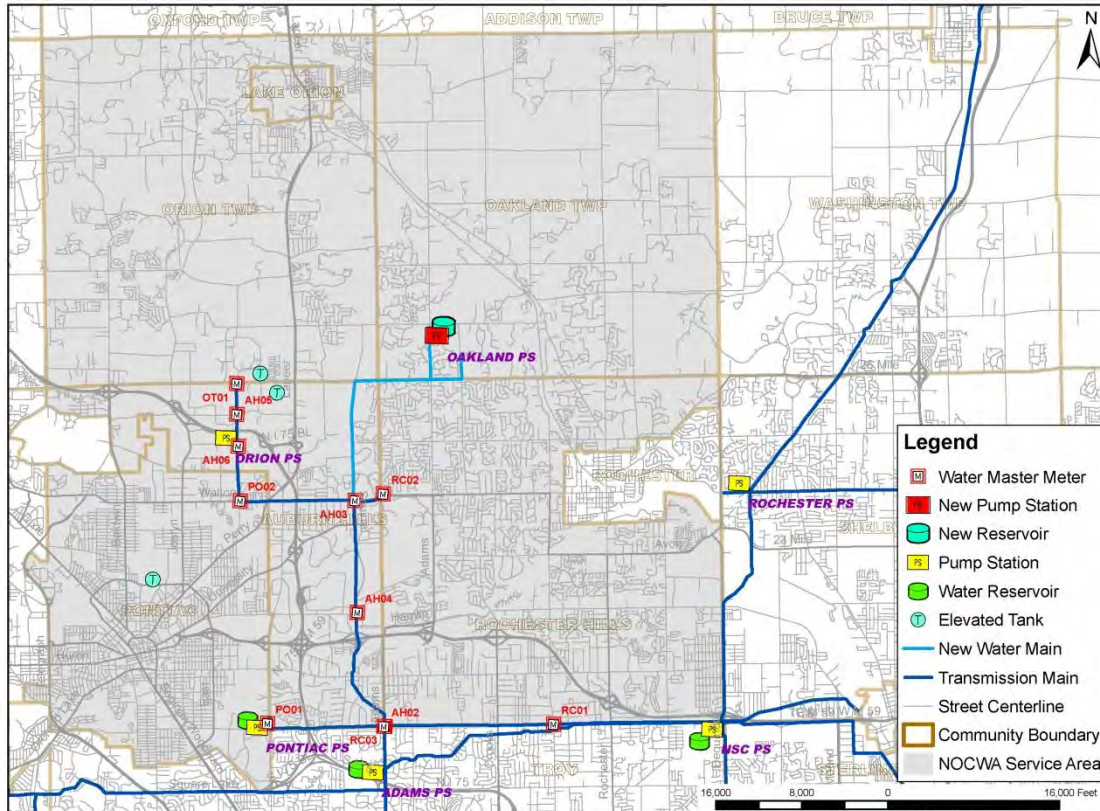


Figure 7-1: CS 1258 - DWSM Water Master Plan Update, Service Area and Water Supply for North Oakland County Water Authority (NOCWA)

7.2 24-Mile Road

Several customers in the vicinity of 24-Mile Road in Macomb County have observed low water pressure during summer months. This area of Macomb County had experienced substantial growth from the 1990's to 2007, and significant new housing and retail development has started again since 2011. Figure 7-2 shows this portion of the service area.

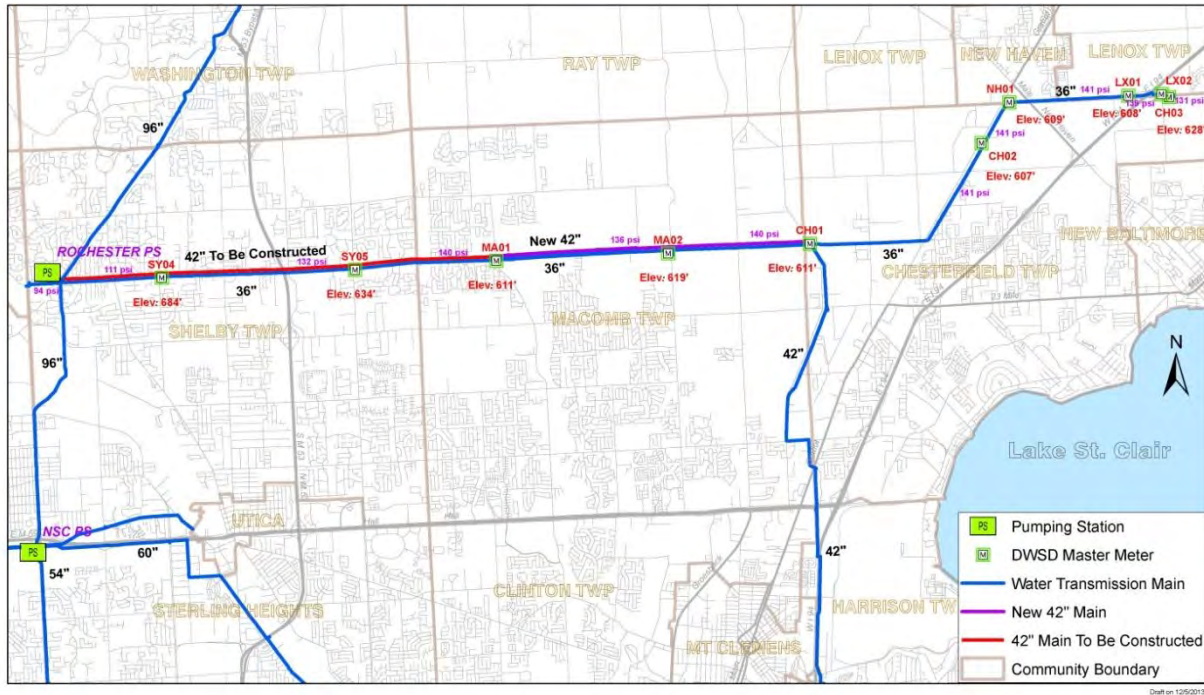


Figure 7-2: DWSD 36" & 42" Water Mains under Construction in 24 Mile Road

A history of the pressure issues identified by customers is summarized in **Table 7-2**.

Table 7-2: History of Pressure Issues Along 24-Mile Road

Date	Pressure Issue
2010	DWSD WS-674 24 Mile Road parallel main project—Fairchild to Foss Road put out for bid.
2010	Macomb reported low pressure issues at MA-03 during peak summer season. Calls to SCC result in increase 10 psi within 20-30 minutes.
2010	Macomb Township voiced frustration with the low pressures in the late evening and early morning hours. More discussion about staggering the start of the irrigation systems was concluded to be a way to mitigate the low pressure, Chesterfield experiences lower pressures in the summer months. In summer, the DWSD Snover valve just south of 24 Mile Rd is closed and CH-01, CH-02 and CH-03 are fed by the Lake Huron WTP through the 24 Mile main and CH-04 and CH-05 are fed by the Northeast WTP.
2011	Design complete for the 24 Mile Road pipeline between Rochester Station to Romeo Plank Rd. This project was bid in January 2011

In 2010, DWSD began construction of a new parallel transmission main on 24-Mile Road to improve water delivery to these customers. This new transmission main will be 36-inch to 42-inch diameter, and it will be parallel to an existing main in 24-Mile Road. Construction was advertised in three contracts due to an easement issue at the eastern end of the construction. As of November 2013, two contracts had been completed. A construction contract for the final segment of 42-inch parallel main

on 24-Mile Road from the Rochester Pump Station to Romeo Plank Road (Project WS-681) is anticipated to be finished by 2017.

The hydraulic model of the transmission system was run to simulate future conditions in the vicinity of 24-Mile Road, after the current construction projects are completed. The modeling was performed with year 2011 maximum day water demands. A meeting was held with representatives of Shelby, Macomb, Chesterfield, and Lenox townships and the Village of New Haven in November 2013. A follow-up meeting with Macomb Township was held in February 2014 to review their projections for population and employment growth over the next twenty years. Substantial new development and expansion of the water service area further north in the township is planned.

Section 7.10 presents recommendations for transmission redundancy, which include a new connection to the 24-Mile Road main from the east end and a new booster pumping station near Chesterfield.

7.3 Downriver

Background information on previous analysis and assessment of the needs of the Downriver transmission system are presented in **Table 7-3**.

Table 7-3: Background on Downriver Transmission System Needs Assessment

Date	Discussion and Findings
Summer 2005	Brownstown Township (BR), City of Flat Rock (FK), Grosse Ile Township (GI), Huron Township (HN) and Ash Township (AS) met with DWSD to discuss low and high pressures issues.
December 2005	A hydraulic analysis performed and a report issued by DWSD.
2006 to 2012	DWSD added the Ready Road Pump Station and Reservoir to its Water Capital Improvement Plan and engaged a consultant to prepare a preliminary and final design. After 2012, the project was suspended pending the completion of the Water Master Plan Update.
December 2011	Technical Advisory Committee (TAC) Analytical Workgroup (AWG) identified the Downriver pressures as an issue to be addressed by the group.
March 2011 AWG	Short-term, mid-term, long-term recommendations and extents of the pressure issues evaluated and discussed.
May 2012 AWG	DWSD's historical flow and pressure data from the Wholesale Automated Meter Reading (WAMR) system was reviewed and shared with the communities.

Table 7-3: Background on Downriver Transmission System Needs Assessment

Date	Discussion and Findings
July 2012 AWG	Background on the Downriver area of the system, improvements made to the Downriver portion of the system, and drivers for selection of Downriver pressure issues were discussed. It was noted that a shutdown of the Allen Road main just north of Pennsylvania would affect 18 meters and cut-off service to Brownstown Twp., Berlin Twp., Flat Rock, Gibraltar, Rockwood, S. Rockwood and Woodhaven. These communities are connected together through emergency connections (ECs) with the exception of South Rockwood and Berlin Township, which only have ECs with each other. This would allow limited water service to transfer across communities, but would only provide minimal and temporary service.
August 2012	The current status of Downriver system including an overview of the map of the system and historical flow and pressure data was reviewed.
September to November 2012	DWSD provided feedback as a follow up to questions from the August AWG meeting. Overall, it was determined that the pressure issues that had existed in 2005 were no longer a problem for that portion of the system. However, emphasis was shifted to a concern regarding redundancy in the portion of the system south of Pennsylvania Avenue. Additionally, the ability of the Ready Road Pump Station and Reservoir to mitigate the redundancy issue was questioned. In case of an interruption of supply from DWSD south of Pennsylvania Avenue, the proposed Ready Road Reservoir would supply the affected system for only about 9 hours. Even a proposed water main along Will Carleton would be an extension from the proposed Ready Road pump station. It would not connect back into the system to loop the Downriver system. A complete loop would require the construction of approximately 6 additional miles of main on the proposed Will Carleton extension.

In April 2013, the AWG offered the following guidance for the statement of work for the Master Plan Update:

1. The proposed Ready Road Reservoir and Pump Station should not be constructed at this time. The timing of this project should be addressed by the Water Master Plan Update project.
2. The issue of system redundancy should be addressed by the Water Master Plan project.
3. DWSD should quickly develop a contingency plan to address a supply disruption south of Pennsylvania Avenue. At a minimum, this plan should address:
 - a. What type of disaster recovery type actions DWSD will perform in case of an emergency or long-term supply disruption?
 - b. How should the DWSD main in Allen Road be protected from third party damage?
 - c. How will DWSD repair damage to the Allen Road main, including a time estimate for making repair?

- d. What actions can the Downriver communities take to obtain emergency water supply through interconnections with their neighboring communities? Included in this should be a study of how much water can be delivered through existing customer mains that serve as loops between TN-1 and TN-3 and GI-1 and GI-3.

This Master Plan Update included additional assessment of future water demands, demand management, emergency response planning, and evaluation of flows and pressures at wholesale meters as documented elsewhere in this report. The Master Plan Update team also met with representatives of the Downriver Community Conference in January 2014 to discuss alternatives.

A total of seven alternatives are examined based on initial position of the AWG and discussion with stakeholders. Hydraulic modeling was performed to develop estimates of pipe diameter and pump station and reservoir capacity. The consideration of emergency supply from the City of Monroe was based on preliminary information regarding the City's water plant capacity. Due to the limited supply available from Monroe, and due to the cost of connecting to Monroe, this alternative is not cost-effective compared to other alternatives.

Table 7-4 presents the seven alternatives and their cost estimates. **Figures 7-3 to 7-9** present preliminary route layouts for the alternatives. All options provide the needed redundancy to the single transmission main. However, Alternative Number 7 is significantly less expensive than the other options, and it is recommended for immediate implementation in Section 7.10.

Table 7-4: Cost Estimate for Downriver Alternatives

Pressure Improvement and Redundancy Supply Alternative		Capitol Improvement Item	Unit	Quantity	Estimated Cost
1	Current Design of Ready Road Pump Station & Will Carleton Extension Pipeline	1-1 Ready Road Pumping Station with a Reservoir of 5 MG	LS	1	\$15,000,000
		1-2 Will Carleton Extension 30" Pipeline	LF	42,540	\$24,247,800
		1-3 Will Carleton Extension 30" Rail or Highway Crossing	LF	150	\$213,750
		1-4 Will Carleton Extension 30" Stream Crossing	LF	50	\$42,750
		1-5 Will Carleton Extension 24" Pipeline	LF	15970	\$7,234,410
		1-6 Will Carleton Extension 24" Rail or Highway Crossing	LF	200	\$226,500
		1-7 Will Carleton Extension 24" Stream Crossing	LF	250	\$169,875
		Total			
2	Parallel Pipeline along Allen Rd./Dixie Hwy	2-1 Parallel 24" Pipeline along Allen Rd.	LF	26,700	\$12,901,440
		2-2 Rail or Highway Crossing for 24" Pipeline along Allen Rd.	LF	50	\$60,400
		2-3 Stream Crossing for 24" Pipeline along Allen Rd	LF	200	\$144,960
		2-4 Parallel 16" Pipeline along Allen Rd.—Fort St.	LF	32,100	\$11,812,800
		2-5 Rail or Highway Crossing for 16" Pipeline along Allen Rd.—Fort St.	LF	200	\$184,000
		2-6 Stream Crossing for 16" Pipeline along Allen Rd—Fort St.	LF	450	\$248,400
		Total			

Table 7-4: Cost Estimate for Downriver Alternatives

Pressure Improvement and Redundancy Supply Alternative		Capitol Improvement Item	Unit	Quantity	Estimated Cost
3	New Reservoir with Pumps to Supply	3-1 Ready Road Pumping Station with 3 Reservoirs (10MG Each)	LS	1	\$30,675,000
		Total			\$30,675,000
4	Supply from Electric Ave. PS and Parallel Pipeline along Allen Rd./Dixie Hwy	4-1 Parallel 16" Pipeline along Allen Rd.—Fort St.	LF	32,100	\$11,812,800
		4-2 Rail or Highway Crossing for 16" Pipeline along Allen Rd.—Fort St.	LF	200	\$184,000
		4-3 Stream Crossing for 16" Pipeline along Allen Rd—Fort St.	LF	450	\$248,400
		Total			\$12,245,200
5	Emergency Water Supply from Monroe System	5-1 New 24" Pipeline connected to Monroe along I-75	LF	63,500	\$30,683,200
		5-2 Rail or Highway Crossing for 24" Pipeline along I-75	LF	250	\$302,000
		5-3 Stream Crossing for 24" Pipeline along I-75	LF	1,200	\$1,449,600
		5-4 Crossing of River Raisin (24" in Diameter)	LF	800	\$724,800
		Total			\$33,159,600
6	Alternative #1 Extension Pipeline Looped to Wick Rd.	6-1 New 30" Will Carleton Extension Pipeline	LF	42,540	\$24,247,800
		6-2 Will Carleton Extension 30" Rail or Highway Crossing	LF	150	\$213,750
		6-3 Will Carleton Extension 30" Stream Crossing	LF	50	\$42,750
		6-4 New 24" Will Carleton-Clark Rd. Pipeline	LF	46,770	\$21,186,810
		6-5 Rail or Highway Crossing for 24" Will Carlton-Clark Rd. Pipeline	LF	200	\$226,500
		6-6 Stream Crossing for 24" Will Carlton-Clark Rd. Pipeline	LF	500	\$339,750
		Total			\$46,257,360
7	Use Community Distribution Pipes for Emergency Water Supply	7.1 New 12" Pipe	LF	2,000	\$736,000
		7-2 Crossing of Huron River (12" in Diameter)	LF	600	\$331,200
		7-3 Tapping Sleeve for Connecting Distribution Pipe to Transmission	EA	5	\$150,000
		7-4 Magnetic Flow Meter and Vault	EA	6	\$690,000
		7-5 Cone Valve and Vault	EA	6	\$3,000,000
		Total			\$4,907,200s

7.4 Phase 1 Conclusions

Based on the investigations of the Adams Road, Macomb County and Downriver pressure and redundancy problems, the following conclusions were reached and next steps proposed.

- Adams Branch originated as a pressure issue and is now primarily of concern due to lack of transmission redundancy. Wholesale customers in this area have investigated solutions for interconnections and transmission improvements between their communities, and these interconnections and improvements would resolve most of the pressure and redundancy problems. These customers have formed a new local water authority called the Northern Oakland County Water Authority.

- NOCWA was formed in October 2014, and it is in the process of optimizing its inter-community operations. The NOCWA communities include the highest elevations of the service area of the DWSD system. NOCWA reports that there are no interconnections among its member communities that will be used as part of regular non-emergency connections. The DWSD Orion pump station upgrade is scheduled for the year 2020, but could be implemented sooner. NOCWA believes that greater utilization of resources within its member communities should result in improvement in pressures. Modeling for this Water Master Plan shows the ability to meet projected 2035 maximum day demands when all booster stations and reservoirs are at design capacity. Collaborative modeling of the DWSD transmission system and NOCWA system, would provide an approach to optimize NOCWA's connection to DWSD.
- 24-Mile Road in Macomb County has experienced pressures below contract limits of some of the wholesale customers. DWSD is constructing improvements to the Rochester Pump Station and constructing a parallel main in 24-Mile Road. When those improvements are complete in 2016, then current pressure problems are expected to be resolved.
- The downriver communities identified a pressure issue approximately 9 years ago. In response, DWSD developed a plan for a new pumping station and transmission main extension for the area. The pressure issues have been largely eliminated since 2007. Redundancy of transmission remains a concern. In the current forecast of flat future water demand in this area, the most cost-effective initial improvement to improve redundancy is to establish agreements and new interconnections between existing water distribution mains owned by the communities in the area. Later in the planning period, depending on growth in the area, then additional DWSD transmission improvements may be justified.

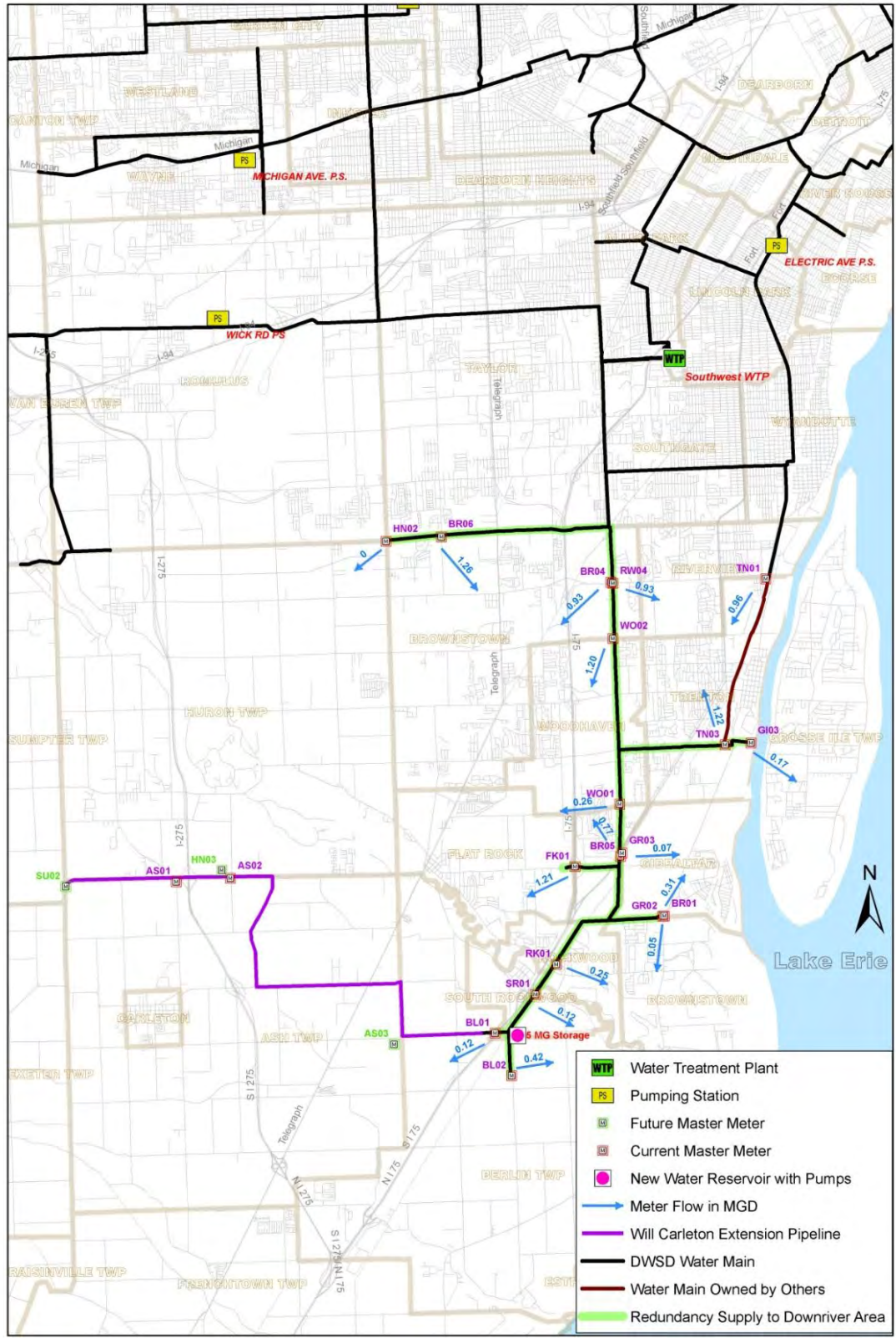


Figure 7-3: Supply Alternative 1 – Current Design of Ready Road Pump Station & Will Carleton Extension Pipeline

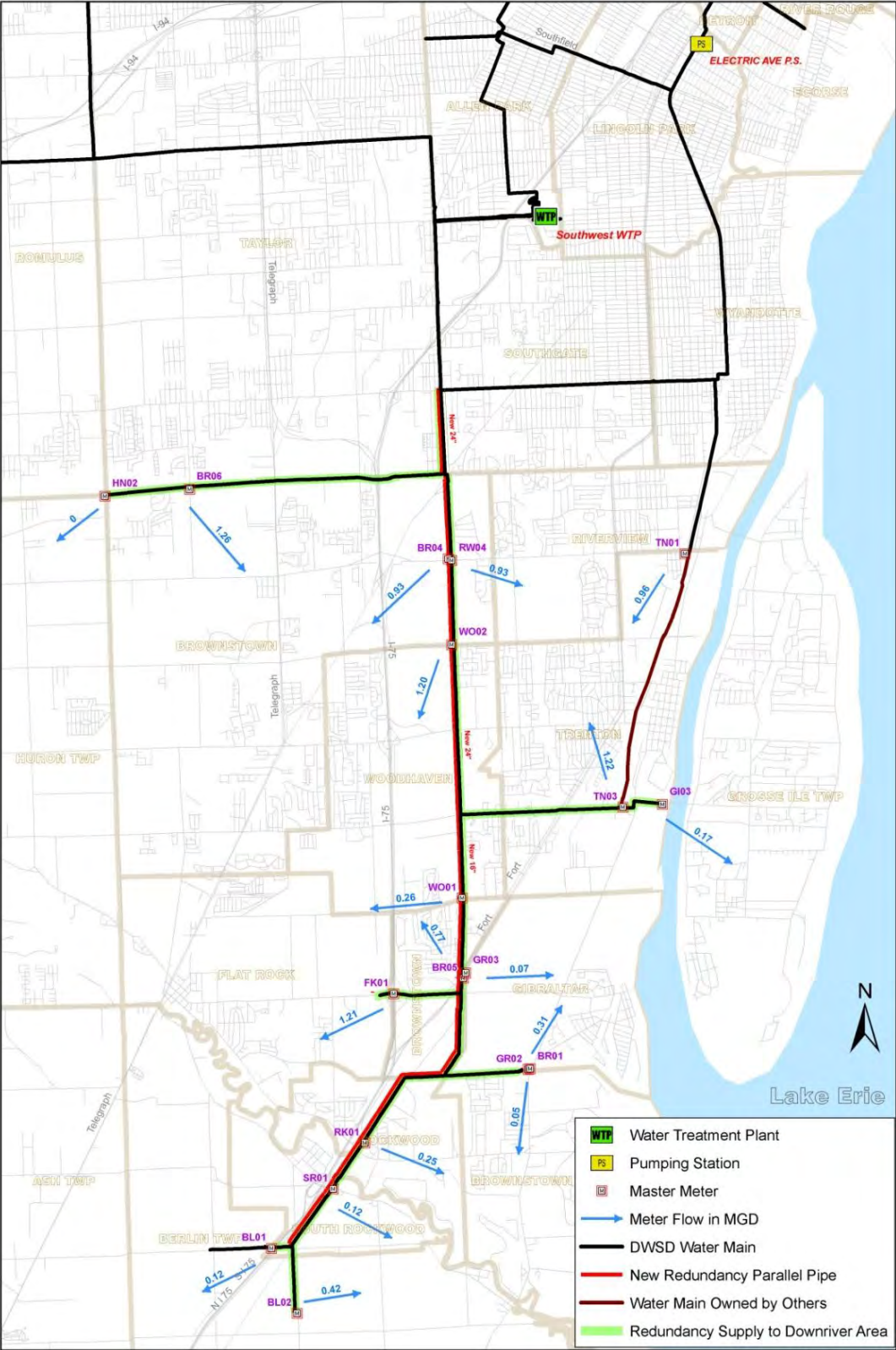
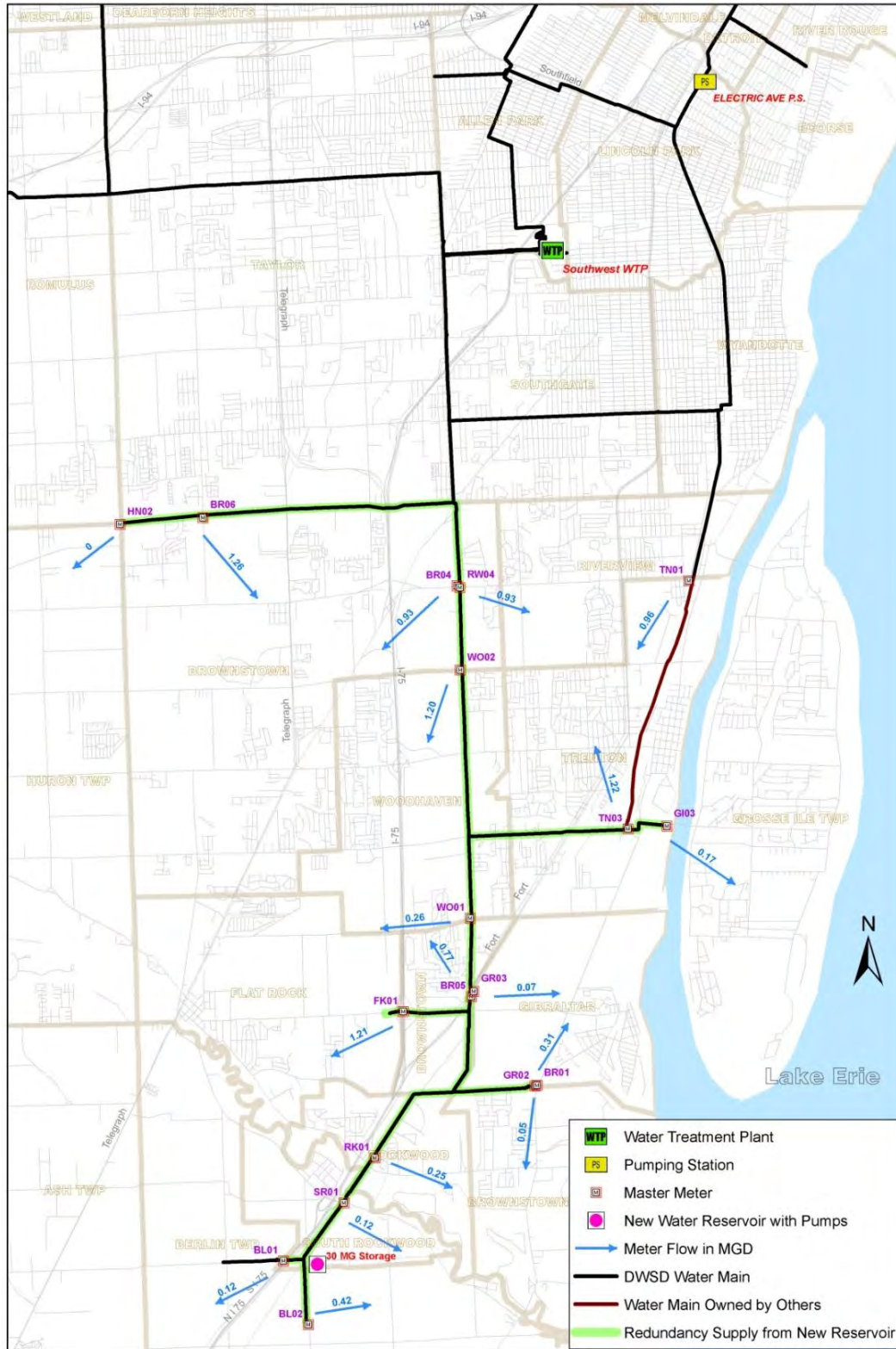


Figure 7-4: Alternative 2 – Parallel Pipeline along Allen Road/Dixie Hwy



Draft on 12/10/2013

Figure 7-5: Alternative 3 – New Reservoir with Pumps to Supply Average Day Demand up to 3 Days



Figure 7-7: Alternative 5 – Emergency Water Supply from Monroe System

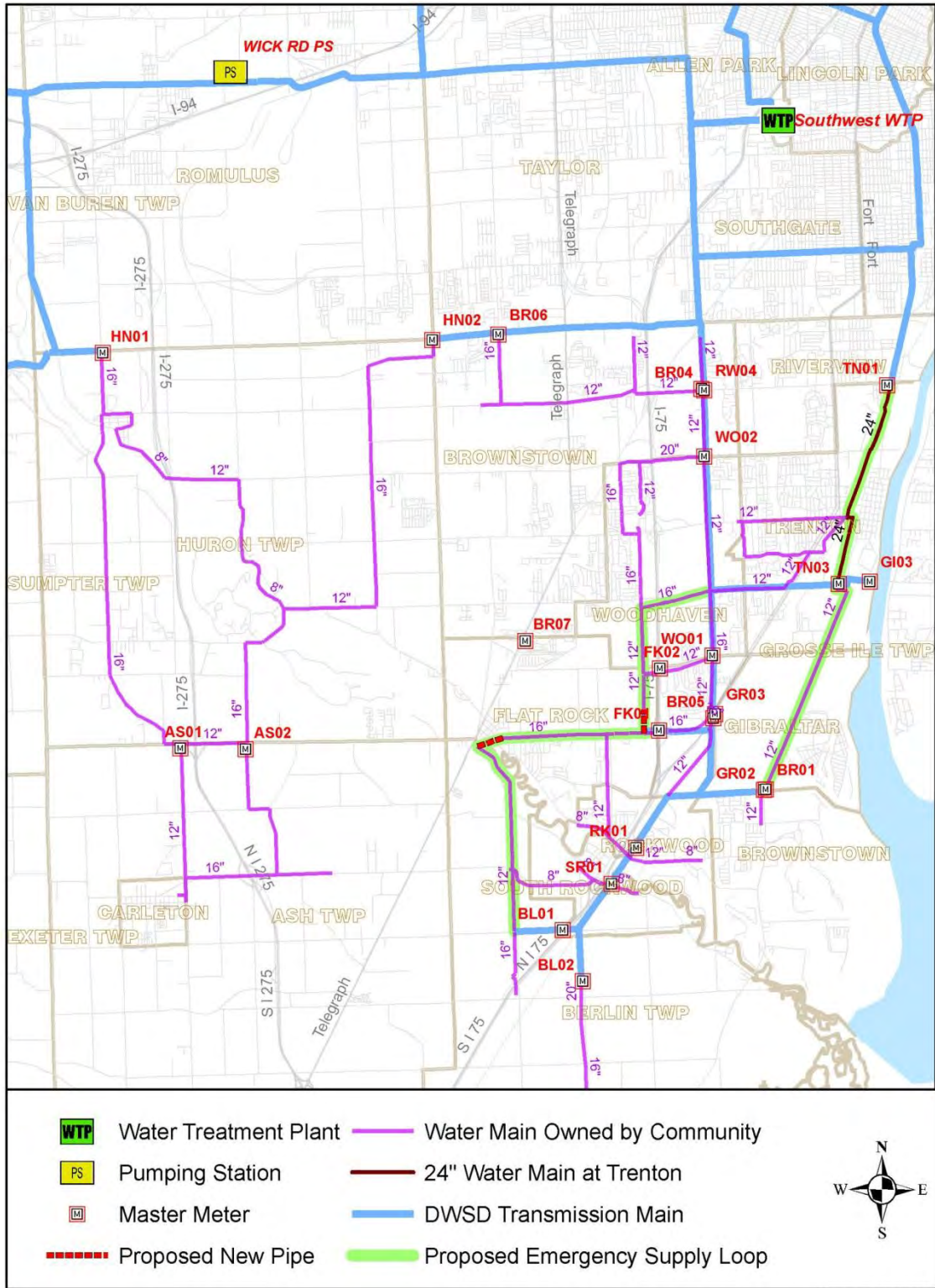


Figure 7-9: Alternative 7 – Use Community Distribution Pipes for Emergency Service

7.5 Energy Management

7.5.1 Introduction

DWSD's transmission system was originally designed to provide peak hour demand to most wholesale customers. The new Model Contract for wholesale water service introduced in 2007 was designed to incentivize wholesale customers to manage peak hour demand on the transmission system and promote regional planning. In the years since 2007, several customers have completed peak hour storage projects, and others are planning to add storage in the near future.

The trend started in 2007 is beginning to change demand patterns in the transmission system. This change in demand patterns provides an opportunity for additional review and optimization of energy management practice at DWSD. Reducing energy use within the transmission system is an important element of long term cost control. Electric energy costs comprise over 40 percent of DWSD's costs for treatment, high lift pumping and booster pumping in the transmission system. Changes in energy management practice early in the planning period will yield long term annual savings in operating cost.

Concurrent with the development of this Master Plan Update, DWSD was moving forward to implement a number of energy management initiatives. DWSD hired a Certified Energy Manager to lead efforts for evaluating a new proposed DTE rate schedule (D11), which could provide significant savings for DWSD. In the two previous years, DWSD consolidated all of its individual DTE accounts under one master agreement which makes it eligible for the new D11 rate schedule.

To provide current data for DWSD's efforts, the master planning team has focused on three aspects of electrical energy cost reduction:

- Peak Electrical Demand and Power Factor
- Optimization of Delivery Points to Wholesale Customers
- Energy Recovery through Hydraulic Turbines

7.5.2 Peak Electrical Demand and Power Factor

Copies of DWSD electrical bills were obtained from DTE for the summer 2013 months. The time period varied for each facility, but generally covered the time period between early December 2013 and January 2014. A summary of the information is provided in Table 7-5.

These data are the basis for the preliminary analysis for this Master Plan Update. DWSD has subsequently began to develop more extensive data and analysis of energy management opportunities.

Table 7-5: Summary of Recent Electrical Energy Bills for Major DWSD Facilities

Facility	DTW Rate Schedule	Dec 2013 Power Factor	Maximum Demand (KW)	Maximum Demand Charge (65%) (KW)	Data Date
Lake Huron	D6	99%	18,155	11,801	8/23/2013
Northeast	D6	99%	11,169	7,260	7/15/2013
Springwells	D6	99%	10,252	6,664	6/24/2013
Southwest	D6	98%	3,878	2,521	6/18/2013
Water Works Park	D6	98%	4,495	2,922	6/24/2013
Adams Road	D6	99%	1,848	1,201	7/15/2013
Eastside	D3	85%			N.A.
Electric Avenue	D3	88%			N.A.
Ford Road	D6	84%	1,037	674	8/23/2013
Franklin	D6	97%	2,217	1,441	7/16/2013
Haggerty	D3	80%			N.A.
Imlay	D6	87%	8,555	5,561	5/30/2013
Joy	D6	98%	1,940	1,261	5/20/2013
Michigan	D6	80%	209	136	7/17/2013
Newburgh	D6	91%	998	649	6/24/2013
North Service Center	D6	98%	5,472	3,557	7/15/2013
Northwest	D6	<i>Not available</i>			
Orion	D6	<i>Not available</i>			
Rochester	D3	49%			N.A.
Schoolcraft	D6	82%	1,015	660	8/19/2013
West Chicago	D3	54%			N.A.
West Service Center	D6	98%	1,858	1,208	8/26/2013
Wick	D6	85%	1,346	875	8/8/2013
Ypsilanti	D3	100%			N.A.

All of DWSD's facilities are either billed under the DTE Rate Schedule D3 or D6. The D3 rates are secondary service where DTE owns the transformer and DWSD receives voltage from the secondary side of the transformer at 120/240 Volts, 208/120 Volts, or 480/277 Volts. DTE will provide D3 service up to 1,000KW (or 1,176 KVA). The D6 and D6.1 rates are primary service where DTE brings the primary feed to the transformer. The transformer is owned by DWSD and they agree to a minimum load of not less than 50 KW for D6 or 10,000 KW for D6.1.

Both D6 rates have maximum demand charges equal to 65% of the highest KW use over the previous 11 months. Peak electrical demand is measured as the highest kilowatt use over a 30 minute period during the peak demand hours of the day (7:00 a.m. to 7:00 p.m.).

The Power Factor (PF) at the facilities is also monitored. The Power Factor is an indicator of how well the voltage and current is used at the facility. When electric motors are less efficient, the shift between the voltage and current use can become large. If this shift becomes too large then other customers on the DTE line are negatively impacted with their power feeds. DTE assesses a penalty for its D6 Schedule customers if the power factor falls below 85%. Customers with PFs less than 70% must install corrective equipment such as VFDs or PF correction capacitors.

7.5.3 Findings on Demand Charges and Power Factors

Electrical service costs represent over 25 percent of the Water Operations annual budget. Electrical service peak demand charges are typically about 25 percent, or more, of an individual facility's monthly electrical energy costs. Therefore, reducing demand charges is a means of reducing electrical service costs.

Reviewing Table 7.5, it would be expected that the maximum demand charge for the facilities would typically occur in the summer months. However, the maximum demand charge for two of these facilities (Imlay and Joy Road) occurred in May. It could have been due to an operational need within the transmission system, related to ongoing construction at one of these facilities or an adjacent facility. The cause that triggered the peak electrical demand charge could be useful to guide operating practice in the future.

The Maximum Demand Charge is a constant monthly charge for each month that follows the event that triggers the maximum demand. The Maximum Demand Charge can account for 20 to 30 percent of the total bill at times. DTE currently sets this charge at a rate of \$14.34/kilowatt for 65% of the peak usage in the preceding 12 months. A recent evaluation by the Financial Advisor to the DWSD Board of Water Commissioners showed that the Imlay Station that is bypassed during the off season paid over \$500,000 in fees associated with the difference between the peak demand charge and the actual peak use for each month.

Under the D6 schedule, three of the facilities have Power Factors less than 85%. They are Ford Road, Michigan Avenue, and Schoolcraft. DWSD can avoid cost penalties by improving the power factor at these facilities. Rochester and West Chicago have power factors below the 70% limit where DTE requires corrective actions.

DTE will perform an analysis for all of DWSD facilities to ensure that each station is on the most cost-effective rate schedule. The last known time this was done was in 2010. Based on the identified booster pumping stations to remain in operation, it will be beneficial for the Department to have another DTE analysis performed.

7.5.4 Delivery Point Optimization

Several wholesale customers are served by multiple supply meters over a range of pressures. The multiple supply points evolved over a period of time. Changes in population served, customer installed storage, and recent improvements and operation of the transmission system are reasons to review delivery points, and change past operating practice to reduce energy costs.

A case study of Redford Township’s delivery points illustrates the potential for optimizing delivery to maintain pressures required by wholesale customers, but reduce the transmission system energy to meet those pressure requirements.

Figure 7-10 shows the existing delivery of water to Redford Township. The Township has 10 supply meters but currently takes the majority of its water from Meter RD09 at the northern end of the City.

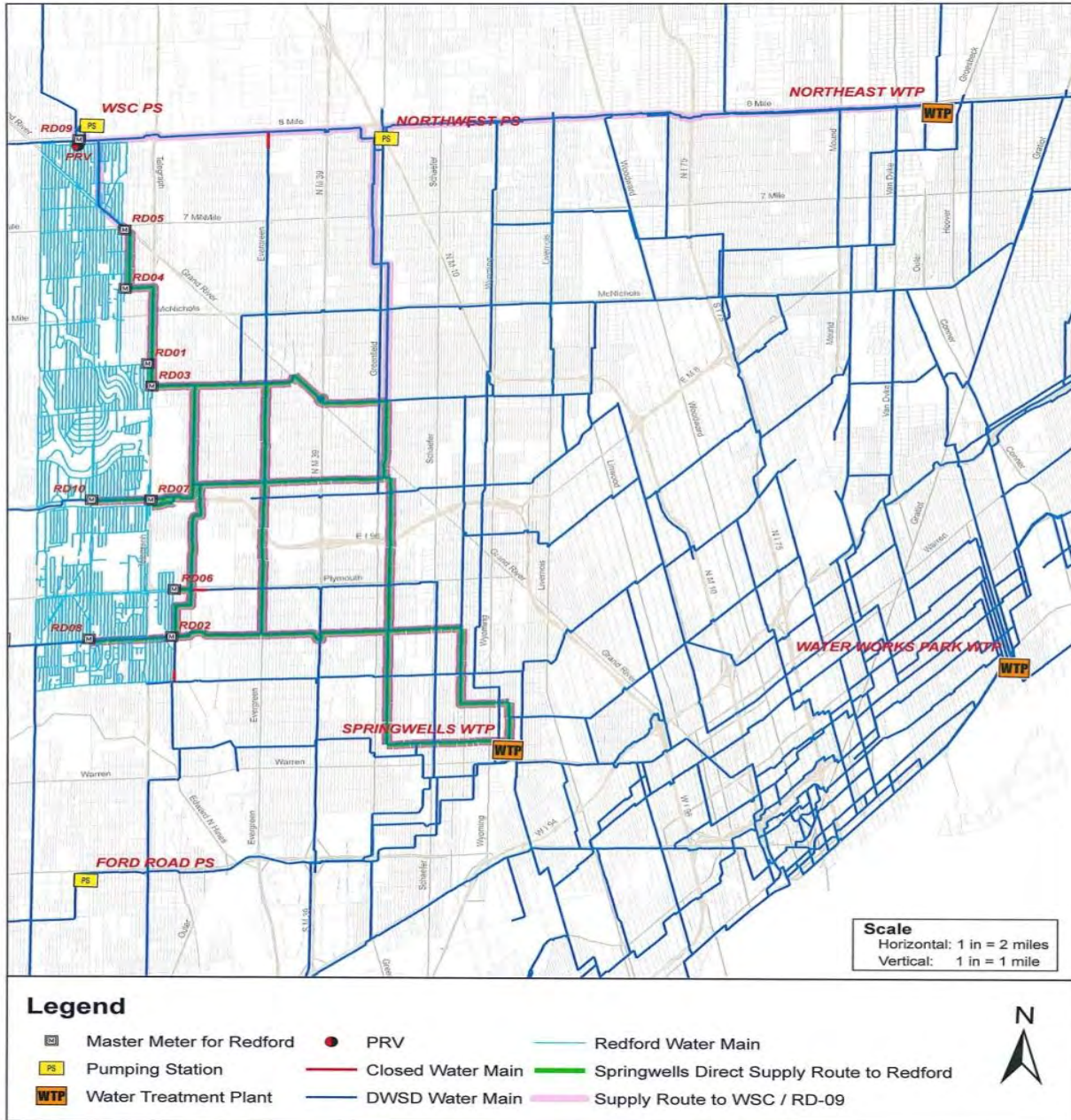
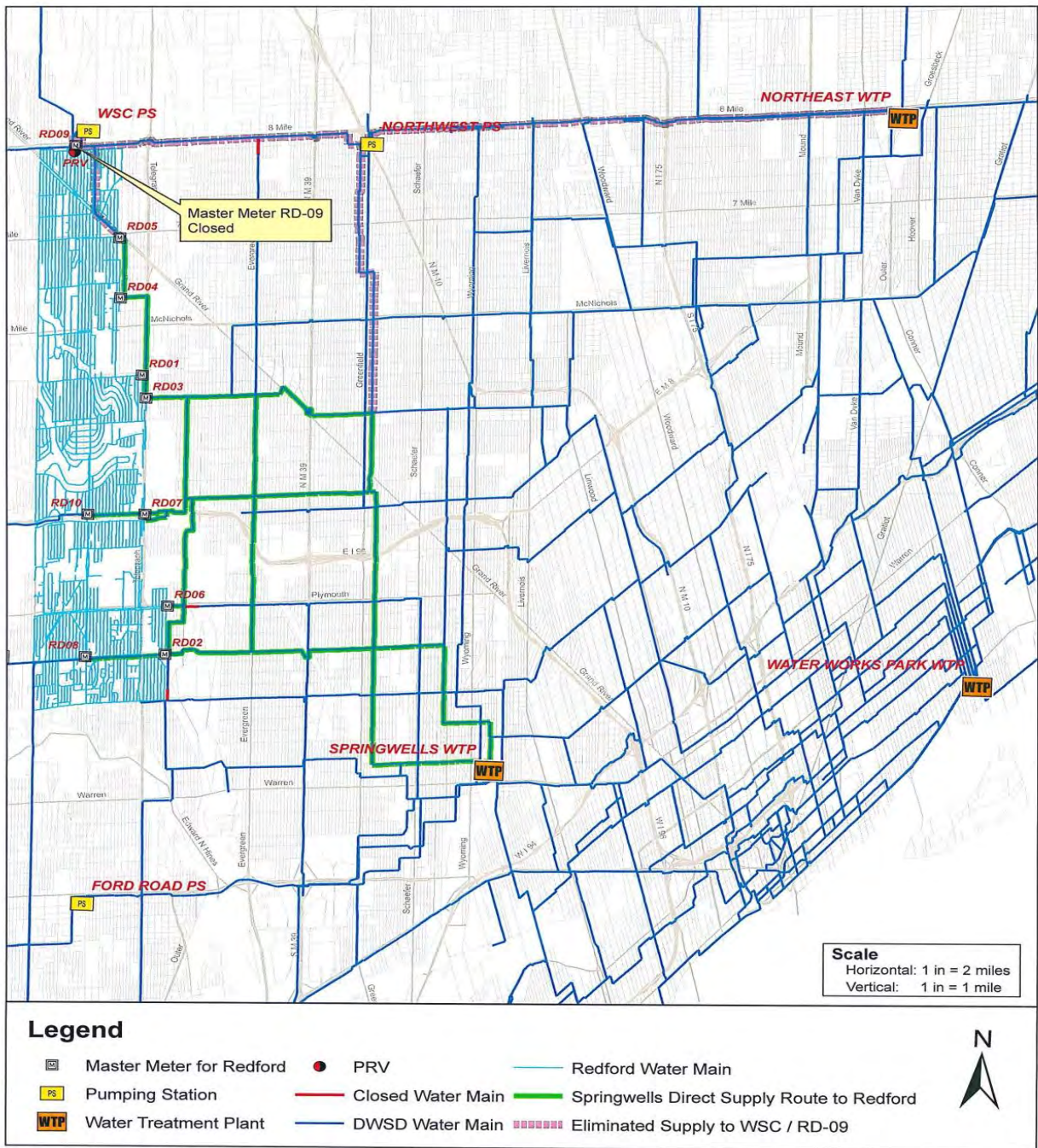


Figure 7-10: DWSD Water Transmission Energy Optimization Study, Water Supply to Redford Township - Existing Operations

This meter has the highest contract pressure requirement of all of Redford’s meters. An evaluation was performed of the feasibility of delivering water only through Redford’s other nine meters, as shown in **Figure 7-11**.



* The model simulation was based on 2011 system average day demand (471.1 MGD), Redford average day demand (4.76 MGD) and minimum pressure of 35 PSI maintained throughout the Redford distribution network.

Figure 7-11: DWSD Water Transmission Energy Optimization Study, Water Supply to Redford Township - Modified Operations

Tables 7-6 and 7-7 present a preliminary evaluation of cost savings for DWSD.

Table 7-6: Comparison of Master Meter Pressure and Flowrate between Existing and Modified Operations

Master Meter ID	Location	Existing Transmission Operations					Modified Operations (Changed Supply from WSC to RD-09)				
		Pressure from Model (PSI)			Flow from Model		Pressure from Model (PSI)			Flow from Model	
		Min	Max	Avg	Flow (MGD)	Percent	Min	Max	Avg	Flow (MGD)	Percent
RD-01	PURITAN/TELEGRAPH	49	76	64	0.00	0.03%	54	75	64	0.00	0.04%
RD-02	CHICAGO/W PARKWAY	62	85	75	0.31	6.54%	60	77	69	1.69	35.61%
RD-03	TELEGRAPH/MIDLAND	50	76	64	0.00	0.07%	55	72	64	0.02	0.52%
RD-04	BENETT/FIVE POINTS	43	71	59	0.03	0.55%	53	70	62	0.00	0.00%
RD-05	GRAND RIVER/7 MILE	40	68	56	0.03	0.54%	53	70	62	0.00	0.00%
RD-06	PLYMOUTH/W PARKWAY	56	81	56	0.06	1.17%	57	74	66	0.18	3.82%
RD-07	SCHOOLCRAFT/TELEGRAPH	53	79	67	0.01	0.18%	62	79	71	0.19	4.04%
RD-08	BEECH-DALY/W CHICAGO	60	80	72	0.79	16.49%	58	75	68	2.23	46.86%
RD-09	8 MILE/MACARTHUR (PRV)	70	109	94	3.39	71.18%	86	109	98	0.00	0.00%
RD-10	SCHOOLCRAFT/BEECH-DALY	54	83	70	0.15	3.25%	58	75	67	0.43	9.10%

Table 7-7: Comparison of Energy Consumption and Annual Energy Savings

Pumping Facility	Flow (MGD)	Avg. Press. (PSI)	Avg. Power (kW)	Daily Energy Use (kW)	Flow (MGD)	Avg. Press. (PSI)	Avg. Power (kW)	Daily Energy Use (kW)	Price (\$/KWh)	Energy Saving (KWh)	Percent Saving	Cost Saving
Northeast WTP	87.36	75.36	2649	63,570	87.19	75.34	2,643	63,434	8.411	49,732	0.21%	\$4,183
Springwells WTP High	102.65	80.45	3323	79,743	102.92	80.18	3,320	79,684	8.411	21,314	0.07%	\$1,793
West Service Center Int.	24.22	45.99	448	10,753	21.19	50.08	427	10,250	8.411	183,770	4.91%	\$15,457
Northwest Pump Station	No Pumping	N.A.	0	0	No Pumping	N.A.	0	0	11.150	0.00	0.00%	\$0

7.5.5 Energy Recovery

The DWSD water transmission system has 19 locations where water is delivered to reservoirs and then re-pumped. Every delivery to a reservoir is an opportunity to recover energy. Also, there are 76 locations where either DWSD or one of its wholesale customers operates a pressure reducing valve (PRV). Each pressure reducing valve is also an opportunity to recover energy.

A pilot study was performed to investigate the economic and engineering feasibility for retrofitting hydraulic turbines in DWSD booster pumping station facilities. Technical Memo 16 demonstrates a potential installation for a 30 kilowatt hydraulic turbine generator at the West Service Center.

An analysis of energy savings possible at Imlay Station is shown in **Table 7.8**. Imlay Station draws suction from the reservoirs, rather than the 120-inch main.

Drawing suction from the 120-inch main would save \$1.9 million per year.

Table 7-8. Energy Use Assessment at Imlay Pumping Station

Energy Loss Evaluation by Pumping Water from Reservoirs	
ADD Average Suction Pressure (PSI)	62
ADD Average Pressure in Reservoirs (PSI)	5
ADD Average Flow (MGD)	108
ADD Energy Loss (kWh)	59,514
MDD Average Suction Pressure (PSI)	55
MDD Average Pressure in Reservoirs (PSI)	5
MDD Average Flow (MGD)	300
MDD Energy Loss (kWh)	145,015
Annual Energy Loss* (kWh)	23,679,227
Energy Rate (\$/kWh)	0.08
Annual Energy Cost	\$1,895,000

* Annual energy loss is computed based on the monthly factors of average day and maximum day demand

7.5.6 Recommended Next Steps

Given the magnitude of energy costs in the annual O&M budget, GLWA should develop an energy management policy and procedures, as discussed in more detail in Chapter 11. Development of new policy and procedures should be based on established practice and case studies for the water industry. For example, the Water Environment Federation's *Energy Roadmap* provides a strategic approach to energy management and several case studies and methodologies.

Key features of energy management strategy should include:

- Strategic Management - policies, practices, measurable goals
- Organizational Culture – a shared vision within the organization regarding energy goals, and cross-functional teams empowered to achieve the goals

- Communication and Outreach – tools for two-way communication with operating staff and wholesale customers
- Demand-Side Management – methods to assess and reduce energy use and cost, bill, analysis, audits and real time monitoring
- Energy Generation – tools to evaluate feasibility, investment, and return on investment
- Continuing Innovation – assess results annually, refining goals, risk management, and further optimization study such as recommended in Section 7.9.1.8

7.6 Phase 2 Evaluations

The second phase of the planning effort included a needs assessment of booster pumping stations and reservoirs and hydraulic modeling of the transmission system projected average day demands and maximum day demands in 2035. Using the hydraulic model, a review was performed for needs for storage, transmission capacity improvements, and opportunities to reduce electrical energy costs for pumping. This analysis presumed that the Northeast water treatment plant would be repurposed, and the total capacity of the remaining four plants would be reduced to 1,040 MGD.

7.6.1 Booster Pump and Reservoir Needs Assessment

A series of meetings were held with DWSD staff and engineering consultants relative to renewal and replacement needs at booster pump stations and reservoirs. To facilitate these meetings, a list of major booster pumping and reservoir assets was created, and then discussion and documentation of needs proceed based on specific assets.

Table 7-9 includes the list of existing major assets for high lift pumping, booster pumping and reservoirs.

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Adams Road	109 MGD			
		Nitrogen Tanks		(2) Nitrogen Tanks
		HVAC System		
		Motor Control Center		Motor Controllers for Power Distribution Center
		Transformer		Tie Breaker, 4800V
		Switch Gear		
		VFD Drive		4800V VFD for LP #1 (installed 1992, no longer supported)
		Chiller System		
		Generators		(2) 4800V Emergency Generators
	18.20 MGD	LP #1		(4) Line Pumps
	18.20 MGD	LP #2		
	18.20 MGD	LP #3		
	18.20 MGD	LP #4		
	18.00 MGD	RP #1		(2) Reservoir Pumps
	18.00 MGD	RP #2		
		Reservoir	10 MG	Prestressed Wire-Wound Concrete Reservoir
		Sump Pumps		(2) Sump Pumps
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
East Side (CANYON)	30 MGD			
		HVAC System		
		Switchgear		
		Generators		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
East Side (CANYON)	10 MGD	RP #1		(3) Reservoir Pumps
	10 MGD	RP #2		
	10 MGD	RP #3		
		Reservoir	10 MG	Underground Concrete Reservoir
		Sump Pumps		(2) Sump Pumps
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
Electric Ave.	24.5 MGD	Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
		HVAC System		
		Transformer		Main T-1, Main T-2
		Switchgear		
		LP #1		(2) Line Pumps
		LP #2		
		RP #3		(2) Reservoir Pumps
	8.64 MGD	RP #4		
		Reservoir #1	6.6 MG	3.3 MG Above Grade PWW Reservoir
		Reservoir #2		3.3 MG Above Grade PWW Reservoir
		Sump Pumps		(2) Sump Pumps
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Ford Road	109 MGD			
		HVAC System		
		Motor Control Center		
		Switchgear		
		VFD Drive		
	18.14 MGD	LP #1		(5) Line Pumps
	10.08 MGD	LP #2		
	10.08 MGD	LP #3		
	10.08 MGD	LP #4		
	10.08 MGD	LP #5		
	10.08 MGD	RP #6		(5) Reservoir Pumps
	10.08 MGD	RP #7		
	10.08 MGD	RP #8		
	10.08 MGD	RP #9		
	10.08 MGD	RP #10		
		Reservoir	10 MG	Reservoir
		Sump Pumps		(2) Sump Pumps
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Franklin	164 MGD			
		HVAC System		
		Motor Control Center		
		Transformer		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Franklin		Switchgear		
		VFD Drive		4800V VFD for line pump 3
		VFD Drive		4800V VFD for line pump 4
		Soft Starters		(1) @ LP #1
		Generators		(2) 4800V Emergency Generators
	30 MGD	LP #1		(4) Line Pumps
	30 MGD	LP #2		
	30 MGD	LP #3		
	30 MGD	LP #4		*Acts as Line pump and Reservoir pump #3
	22 MGD	RP #1		(2) Reservoir Pumps
	22 MGD	RP #2		
		Reservoir	10 MG	Concrete semi-underground reservoir
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Haggerty Road	91 MGD			
		HVAC System		
		Motor Control Center		MCC No. 1 - 480V
		Transformer		
		Switchgear		
		VFD Drive		4160V VFD for Line pump 1
		VFD Drive		4160V VFD for Line pump 2
		VFD Drive		4160V VFD for Line pump 3 & Reservoir pump 3
	21 MGD	LP #1		(3) Line Pumps

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Haggerty Road	21 MGD	LP #2		
	21 MGD	LP #3		*Acts as Line pump and Reservoir pump #3
	14 MGD	RP #1		(2) Reservoir Pumps
	14 MGD	RP #2		
		Reservoir	10 MG	At Grade Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Imlay	575 MGD			
		HVAC System		
		Motor Control Center		
		Motor		P2 - 13,800V, P4 - 13,800V, P5 - 13,800V
		Transformer		
		Switchgear		
		VFD Drive		13,800V VFD for Pump 1
		VFD Drive		13,800V VFD for Pump 6
		VFD Drive		13,800V VFD for Pump 7
		VFD Drive		13,800V VFD for Pump 8
		VFD Drive		13,800V VFD for Pump 3
		Soft Starters		(3) @ LP #2, #4, #5
		Chiller System		
		Generator		Emergency Generator
	75 MGD	LP #3		(6) Line Pumps

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)	
Imlay	70 MGD	LP #4			
	70 MGD	LP #5			
	70 MGD	LP #6			
	70 MGD	LP #7			
	70 MGD	LP #8			
	75 MGD	RP #1		(2) Reservoir Pumps	
	75 MGD	RP #2			
			Fill System		8 electric motor operated cone valves with associated isolation BFVs, 4 manually-operated gates, with the system being on Ovation for control
			South Bypass		2 - 54" check valves in parallel and associated isolation BFVs with Ovation connected for remote monitoring
			West Bypass		24" pressure sustaining valve with associated isolation gates (2)
		Storm Lift Sta.		Storm drainage lift station	
		Reservoir	20 MG	Reservoir (actual capacity is about 17 MG)	
		Main Building			
		Sump Pumps		(2) Sump Pumps	
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas	
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics	
Joy Road	94 MGD				
		HVAC System			
		Motor Control Center			
		Transformer			
		Switchgear			

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Joy Road		VFD Drive		
		Soft Starters		(2) @ LP #2, #3
		Generators		(2) 4800V Emergency Generators
	15.84 MGD	LP #1		(3) Line Pumps
	15.84 MGD	LP #2		
	14.80 MGD	LP #3		
	16.13 MGD	RP #1		(3) Reservoir Pumps
	16.13 MGD	RP #2		
	14.80 MGD	RP #3		
		Reservoir #1	10 MG	5 MG Reservoir
		Reservoir #2		5 MG Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Lake Huron WTP	480 MGD	Capacity		Total Capacity is 480 MGD; Firm Capacity is 420 MGD.
		High Lift PS #1		Eight vertical 60 MGD pumps driven by synchronous electric motors. Station constructed for 20 pumps. Existing pumps are in opening positions for 2-9.
	60 MGD	High Lift PS #2		Pumps 2, 8 & 9 installed in 2000 and are three stage pumps rated at 415 ft TDH
	60 MGD	High Lift PS #3		Pumps 3-7 are original pumps (refurbished in 98-01), four stage pumps rated at 415 ft TDH
	60 MGD	High Lift PS #4		
	60 MGD	High Lift PS #5		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Lake Huron WTP	60 MGD	High Lift PS #6		
	60 MGD	High Lift PS #7		
	60 MGD	High Lift PS #8		
	60 MGD	High Lift PS #9		
		Reservoir #1	15 MG	Clearwells 1 and 2 below grade and are 15 MG each
		Reservoir #2	15 MG	
		Reservoir #3	14 MG	Clearwell 3 is above grade and is 14 MG
Michigan Ave	29 MGD			
		HVAC System		
		Motor Control Center		MCC No. 1 - 480V; MCC No. 2 - 480V
		Transformer		
		Switchgear		
	3.60 MGD	LP #1		(3) Line Pumps
	3.60 MGD	LP #2		
	4.32 MGD	LP #3		
	8.64 MGD	RP #4		(2) Reservoir Pumps
	8.64 MGD	RP #5		
		Reservoir	3.5 MG	Steel Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Newburgh	52 MGD			
		HVAC System	NA	

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Newburgh		Motor Control Center		
		Transformer		
		Switchgear		
	8 MGD	LP #1		(5) Line Pumps
	8 MGD	LP #2		
	12 MGD	LP #3		
	12 MGD	LP #4		
	12 MGD	LP #5		
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
	Northeast WTP	608 MGD		
		High Lift PS #1		Twelve single-stage, vertical centrifugal, double volute pumps
		High Lift PS #2		
		High Lift PS #3		
		High Lift PS #4		
		High Lift PS #5		
		High Lift PS #6		
		High Lift PS #7		
		High Lift PS #8		
52 MGD		High Lift PS #9		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Northeast WTP	52 MGD	High Lift PS #10		
	50 MGD	High Lift PS #11		
	50 MGD	High Lift PS #12		
	50 MGD	High Lift PS #13		
	49 MGD	High Lift PS #14		
	52 MGD	High Lift PS #15		
	52 MGD	High Lift PS #16		
	52 MGD	High Lift PS #17		
	49 MGD	High Lift PS #18		
	50 MGD	High Lift PS #19		
	N	50 MGD	High Lift PS #20	
		Reservoir #1	8 MG	One 8 MG reservoir under the filters
		Reservoir #2	15 MG	Two 15 MG each subsurface reinforced concrete reservoirs north of the HLPS
		Reservoir #3	15 MG	
North Service Center	271 MGD			
		HVAC System		
		Motor Control Center		MCC No. 1 - 5 480V
		Transformer		
		Switchgear		
		VFD Drive		4800V VFD Drive for Line pump 7
		VFD Drive		4800V VFD Drive for Line pump 8
		VFD Drive		4800V VFD Drive for Line pump 9
		VFD Drive		4800V VFD Drive for Line pump 10
		Generators		(4) 4800V Emergency Generators; (1) Portable
	LP #1		Out of Service	
	LP #2		(9) Line Pumps are in service	

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
North Service Center	19.30 MGD	LP #3		
	23 MGD	LP #4		
	19.30 MGD	LP #5		
	19.30 MGD	LP #6		
	30 MGD	LP #7		
	30 MGD	LP #8		
	30 MGD	LP #9		
	30 MGD	LP #10		
	15 MGD	RP #1		(4) Reservoir Pumps
	15 MGD	RP #2		
	20 MGD	RP #3		
	20 MGD	RP #4		
		Reservoir #1	20 MG	10 MG Reservoir
		Reservoir #2		10 MG Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Northwest	50 MGD			
		HVAC System		
		Switchgear		
	10 MGD	RP #1		(5) Reservoir Pumps
	10 MGD	RP #2		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Northwest	10 MGD	RP #3		
	10 MGD	RP #4		
	10 MGD	RP #5		
		Reservoir	10 MG	Underground Concrete Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Orion	14 MGD			
		HVAC System	NA	
		Motor Control Center		MCC No. 1 - 480V
		Transformer		
		Switchgear		
	2 MGD	LP #1		(4) Line Pumps
	4 MGD	LP #2		
	4 MGD	LP #3		
	4 MGD	LP #4		
		Main Building (Temporary)		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
	Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics	

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Rochester	72 MGD			
		HVAC System	NA	
		Motor Control Center		
		Transformer		
		Switchgear		
		VFD Drive		4160V VFD Drive for Line pump 1
		VFD Drive		4160V VFD Drive for Line pump 3
	14.40 MGD	LP #1		(5) Line Pumps
	14.40 MGD	LP #2		
	14.40 MGD	LP #3		
	14.40 MGD	LP #4		
	14.40 MGD	LP #5		
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Roseville	21 MGD			
		HVAC System	NA	
		Motor Control Center		MCC No. 1 - 480V
		Transformer		
		Switchgear		
	3 MGD	LP #1		(4) Line Pumps
	3 MGD	LP #2		
	5 MGD	LP #3		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Roseville	10 MGD	LP #4		
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Schoolcraft	80 MGD			
		HVAC System		
		Transformer		
		Switchgear		
		Generators		(2) 4800V Emergency Generators
	20 MGD	LP #1		(3) Line Pumps
	20 MGD	LP #2		
	20 MGD	LP #3		*Acts as Line pump and Reservoir pump #2
	20 MGD	RP #1		(1) Reservoir Pump
		Reservoir	10 MG	Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics, etc.
Southwest WTP	305 MGD			

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Southwest WTP	55 MGD	High Lift PS #1		Seven pumps driven by synchronous electric motors, cone-type discharge control valves with hydraulic oil valve operators
	55 MGD	High Lift PS #2		
	25 MGD	High Lift PS #3		
	30 MGD	High Lift PS #4		
	30 MGD	High Lift PS #5		
	55 MGD	High Lift PS #6		
	55 MGD	High Lift PS #7		
		Reservoir #1	10 MG	Diameter: 338' Depth: 15'
		Reservoir #2	10 MG	
		Reservoir #3	10 MG	
Springwells WTP	780 MGD			
	40 MGD	HLP#1		(4) High lift pumps located in the high pressure district
	40 MGD	HLP#2		
	40 MGD	HLP#3		
	40 MGD	HLP#4		
	50 MGD	HLP#5		(4) High lift pumps located in the intermediate pressure district
	50 MGD	HLP#6		
	50 MGD	HLP#7		
	50 MGD	HLP#8		
	60 MGD	HLP#9		(2) High lift pumps located in the intermediate pressure district
	60 MGD	HLP#10		
	60 MGD	HLP#11		High lift pumps located in the high pressure district

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Springwells WTP	60 MGD	HLP#12		
	60 MGD	HLP#13		
	60 MGD	HLP#14		
	60 MGD	HLP#15		
		Reservoir #1	21 MG	
		Reservoir #2	20 MG	
		Reservoir #3	22 MG	
Water Works WTP	640 MGD			
				DCS/Ovation
	80 MGD	HLP		High lift pumps installed in 1962
	80 MGD	HLP		
	80 MGD	HLP		
	60 MGD	HLP		
	60 MGD	HLP		
	60 MGD	HLP		
	60 MGD	HLP		
	60 MGD	HLP		
	40 MGD	HLP		
	40 MGD	HLP		
	40 MGD	HLP		
	40 MGD	HLP		
	2.4 MGD	RP #1		
	2.4 MGD	RP #2		
	2.4 MGD	RP #3		
		Reservoir #1	8 MG	

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Water Works WTP		Reservoir #2A	5.8 MG	
		Reservoir #2B	5.9 MG	
West Chicago	36.5 MGD			
		(Asset Data not listed)	NA (Res. has been removed)	(Asset Data not listed)
West Service Center PS	266 MGD			
		HVAC System		
		Motor Control Center		MCC No. 1 - 5 208V
		Transformer		
		Switchgear		
		Generators		(2) 4800V Emergency Generators
	30 MGD	LP #1		(6) Line Pumps
	30 MGD	LP #2		
	30 MGD	LP #3		
	28.80 MGD	LP #4		
	29.50 MGD	LP #5		
	29.50 MGD	LP #6		
		VFD		
		Discharge Division Valves		
	24 MGD	RP #1		(4) Reservoir Pumps
	24 MGD	RP #2		
	20 MGD	RP #3		

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
West Service Center PS	20 MGD	RP #4		
		Reservoir #1	20 MG	Water Storage Reservoir
		Reservoir #2		Water Storage Reservoir
		Main Building		
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Wick Road	72 MGD			
		HVAC System		
		Motor Control Center		
		Nitrogen Tanks		(2) Nitrogen Tanks
		Transformer		
		Switchgear		
		Generators		(2) 4160V Emergency Generators
	18 MGD	LP #1		(3) Line Pumps
	18 MGD	LP #2		
	12 MGD	LP #3		*Acts as Line pump and Reservoir pump #3
	12 MGD	RP #1		(2) Reservoir Pumps
	12 MGD	RS #2		
		VFD		L1, L2, L3/R3
		Reservoir #1	10 MG	Buried concrete Reservoir
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas

Table 7-9: Major Pumping and Reservoir Assets

Station/ Reservoir	Pumping Capacity	Asset (1)	Storage	Asset Description (1)
Wick Road		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics
Ypsilanti	54 MGD			
		HVAC System	NA	
		Transformer		
		Switchgear		
		Nitrogen Tanks		(2) Nitrogen Tanks
		VSDs		(3) VSDs on all three line pumps
		Soft Starters		(3) @ LP #1, #2, #3
		Generator		(1) Emergency Generator
	18 MGD	LP #1		(3) Line Pumps
	18 MGD	LP #2		
	18 MGD	LP #3		
		Temporary Building		Existing Temporary Building
		Main Building		
		Sump Pumps		(2) Sump Pumps
		Grounds/Fences/Gates		Perimeter fencing, gates, driveways & parking areas
		Primary Instruments		Magnetic flow meters, level transmitters, pressure transmitters, vibration transmitters, motor temp transmitters, power monitoring, motor protection electronics

(1) Major Assets are in BOLD as listed in DWSD Asset Management Data Base "BOOSTER STATIONS AND SEWAGE STATIONS -ALL ASSETS (EXCLUDING DELETED AND INACTIVE) as of November 2013

The needs assessment results are documented in TM-14 in the appendix. The results of the needs assessment have been incorporated in the preliminary 20-year CIP in TM-17.

7.6.2 Hydraulic Modeling

A system-wide master planning hydraulic model was developed to simulate Year 2035 demands. The model was created based on an operational model of the system developed by DWSD under a concurrent contract. The model was updated by the master planning team with new information on nonrevenue water and recent and proposed system improvements.

The hydraulic model uses WaterGEMs modeling software. There are approximately 25,000 pipes in the model network. Demands were developed for each wholesale meter and for districts of retail meters based on calculated domestic, commercial, institutional, and major industrial demands, plus allowances for nonrevenue water. Demand patterns for each meter were developed from a review of 3 years of demands and are documented in TM-3. All modeling demands are consistent with the wholesale customer projections documented in TM-15. Additional discussion of the hydraulic model is presented in TM-11.

The hydraulic model was used to examine a range of service delivery options. The results of the modeling are presented below through the description of proposed transmission improvement programs.

7.7 Proposed Transmission Programs

Four programs are identified for improvements to the transmission system:

- Program to Decommission Certain Booster Pumping Stations
- Program to Optimize Service Delivery
- Program to Improve Transmission Redundancy
- Program for Renewal and Reliability

Leak detection programs are discussed in Chapter 9.

Accompanying this master plan report is a 20-year Capital Improvement Plan (CIP) spreadsheet that lists approximately 350 CIP projects. These projects are organized by program, including the four programs listed above for transmission, plus other programs for transmission, distribution, and metering and non-revenue water.

7.8 Program to Decommission Certain Booster Pumping Stations

The requirements for booster pumping stations were examined using the 2035 MDD model. There are 20 booster pumping stations in the transmission system. 2035 modeling results show that potentially 4 stations could be bypassed. These are generally older stations where present day demand and projected future demand are lower than the demands when the original stations were built.

1. The Electric Ave Station has been used infrequently in the last 5 years. It could be permanently decommissioned based on the MDD model simulation while still meeting wholesale customer contract pressures. The Southwest High Lift station would be used to meet wholesale customer pressure requirements.
2. The Michigan Ave Station is a single water transmission to supply the City of Wayne and some master meters for Westland, Romulus, Garden City and Inkster. The upcoming Glenwood transmission main project will not provide redundancy for the function of Michigan Ave Station. However, following the completion of the Glenwood project, an additional 24-inch transmission main on Newburgh Road from Cherry Hill to Glenwood would allow the decommissioning of the Michigan Avenue Station. This new transmission main would also provide redundant supply to this area. This \$5,000,000 project is included in the proposed 20-year CIP.
3. The West Chicago Station could be decommissioned if both Livonia and Westland agree to change flow splits for the Meters LV-15 (approximately 15% of total water supply to Livonia) and WL-06 (approximately 10% of total water supply to Westland). Per the model simulations, the supplies from the two meters can be compensated by the other master meters serving the two communities.
4. The Northwest Station and reservoir could be decommissioned with a change in flow split within the SOCWA system
5. The existing reservoirs at Northwest, Electric Avenue, and Michigan Avenue would also be decommissioned. The Electric Avenue and Michigan Avenue reservoirs are relatively small, and the Northwest reservoir is a below grade reservoir. There is sufficient storage and transmission redundancy in each of the operating zones where these reservoirs are located, particularly if reservoirs operated by SOCWA could be used in an emergency to supply water outside of SOCWA's service area.
6. **Table 7-10** shows current flow splits for customers served by these stations.

The following approach is proposed for DWSD to work with its customers to examine opportunities for changing flow splits to reduce future capital and annual operating costs:

1. Schedule meetings with the community to present DWSD's interest in reapportioning the community's supply during the 3rd quarter of 2015.
2. Present the need to close down the particular facility (condition of the facility, the cost to renovate, energy costs, etc.).
3. Ask their engineer to analyze the new distribution of flow in their local model and identify any improvements that may be needed (if any) within their community. Complete these analyses by the end of 2015.
4. Develop an approach that addresses the community's need for peak supply, non-peak supply, and redundancy. Complete this during the first quarter of 2016.

The current CIP includes \$45,000,000 for upgrades to the Electric Avenue, Northwest, Michigan Avenue and West Chicago. After deducting the \$5,000,000 for the new 24" main along Newburgh Road between Cherry Hill Road and Glenwood Avenue, the net savings would be approximately \$40,000,000. Annual energy cost savings, including reduction in demand charges based on the current rate schedule, are estimated to be \$300,000 to \$500,000 annually.

Figure 7-12 shows alternative locations for proposed new transmission mains to replace the Michigan Avenue booster station. One alternative is a 24-inch main on Newburgh Road between Cherry Hill and Glenwood Avenue. A second alternative is a 30-inch main on Merriman Road. The Newburgh Road alternative allows for the bypass of the Michigan Avenue Pump Station and provides a redundant transmission supply to the City of Wayne, which is currently supplied only by single source. The Merriman Road alternative provides a more direct supply and improved pressure from the Ford Road booster station, but it does not provide the redundant loop at the west end of the Michigan Avenue main. Both alternatives should be examined during design, along with a third alternative to create a loop from a main in Hannan Road extending from the south and supplied from Wick Road booster station.

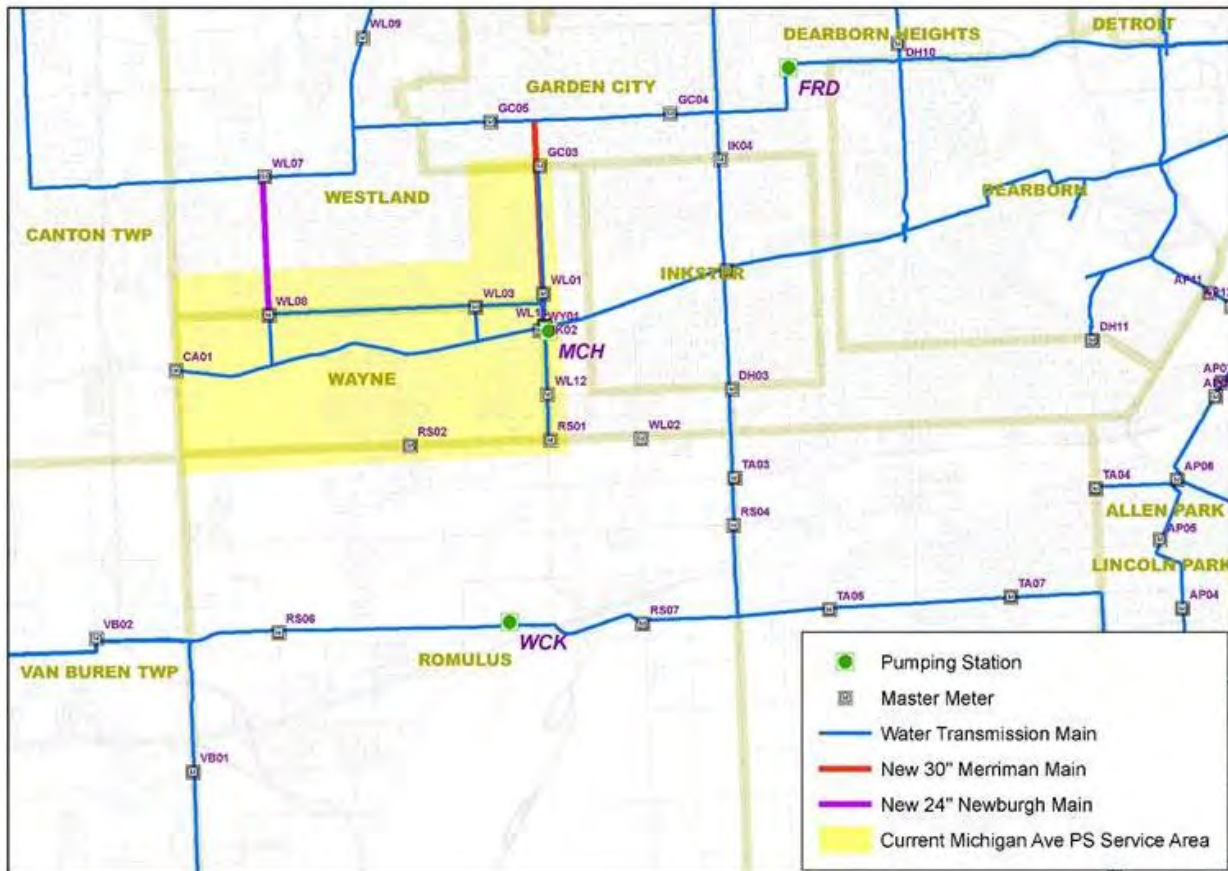


Figure 7-12: Proposed Alternative Mains on Newburgh Road or Merriman Road to Replace the Michigan Avenue Booster Station

7.9 Program to Optimize Delivery to Certain Customers

The program to optimize delivery is targeted at those customers who are supplied water at pressures greater than needed for the customer's service area. Some supply pressures are dictated by requirements of adjacent customers, and some supply pressures may be high due to the transmission main from which the customer is served.

Table 7-11 presents an analysis of opportunities to optimize service at lower pressure to certain customers. Changes to the customer supply point or to the transmission system are identified, capital costs are estimated, along with energy savings and payback period.

This program also includes projects required to maintain adequate chlorine residual for Lapeer City and Mayfield. These customers are located on the western end of the 72-inch main to Flint, and the water age will be in excess of 20 days when Genesee County discontinues its water purchases from DWSD in July 2016. DWSD is currently performing bench scale tests to determine the requirements for chlorine booster stations. If chlorine booster stations are required, then a design build project would be awarded later in 2015 to build the new facilities.

Figures 7-13 and 7-14 show the projected service areas of the four water treatment plants in 2035 for average daily demand and maximum day demand. The new service areas will increase the service area for the Lake Huron and Water Works Park plants. Finished water supply from Lake Huron will be at a higher pressure, and three customers – Madison Heights, Sterling Heights and Troy – have meters in critical locations relative to the new service areas. **Table 7-11** shows the analysis of meters and anticipated new pressures. All of the meters are within or very close to existing contract limits. Therefore, it is recommended that no new PRVs be installed, but instead monitor pressures as the new service areas are implemented and assess the need for PRVs or changes in contract limits in the future.

Table 7-10: Booster Pump Stations and Existing Flow Splits

Booster Station	Affected Community	Supply Percentage	Affected Master Meters
West Chicago	Livonia	15%	LV-15
West Chicago	Westland	10%	WL-06
Northwest	S.O.C.W.A.	25%	SE-05, 07
Michigan Avenue	City of Wayne	100%	WY-01
Michigan Avenue	Romulus	4%	RS-01, 02
Michigan Avenue	Westland	16%	WL-01, 03, 08, 12, 13
Michigan Avenue	Garden City	24%	GC-03
Michigan Avenue	Inkster	0%	Emergency Supply Meter, IK-02

Table 7-11: Analysis of Meters and Anticipated New Pressures for the New Lake Huron Service Area

Community	Meter	Model Simulation		Contract Limits	
		Min	Max	Lower	Upper
MADISON HEIGHTS	_MH01	55	62	51	78
	_MH02	57	67	53	80
STERLING HEIGHTS	_ST02	58	71	55	77
	_ST03	71	86	59	90
	_ST04	61	78	49	79
	_ST05	60	71	56	82
	_ST06	74	91	68	89
	_ST07	73	89	69	90
	_ST08	70	86	61	83
	_ST09	58	73	50	73
	_ST10	64	75	65	86
	_ST11	43	60	31	54
	TROY	_TY01	60	75	50
_TY03		56	68	51	77
_TY04		125	136	114	149
_TY06		98	102	87	111
_TY07		86	89	72	96
_TY08		119	131	104	134

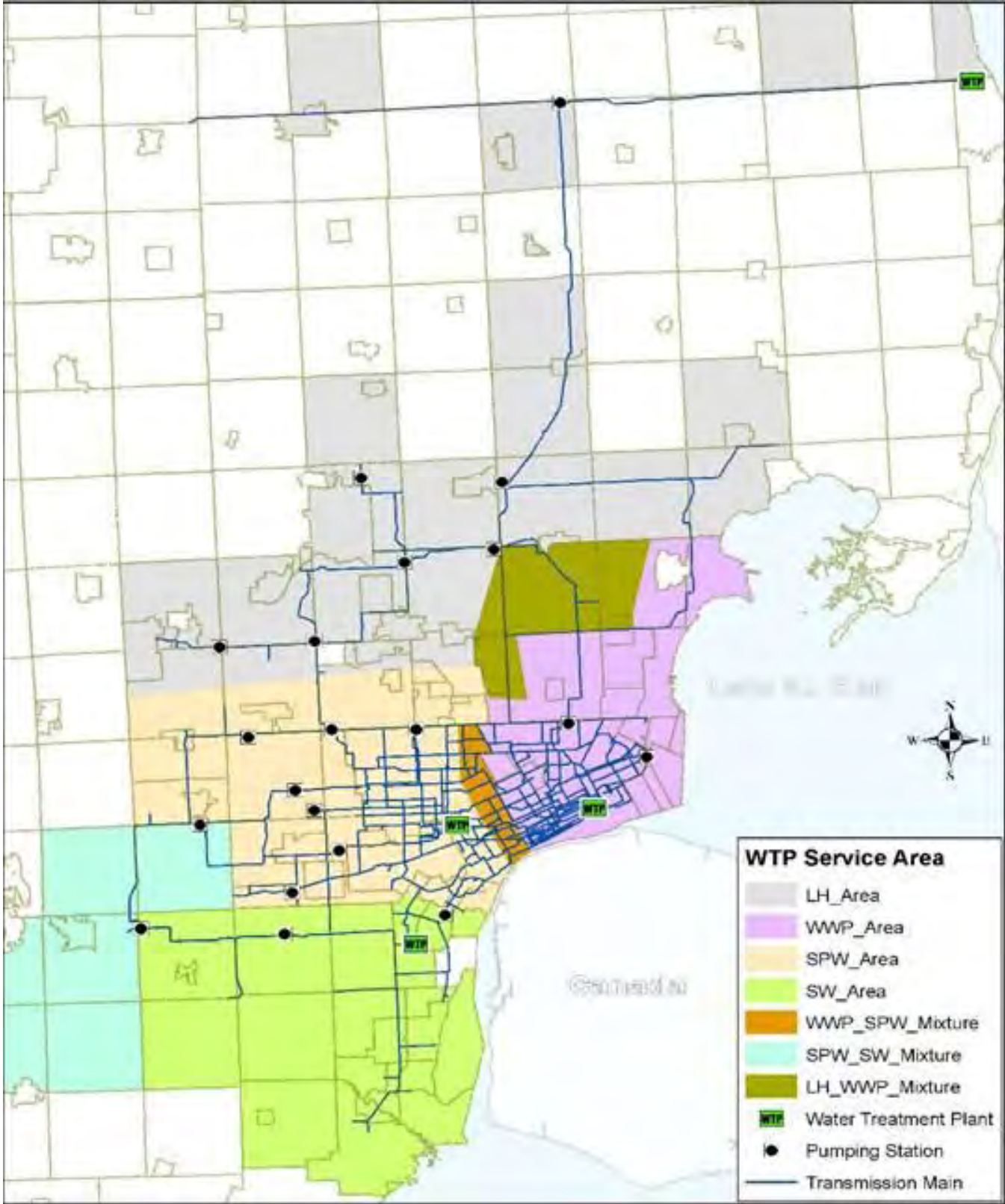


Figure 7-13: 2035 Service Areas for Average Daily Demand

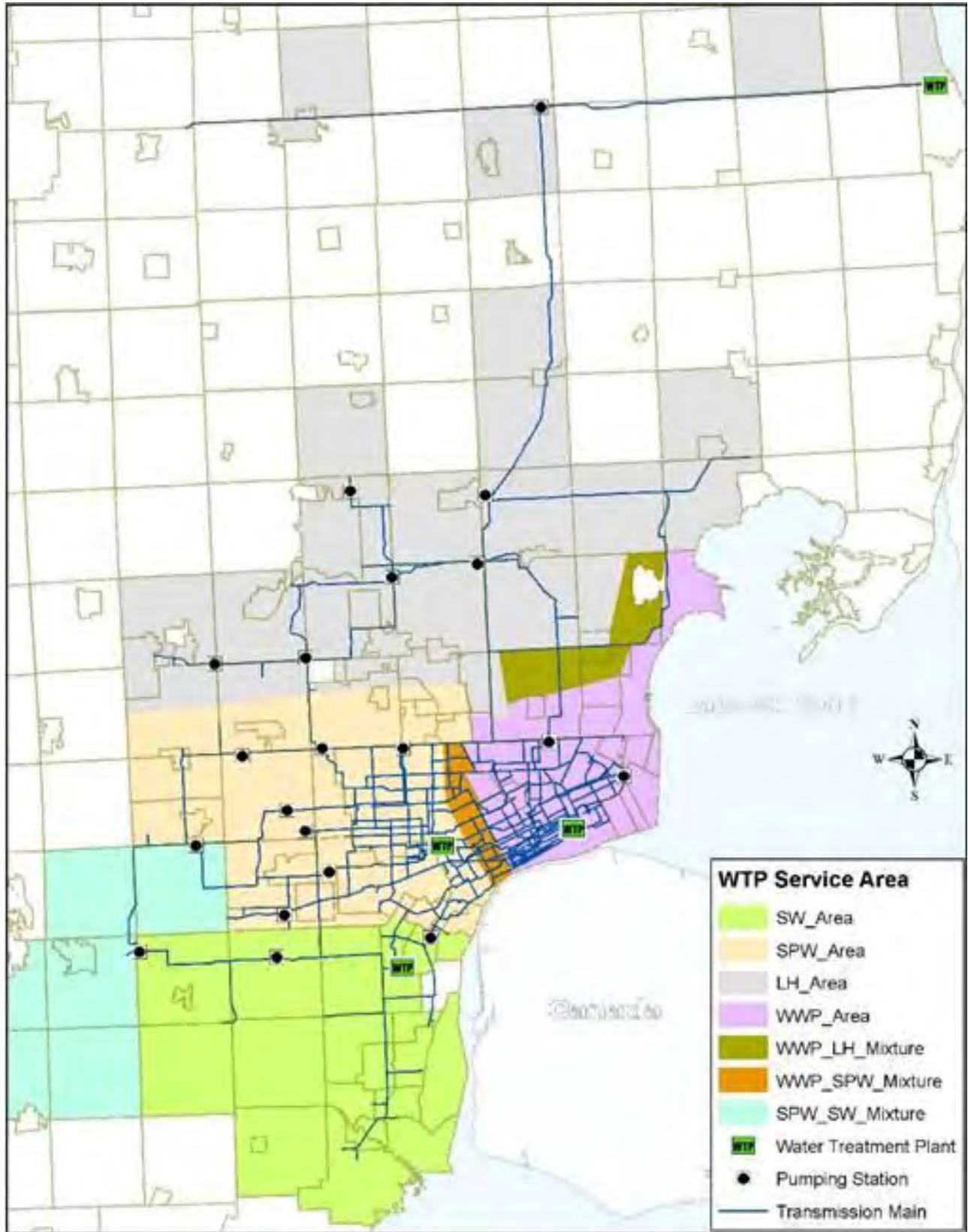


Figure 7-14: 2035 Service Areas for Maximum Daily Demand

7.9.1 Energy Evaluation Based on Seasonal Adjustment (Non-Peak Period)

An energy evaluation was conducted to identify communities that may have excess pressure within their system. This analysis is based on an assumed 35 psi minimum pressure at the highest point of service within the community and the associated headloss from the master meters to the high point.

If a community was found to have excess pressure then the lower contract pressure limit was adjusted by the excess amount and energy savings within the system was calculated. If capital improvements were required then the costs were identified and the associated payback period was computed.

Generally, the energy savings were based on supplying the lower contract pressures during the non-peak period (October through April) and then supplying pressures within the contract limits during the summertime when demands are highest.

The energy evaluation is considered preliminary since each individual system was not modeled as part of this analysis. It is recommended that savings that provide a payback period of less than 25 years should be investigated further with the communities.

The findings for each community with potential excess energy are summarized in **Table 7.11**. A description of each is provided below.

7.9.1.1 Macomb Township

24 Mile Road is the main supply to Macomb Township. By 2035 two parallel mains will supply the area. The lowest pressure supplied to this main is currently controlled by Rochester Hills and communities north of 24 Mile Road. The energy analysis for this area is based on splitting the discharge header at Rochester Station and additional piping so that the two mains along 24 Mile Road could be separated. The one main would supply Rochester Hills and the communities north of 24 Mile Road while the other main would supply Macomb Township.

7.9.1.2 Keego Harbor/Slyvan Lake

These communities are supplied by the 42-inch loop between the Adams Road and Franklin Pump Stations. The pressure for the area is controlled by West Bloomfield master meters. To supply a lower pressure to Keego Harbor and Slyvan Lake would require a pump station at West Bloomfield to boost its pressure. The estimated payback period exceeds 35 years, so it is not considered cost-effective.

7.9.1.3 Utica

Utica is supplied by the 24-inch main along Utica Road and Auburn Road. This main also supplies two of Shelby Township's meters which control the pressure along Utica Road. Savings could be achieved by constructing a main from Auburn south to M-59. **Figure 7-15** shows the proposed new supply main for Utica.



Figure 7-15: Proposed New Supply to Utica

In conjunction with the change to service for Utica, a hydraulic turbine should be evaluated for the North Service Center cone valves, which throttle pressure from Imlay Station. In the current operation, the energy dissipated through the cone valves has a value of \$1,600,000 annually. Some of this energy loss will be reduced after the service area for the Lake Huron plant is extended to the south of the North Service Center. A new hydraulic turbine would take the place of one or more cone valves and recover approximately \$400,000 in renewal energy annually.

7.9.1.4 Plymouth

This community is supplied from the Sheldon main. To realize the energy savings supplying the City of Plymouth, pump stations would be required to supply Plymouth and Northville Townships.

7.9.1.5 Gibraltar/South Rockwood

These communities are supplied via the Allen Road main. This main is controlled by the contract pressures to Brownstown. To realize the energy savings supplying Gibraltar and South Rockwood a pump station would be required to supply the Brownstown supply north of these communities.

7.9.1.6 River Rouge

River Rouge is supplied via the main along Jefferson. This main is controlled by the contract pressures to Melvindale. To realize the energy savings supplying River Rouge a pump station would be required to supply Melvindale at ME-03. However, since ME-03 only accounts for 10% of the supply to Melvindale, it should be investigated whether this meter could be closed. Closing the meter would realize the energy savings without additional capital improvements in the transmission system.

7.9.1.7 Grosse Pointe Park

Energy savings supplying Grosse Pointe Park may be achieved by creating a new district with lower pressures in the Detroit area that supplies the community. The estimated cost is based on an assumed rehabilitation cost associated with valves required to isolate the area.

7.9.1.8 Summary

The energy optimization analysis performed for this Water Master Plan Update is a first step to using new data available through the wholesale meter systems, the hydraulic model, and its energy analysis features. An additional study is recommended to be performed jointly by DWSD and respective customers in operating zones where further optimization is considered feasible.

THIS PAGE INTENTIONALLY LEFT BLANK.

Table 7-12: Reducing Master Meter Supply Pressure for Communities with Excess Pressure

Region	Community	Master Meter ID	Existing Contract Lower Limit (psi)	Excess Pressure ⁽¹⁾ (psi)	Contract Lower Limit Change (psi)	Off-Peak Season energy Saving ⁽²⁾ (kWh)	On-Peak Season energy Saving ⁽²⁾ (kWh)	Energy cost saving ⁽³⁾ (\$)	CIP Cost (\$1,000)	Payback (Years)	Notes
2	Keego Harbor	KH-01	92	45	92 / 47	198,952	0	15,916	1,452	>35	Local pumping is required to boost the pressure for the West Bloomfield master meters to obtain the energy savings in Keego Harbor and Sylvan Lake
2	Sylvan Lake	SL-01	93	38	93 / 55						
3	Utica	UT-01	107	56	107 / 51	84,946	0	6,796	220	32	Build a new isolation valve and a redundancy pipeline to serve Utica.
4	Plymouth	PT-02	65	31	65 / 34	233,000	0	18,640	577	31	Local pumping required to boost pressure for Northville and Plymouth Township to obtain the energy savings in City of Plymouth.
		PT-03	125	31	125 / 94						
		PT-04	75	31	75 / 44						
6	Gibraltar	GR-02	61	8	61 / 53	637,140	0	50,971	742	15	Local pumping is required to boost the pressure for Browntown Township.
	GR-03	57	8	57 / 49							
6	South Rockwood	SR-01	60	9	60 / 51						
6	River Rouge	RR-01	58	10	58 / 48	260,663	0	20,853	113	6	Local pumping is required to boost the pressure for Melvindale, ME-3. If ME-3 can be closed (10% of supply) then no capital improvements required.
		RR-02	54	10	54 / 44						
		RR-03	53	10	53 / 43						
7	Grosse Pointe Park	GK-01	59	12	59 / 47	302,301	92200	31,560	500	16	Annual energy savings obtained by isolating a portion of the WWP Intermediate Zone. Approximately 20 isolation valves are required to be closed to isolate the area.
		GK-02	59	12	59 / 47						
		GK-03	60	12	60 / 48						

THIS PAGE INTENTIONALLY LEFT BLANK.

7.10 Program to Improve Transmission Redundancy

This program includes continuing work with NOCWA on the Adams Branch and with the Downriver customers to resolve their respective needs for redundancy for the single transmission mains that serve their areas. Capital improvement projects are included in the capital improvement plan in TM-17 to provide the recommended redundancy or pumping station upgrades.

Table 7-13 presents an analysis of customers served by single transmission mains and proposed means to create redundancy through inter-customer emergency connections.

7.10.1 Chesterfield Pump Station

This program includes the construction of a new pumping station and reservoir in Chesterfield near 23 Mile Road. This proposed pumping station provides an alternative route to supply water north of 24-Mile Road in the event of a break on the 96-inch main south of 24-Mile Road. A new reservoir in Chesterfield also provides storage in region of the system where there is relatively little regional storage compared to other parts of the service area. The design concept for this facility is based on the recommendations of the 2004 Water Master Plan.

7.10.2 Downriver

This program includes a short reach of new parallel 24-inch main on Allen Road between Pennsylvania and Van Horn. Also included is a hydraulic modeling study by DWSD to confirm the concept plan for interconnections between communities shown in **Figure 7-9**. Following the modeling study, it is anticipated that the respective wholesale customers would complete their interconnections as part of planned water main extensions and upgrades.

There is a question regarding the legal ownership of the existing 24-inch main between TN-01 and TN-03 in Trenton. This main was not listed on the inventory of facilities that were transferred to DWSD by Wayne County in 1960. This main is critical to the redundancy solution for Downriver, and from a system perspective, it would be operationally advantageous if this became a GLWA main after legal review of historic documents.

Table 7-13: Wholesale Customers Served by Single Transmission Mains

High Priority Master Meters*	All Master Meters on the Single line	Single Main	Whole 2035 Population Served	Whole Average Day Demand 2035 (MGD)	Master Plan Conclusion
WAMR_CM02, WAMR_CM01, WAMR_WA01, WAMR_NV04, WAMR_CM03, WAMR_NV05,W AMR_WX01	WAMR_FT08, WAMR_WB03, WAMR_FT09, WAMR_WB02, WAMR_FT10, WAMR_WB06, WAMR_WB07, WAMR_CM02, WAMR_CM01, WAMR_WA01, WAMR_NV04, WAMR_CM03, WAMR_NV05,W AMR_WX01	From Haggerty Station To Wixom	150,256	18.5	Investigate the use of adjacent community water systems during an emergency along 14 Mile Road west of the Franklin Station. The emergency plan should include the Haggerty and Franklin reservoir supplies.
WAMR_BR06, WAMR_BR04, WAMR_RW04, WAMR_WO02, WAMR_TN03, WAMR_WO01, WAMR_GR03, WAMR_BR05, WAMR_FK01, WAMR_GR02, WAMR_BR01, WAMR_RK01, WAMR_SR01, WAMR_BL02, WAMR_BL01	WAMR_BR06, WAMR_HN02, WAMR_BR04, WAMR_RW04, WAMR_WO02, WAMR_TN03, WAMR_GI03, WAMR_WO01, WAMR_GR03, WAMR_BR05, WAMR_FK01, WAMR_GR02, WAMR_BR01, WAMR_RK01, WAMR_SR01, WAMR_BL02, WAMR_BL01	From Southwest Plant To Berlin Township	98,247	9.7	Recommendations have been made for wholesale customer interconnections plus DWSD transmission project with total cost of \$5 million.
WAMR_YT01	WAMR_YT01	From Ypsilanti Station To Ypsilanti	90,124	8.2	YCUA to determine if communities can be supplied by remaining master meters if YT-01 is out of service. Explore parallel feed to YCUA, YT-01.
WAMR_CT04	WAMR_CT04	To Clinton Township	54,396	5.5	Clinton to determine if community can be supplied by remaining master meters if CT-04 is out of service. Explore parallel feed to CT-04.
WAMR_AH05, WAMR_OT01	WAMR_AH05, WAMR_OT01	From Orion station To Orion	28,443	4.6	Inter community redundancy improved by NOCWA

Table 7-13: Wholesale Customers Served by Single Transmission Mains

High Priority Master Meters*	All Master Meters on the Single line	Single Main	Whole 2035 Population Served	Whole Average Day Demand 2035 (MGD)	Master Plan Conclusion
WAMR_WB08, WAMR_KH01, WAMR_SL01	WAMR_WB08, WAMR_KH01, WAMR_WB01, WAMR_SL01	To Keego Harbor	36,413	4.4	Investigate the use of adjacent community water systems to develop an emergency operations plan for the service to Keego Harbor.
WAMR_PO01	WAMR_PO01	To Pontiac	33,860	4.1	Inter community redundancy improved by NOCWA
WAMR_NH01, WAMR_LX01, WAMR_LX02	WAMR_CH02, WAMR_NH01, WAMR_LX01, WAMR_LX02, WAMR_CH03	To Lenox Township	57,729	4.7	Investigate Chesterfield's ability to serve Lenox and New Haven during an emergency. Investigate emergency interconnect with the New Baltimore water system.
WAMR_EC01	WAMR_EC01	To Ecorse	7,656	2.9	Investigate emergency supply from adjacent communities. Investigate emergency interconnect with the Wyandotte water system.
WAMR_SS04, WAMR_SS05, WAMR_SS03	WAMR_ED02, WAMR_SS04, WAMR_HW06, WAMR_SS05, WAMR_SS03, WAMR_GW02, WAMR_GW03	To Grosse Pointe Wood	32,952	2.9	Communities to investigate whether their systems can be supplied by their other master meters during an emergency. Investigate use of adjacent community water systems for supply during an emergency.
WAMR_AH04, WAMR_AH03, WAMR_AH06	WAMR_AH04, WAMR_AH03, WAMR_RC02, WAMR_PO02, WAMR_AH06	From Adam Station To Auburn Hills	52,060	7.5	Inter community redundancy improved by NOCWA
WAMR_WY01, WAMR_WL03	WAMR_WY01, WAMR_WL03, WAMR_WL08, WAMR_WL12, WAMR_RS01, WAMR_WL01, WAMR_WL06	To Michigan Avenue Station	37,221	4.1	Recommendation for new 24" D.I. Pipeline along Newburgh Road between Cherry Hill Road and Glenwood Avenue

Table 7-13: Wholesale Customers Served by Single Transmission Mains

High Priority Master Meters*	All Master Meters on the Single line	Single Main	Whole 2035 Population Served	Whole Average Day Demand 2035 (MGD)	Master Plan Conclusion
WAMR_LA02, WAMR_LA01, WAMR_LA03	WAMR_IC01, WAMR_LA02, WAMR_LA01, WAMR_LA03	From Imlay Station To Flint	23,152	2.4	Lapeer has wells for back up supply.
WAMR_SU01	WAMR_VB05, WAMR_VB04, WAMR_SU01	To Van Buren Township	14,180	1.1	Van Buren to investigate whether their system can be supplied by their other master meters during an emergency. Investigate use of adjacent community water systems for supply to Superior Township.
WAMR_GI01	WAMR_GI01	To Grosse Ile Township	6,773	0.7	Grosse Isle to investigate whether their system can be supplied by their other master meters during an emergency.
WAMR_UT01	WAMR_SY06, WAMR_SY01, WAMR_UT01	To Utica	12,273	1.3	Investigate emergency service from Shelby Township or Sterling Heights. Installation of 16" main for energy reduction would also provide needed redundancy.

7.10.3 New Isolation Gates for 96-inch Main

The installation of additional isolation gates on the 96-inch main will require that the main be depressurized to approximately 30 psi for safety to during the installation of line stops.

While the main is depressurized, it will be necessary to provide temporary booster pumping at each metered connection, and issue a boil water advisory. It is possible that interconnections between wholesale customer systems would minimize the number of temporary booster stations. This work should be scheduled for a non-winter month outside of the peak outdoor irrigation season, such as October through April, exclusive of the winter months.

There are two existing isolation gates: one is immediately north of the North Service Center, and one in north of Romeo in Bruce Township at McKay Road and 33-Mile Road.

New gates are proposed at five locations, as shown in **Figure 7-16**:

1. Almont Township, near 44-Mile Road, and south of the existing 16-inch blind tee at Graham Road and Rider Road

2. Bruce Township, near Bordman Road, and south of the existing 16-inch blind tee at Scotch Settlement Road and Bordman Road
3. Washington Township, near 30-Mile Road, and south of WC-01 at 31-Mile Road and GTWRR
4. Washington Township, near Brown Road, and north of the 16-inch tee serving SY-2, SY-7, and WC-2.
5. Shelby Township, near 24-Mile Road, north of the 24-inch tee serving the Rochester booster pump station.

If 60-inch isolation gates are installed, the estimated head loss is 0.5 psi per gate. This is negligible head loss for this transmission main.

The following is a preliminary sequence of work.

The work would be divided into two phases. Phase 1 would be for work north of the existing isolation gate at McKay Road and 33-Mile Road. Phase 2 would be for work south of that isolation gate. Phase 1 could be completed within approximately 7 working days with multiple shift operations. The boil water advisory would extend for a second week. Phase 2 could be completed within two weeks with a multiple shift operation. The boil water advisory would extend for a third week.

7.10.3.1 First Phase Isolation Gates

During the first phase, customers south of the isolation gate at McKay Road and 33-Mile Road would receive average day water supply from the North Service Center at or near contract pressures.

Prior to construction, perform detailed planning and scheduling with two customers -- Almont and Bruce Township -- regarding arrangements for temporary pumping and boil water advisories.

1. Establish temporary pumping at meter AC-1 and BU-1.
2. Establish work area for hot taps at approximately 1,000 feet north and south of the new isolation gate
3. Make provision for draining the 96-inch main at the temporary pumping locations.
4. Close the isolation gate at McKay Road and 33-Mile Road.
5. Continue to supply water from the NSC to points south of McKay Road and 33-Mile Road.
6. Cease pumping at Imlay Station. AC-1 and BU-1 will be supplied from the volume of water in the main.
7. Begin temporary pumping from AC-1 and BU-1. Drain water from the pipe as needed. Drop the pressure in the 96-inch main to a pressure acceptable for installation of hot taps.
8. Complete four hot tap installations.
9. Increase pressure to approximately 60 PSI in impacted area, continue temporary pumping.

10. Install line stops for new isolation gate for near Bordman Road.
11. Install new isolation gate near Bordman Road.
12. Remove line stops for Bordman Road.
13. Install line stops for new isolation gate for near 44-Mile Road.
14. Install new isolation gate near 44-Mile Road.
15. Remove line stops for 44-Mile Road.
16. Initiate pumping at Imlay Station.
17. Restore normal water service north of McKay and 33-Mile Road.
18. Perform water quality testing and inspections over required period, then lift boil water alert.
19. Total estimated time for temporary pumping is one week, for Boil Water Alert (BWA) is two weeks.

7.10.3.2 Second Phase Isolation Gates

During the second phase, customers north of the existing isolation gate at McKay Road and 33-Mile Road would receive average day water supply from Imlay Station at or near contract pressures.

Use an approach similar to Phase 1. Install the new isolation gates from north to south. As each new isolation gate is completed, move the boundary of supply from Imlay Station further south.

7.11 Program for Renewal and Reliability

This program includes continuing upgrades for transmission mains and booster pump stations and reservoirs to extend or renew service life. A series of projects for water transmission main condition assessment and rehabilitation are proposed, along with identified needs for transmission isolation valves, valve exercising projects, booster pump station upgrades and reservoir improvements and repairs. There are approximately 260 transmission system valves that are 20 inches in diameter or larger. DWSD should develop condition assessment protocols and best practices over the period 2016 to 2020 to guide this program over the planning period.

The implementation of the proposed transmission programs will allow DWSD to operate its system on an emergency basis with any one plant out of service. **Figures 7-17 to 7-20** show how average day demand could be supplied under different plant outage scenarios.

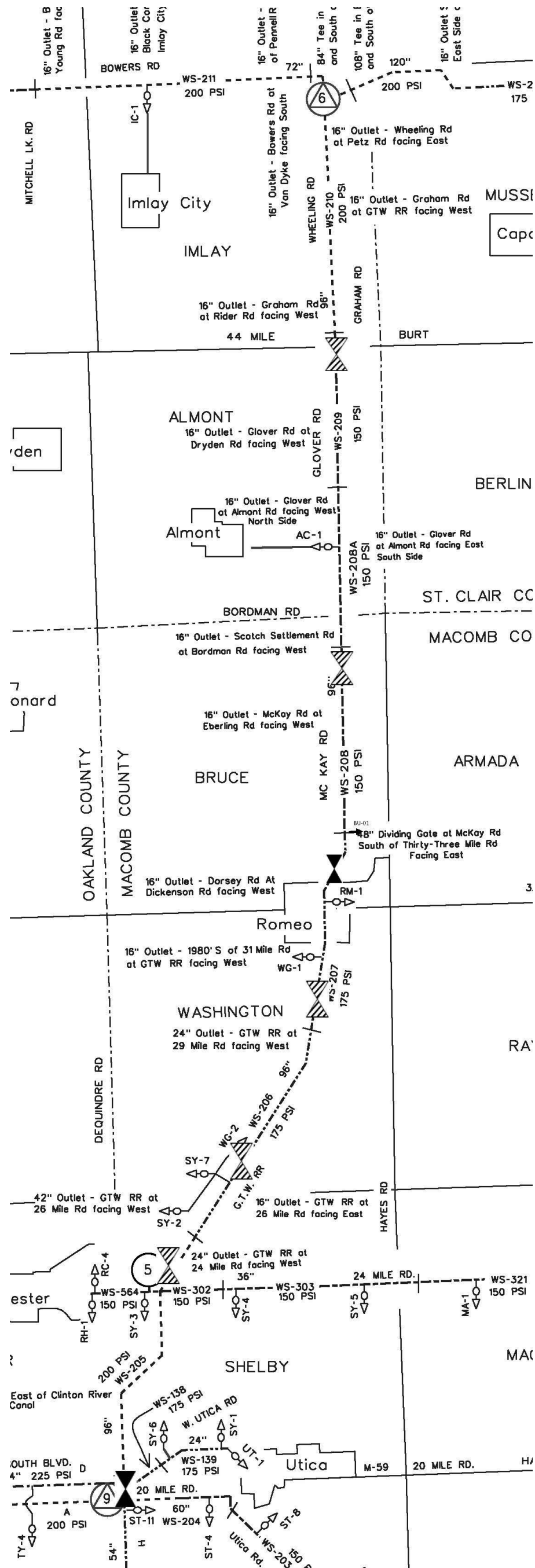


Figure 7-16: Proposed Isolation Valves for 96-Inch Transmission Main

THIS PAGE INTENTIONALLY LEFT BLANK.

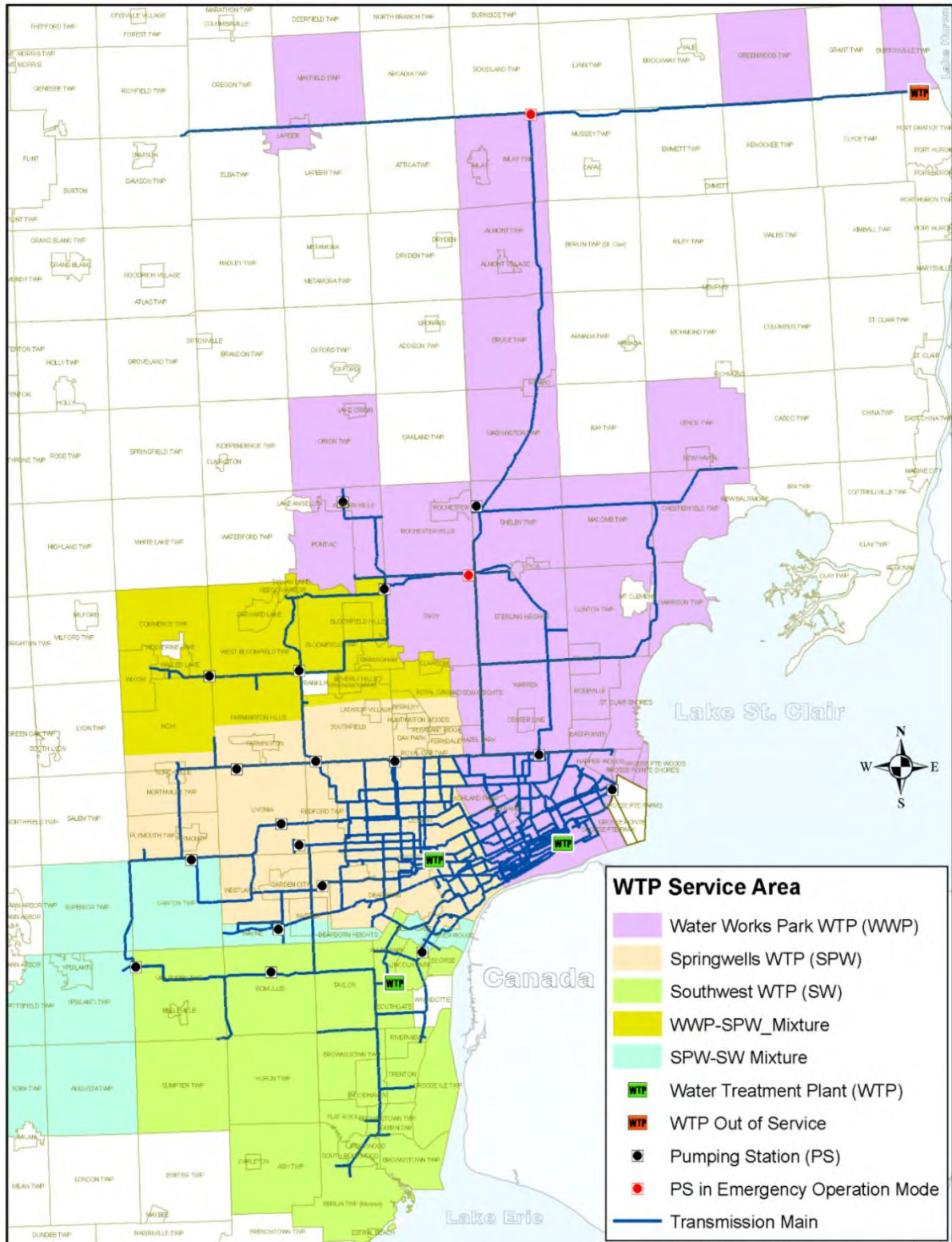


Figure 7-17: Average Day Demand with Lake Huron WTP Out of Service

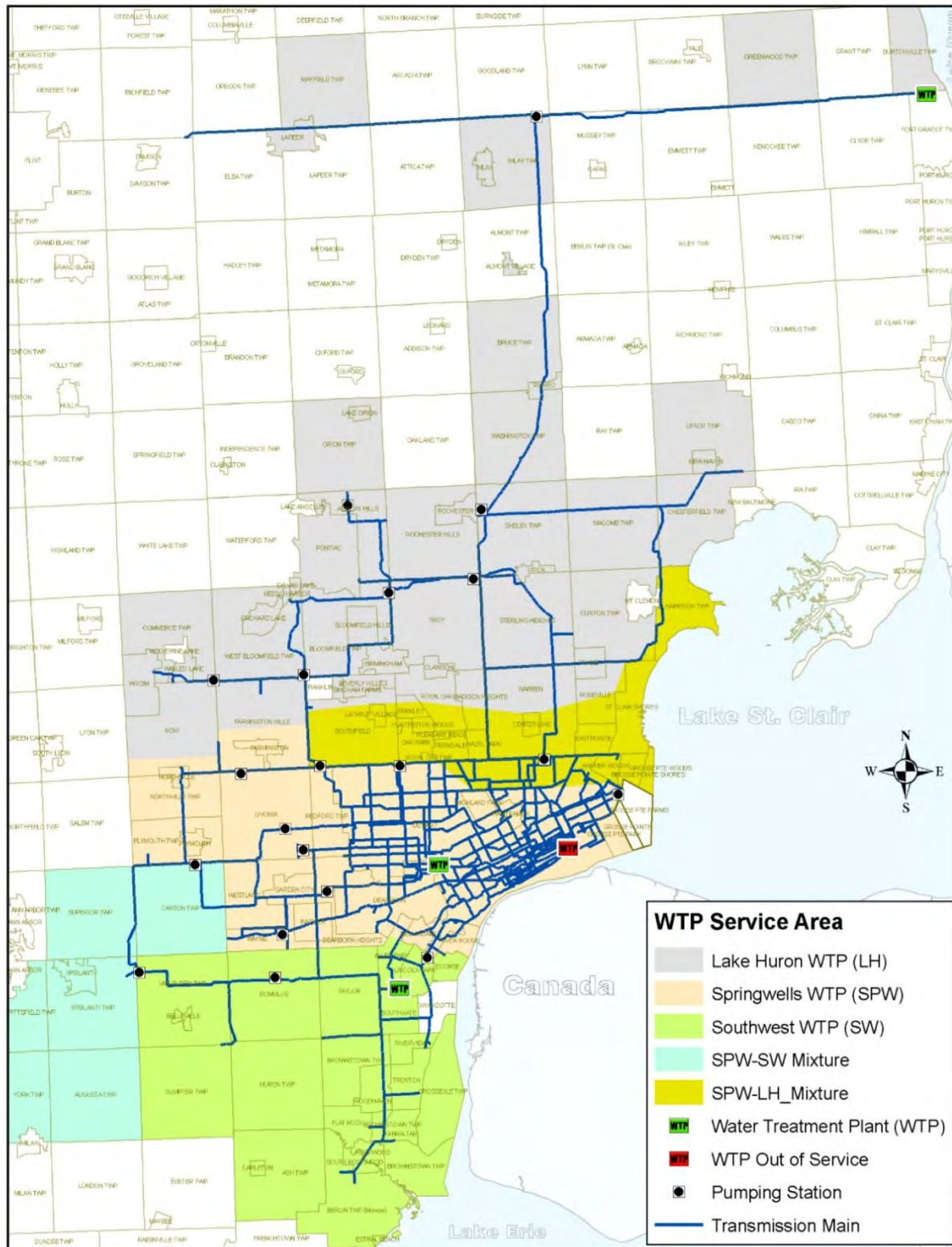


Figure 7-18: Average Day Demand with Water Works Park WTP Out of Service

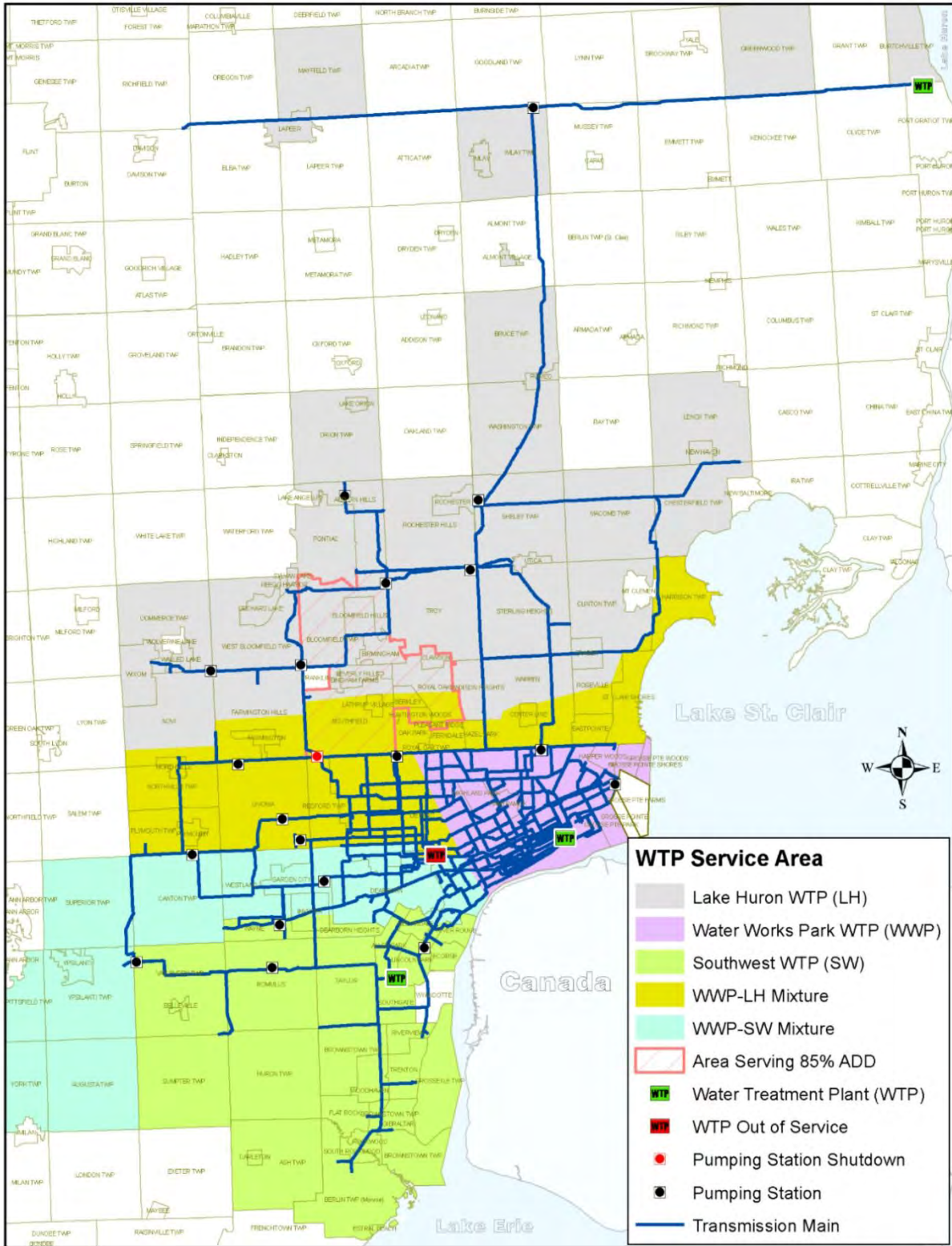


Figure 7-19: Average Day Demand with Springwells WTP Out of Service

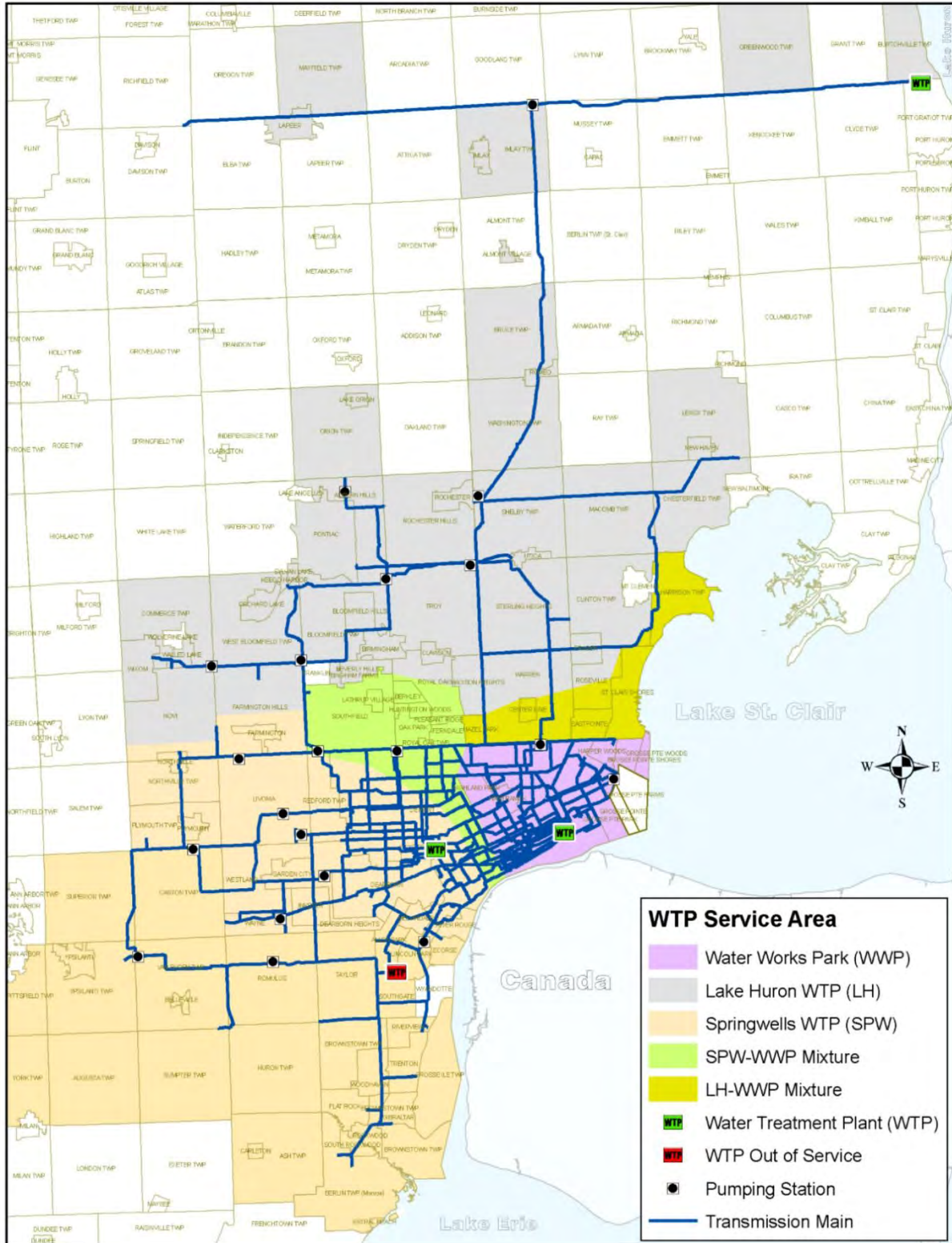


Figure 7-20: Average Day Demand with Southwest WTP Out of Service

8 Distribution

The master plan for Detroit's water distribution system is based on the findings of technical evaluations and input from the retail customer outreach program. The technical evaluations included a review of DWSD's past and current practice for water main rehabilitation, review of current and emerging practice in other cities, and discussions with DWSD Field Services Group and City of Detroit representatives. The retail customer outreach program included Retail Steering Committee and a series of neighborhood meetings conducted over the term of the master planning study.

With the anticipated completion of institutional arrangements by the end of January 2016, the new Great Lakes Water Authority will take responsibility for the regional water system and the City of Detroit will take responsibility for its retail water system within the city limits. The schedule for preparation of this Water Master Plan Update only allowed a limited opportunity to discuss the planning concepts with representatives of the City of Detroit. The planning team welcomes further discussion with City of Detroit representatives to refine this plan and incorporate new ideas.

This chapter covers the following topics:

- Customer Service
- Rehabilitation and Rehabilitation
- System Consolidation
- Lead Pipe Services
- Proposed Capital Improvement Plan

Findings and recommendations on nonrevenue water and metering, both retail and wholesale, are discussed in Chapter 9.

8.1 Retail Customer Service

The customer outreach program for this planning project included the formation of a Retail Customer Steering Committee and the participation of Detroit residents at a series of neighborhood meetings. Technical Memorandum 1 in the appendix describes the objectives, scope, and results from the retail customer outreach program. The program was constructive in identifying the following retail service priorities from the standpoint of citizens:

- Affordable water rates
- Assurance that customer service questions and requests are understood and will be addressed
- Responsiveness to water leak reports, particularly flooded basements in vacant buildings
- Maintain high standards for water quality and water distribution infrastructure

Retail customers regularly expressed concern regarding the high cost of the sewer service portion of their water bills. At the same time, most customers believe that the portion of the bill for water is reasonable and affordable.

DWSD has initiated a series of retail customer service initiatives over the last two years.

- Automated Meter Reading Program
- Fire Hydrant Winterization
- Assistance with Water Bill Payments
- Leak Detection and Repair Contracting
- Customer Service Center
- On-line Water Bill Payment
- Shut-off Program
- Budget Billing Program
- Water Main Break Repair Contracting

The water service shut-off program was the subject of intense public discussion in 2014. Updates to the program were made in the second half of 2014 and there have been significant improvements in retail water bill collection through 2014 and 2015.

Formation of the new DWSD-Retail organization is focusing attention on the needs and methods for continuing improvement in customer service. A responsive customer service program is an essential element of financial sustainability for retail water service.

DWSD has made improvements in its response to water main breaks and leaks. In FY2014, there were approximately 1,920 breaks compared to an average of 1,446 breaks in preceding years. The winter of FY2014 was extremely cold, and DWSD retained contractors to augment its own forces in break repair.

DWSD's goal is to respond to reports of water leaks and make repairs within 4 days. During FY2014, 56 percent of reported leaks were fixed in 1 day, 22 percent within 2 days, and 12 percent within 3 days. The remaining 10 percent were completed within 4 or more days.

8.2 Rehabilitation and Replacement of Distribution Mains

Rehabilitation and replacement of water distribution mains in the City has benefits for customer service, reduction of nonrevenue water, and supporting redevelopment of the City and property owners. Over the next 5 years, the new DWSD-Retail organization can develop a strategy for its new mission focused on the City of Detroit, and a key part of that new mission is rehabilitation and replacement of aging water mains in the city. A framework for such a strategy is presented below.

8.2.1 Recent Rates of Water Distribution Main Replacement

DWSD has generally allocated \$10 million annually for water distribution main rehabilitation in its annual capital improvement programs. Generally, water main rehabilitation is performed by replacing old water mains with new ductile iron pipe. During the fiscal years 2002 to 2008, approximately 140 miles of water transmission and distribution mains were re-constructed in the City. From July 2008 to June 2014, less than 45 miles of distribution main were reconstructed. In Fiscal Year 2015, DWSD's distribution system rehabilitation program was restructured. Several new projects were initiated: WS-648A, WS-685, WS-686, WS-691 and WS-693 totaling approximately \$35 million and including approximately 20 miles of water main replacement. In the restructuring of the water distribution main replacement program, DWSD discontinued its traditional practice of furnishing water main and appurtenances for water main contracts. Contractors are now responsible for furnishing water mains and appurtenances in conformance with DWSD specifications.

Over the last 20 years, funding for distribution main replacement in Detroit has allowed for the replacement of less than 10 miles per year, or approximately 0.3 percent of the system annually. In the last 5 years, the replacement rate has been significantly less than this. DWSD's records indicate a total of 2,700 miles of water distribution main in the City of Detroit. Therefore, the recent rate of replacement is low relative to the expected service life of distribution pipeline.

8.2.2 Water Distribution Main Renewal Alternatives

In Phase 2 of this planning study, the project team conducted technical investigations of alternative technologies for pipe rehabilitation and replacement. A series of meetings were held with DWSD Field Services Group representatives to discuss current practices in Detroit and practices in other cities in the United States and Canada.

The technologies reviewed included:

- Ductile Iron Pipe
- Cleaning and Cement Mortar Epoxy Lining
- PVC Pipe
- HDPE Pipe
- Pipe Bursting
- Slip-Lining
- Tight-Fit Lining
- Cured-in Place Lining
- Polymeric Lining
- Horizontal Directional Drilling

DWSD is currently spending \$250 to \$300 per foot for construction of new ductile iron water mains. Alternatives technologies were investigated for applicability to reduce water main renewal cost to \$150 to \$200 per foot.

A three year pilot program is recommended for DWSD to determine the optimal application of the following technologies:

- Ductile Iron Pipe
- PVC Pipe
- HDPE Pipe
- Pipe Bursting
- Slip-Lining

The pilot program would include the development of specifications, standard installation details, construction inspection protocols, and evaluation criteria testing completed construction. The outcome of the pilot program would be guidelines regarding which technologies are most cost-effective in which locations. The pilot program is described in more detail later in this chapter.

A review of potential new pipe materials and construction techniques that were considered and discussed with DWSD is presented below.

8.2.2.1 Ductile Iron Pipe

Ductile iron is the strongest and most durable of pipe materials considered in this comparison. Ductile iron has a higher yield stress; the minimum yield stress for ductile iron is 42,000 psi, this is approximately 6 times the tensile strength of PVC, and 24 times the tensile strength of HDPE material. Since the wall thickness required for DI pipe to resist internal pressure is substantially smaller than either PVC or HDPE pipe, DI pipe has a larger a flow cross-sectional area than either the PVC or HDPE pipe product. For instance, the ID of an 8" diameter class 350 ductile iron pipe (8.43 inches) is comparable to the ID of a 10" dia SDR 11 HDPE pipe (8.8 inches). The Hazen-Williams flow coefficient for DI pipe is close to that of PVC and HDPE pipe, with a typical "C" value of 140 for design of new pipe.

The strength of DI pipe is not affected by temperature variations during installation and operation. Both PVC and HDPE are thermoplastic materials, pipe produced from these materials are rated for a design temperature of 73.4 °F, and are derated for water system operating temperatures greater than 80°F. While all pipe products are subject to thermal effects, expansion and contraction with change in temperature, these effects are moderate for DI pipe. PVC has a thermal expansion coefficient 4.5 times that of ductile iron; HDPE has a thermal expansion coefficient 18 times greater than ductile iron.

DI pipe will resist 4 times the hydrostatic burst pressure of PVC pipe, and up to 6.1 times the hydrostatic burst pressure of HDPE pipe. DI pipe is not affected by long-term stress from internal pressure. There is no measurable relationship between applied tensile stress (applied hoop stress) and time to failure.

DI pipe is more resistant to possible damage during handling, shipping and installation. DI pipe has more than 13 times the impact strength of PVC pipe, and up to 12 times more impact strength than HDPE pipe. For PVC and HDPE pipe, nicks or gouges that have a depth greater than 10 percent of the pipe wall thickness significantly reduce the performance life of the product. DI pipe has higher pipe stiffness than PVC or HDPE pipe. DI pipe will resist up to 8 times the crushing load of PVC pipe, and up to 82 times greater than HDPE pipe.

For service connections that are 1-1/2 inches and smaller, DI pipe may be directly tapped with a corporation stop unlike PVC that requires a tapping saddle, or HDPE pipe which requires a fusion coupling.

Ductile iron is typically 2 to 3 times the material and installation cost compared with PVC and HDPE. DI pipe is metallic, and is subject to potential corrosion due to stray currents and dissimilar metals. DI pipe must be protected when installed in potentially corrosive environments that include:

- Soils that are poorly drained or have high moisture content;
- Soils with low measured resistivity;
- Soils with low pH;
- Soils with low or negative oxidation-reduction potential; and

With available laying lengths of 18- to 20-feet, a water distribution system using DI pipe will have numerous pipe joints, and a greater potential for leakage if the pipe joints are not installed correctly. Joint leakage is minimized by adhering to the manufacturer's recommended installation instructions, performing industry standard construction oversight procedures, and implementation of pressure testing procedures on all completed piping systems.

8.2.2.2 Cleaning and Cement Mortar/Epoxy Lining

Conventional cement mortar lining provides a cement liner to the internal wall of the pipe after the pipe has been cleaned by removing tuberculation and some of the graphitization that has occurred on the inside of the pipe. A thorough cleaning of the pipe is required before the lining can be applied using an electric or air-powered rotating head, or a drag-line cleaning head. However, it should be noted that the completed cement mortar linings do not require adherence to the pipe wall. Cleaning and cement mortar lining does not provide a quantifiable structural improvement. Cleaning and cement mortar lining is typically utilized when the existing pipe is in good structural condition and does not require structural rehabilitation. Cleaning and cement mortar lining will typically improve hydraulic capacity in the existing pipe and arrest internal pipe corrosion, extending the life of the existing pipe.

Epoxy lining is another non-structural material that is applied to the internal surface of the pipe. The epoxy is sprayed onto the surface after a thorough cleaning. Epoxy lining is typically thinner than cement, maintaining more of the pipe's nominal inside diameter.

Cleaning and cement mortar/epoxy lining are AWWA M28 Class I non-structural liners.

In general, cleaning and cement mortar lining or epoxy lining are not considered applicable to most of DWSD's water distribution mains due to the age of the mains. This technology may be applicable to certain larger diameter transmission mains within the city.

8.2.2.3 PVC Pipe

For the 6-inch through 12-inch pipe sizes the applicable industry standard for PVC pipe is AWWA C900. PVC pipe manufactured under this standard is a blend of PVC resin and filler materials that meet or exceed the cell classification 12454B as defined in ASTM D1784. AWWA C900 designates three diameter to wall thickness ratios (DR's) and pressure classifications or maximum working pressures as shown in **Table 8-1**. A surge pressure allowance based on an instantaneous velocity change of 2 fps is included in the pressure classification.

Table 8-1: Dimension Ratios, Pressure Class, and Surge Allowance for AWWA C900 PVC Pipe

Dimension Ratio (DR)	Pressure Class/ Maximum Working Pressure (psi) at 73.4°F	Surge Pressure Allowance (psi)
25	100	30
18	150	35
14	200	40

PVC pipe has been used successfully in many water distribution systems for several years, without and significant material issues. PVC pipe is thermoplastic material, resistant to corrosion, and is unaffected by aggressive soil conditions. Due to the pipe wall thickness required to resist internal pressure, PVC pipe has a flow cross-section area that is smaller than DI pipe, but greater than HDPE pipe. To offset the reduced flow area, PVC pipe has a higher Hazen-Williams flow coefficient, with typical "C" value of 150 used for design of new pipe.

The material cost for PVC pipe is comparable to that of HDPE pipe, and is lower than DI pipe.

Some water utilities have reported PVC pipe failures including joint leakage to catastrophic failure during tapping operations. Damage to PVC pipe from impact by construction equipment usually propagates in a spiral direction along the length of the pipe, and often requires the entire pipe segment to be removed and replaced.

The maximum working pressures are for water at a service temperature of 73.4°F. For water at service temperatures greater than 80°F, the pressure classes of pipe must be reduced by appropriate pressure rating reduction coefficient listed in AWWA C900. Since water temperatures in the distribution system are seldom above 70°F, this does not appear to be an issue for PVC pipe in this project.

8.2.2.4 HDPE Pipe

The industry standard ASNSI/AWWA C906-99 describes three material code designations for polyethylene (PE) material, PE 2406, PE 3406, and PE 3408. The three PE material codes provide two series of pressure class or working pressure ratings that describe the hydrostatic design basis (HDB) for PE pipe and fittings. The PE 2406 and PE 3406 materials use an HDB of 1250 psi and PE 3408 uses an HDB of 1600 psi. Pipe products within the two series of pressure classes are further classified into

10 standard dimensional ratios with diameters ranging from 3- through 54-inches. Pipe and fittings manufactured from PE 3408 materials cover a broader range of working pressures from 51 psi through 254 psi. HDPE pipe is available with ductile iron pipe size (DIPS), iron pipe size (IPS), or ISO pipe sizing systems.

HDPE pipe is chemically inert, resistant to corrosion, and will remain unaffected when installed in aggressive soils, or soils conditions such as cyclic wetting and drying that promote corrosion of metallic pipeline materials such as ductile iron. Unlike DI or PVC pipe, HDPE pipe is unaffected by freezing temperatures. Water can be frozen solid in HDPE pipe without damaging the pipe.

Since individual HDPE pipe segments, fittings, and service connections can be connected by butt fusion or electrofusion joints, the portions of the water distribution system that are constructed from HDPE pipe and fittings offer an essentially “leak free” system independent of pipeline valves and appurtenances. The potential for leakage remains where HDPE transitions to DI pipe or fittings (HDPE/mechanical joints at valves, and at flanged HDPE/HDPE connections). However, by reducing the overall number of pipe joints in the water system, use of the HDPE Pipe offers significant savings by reducing unaccounted losses from the water distribution system.

HDPE pipe is a tough, lightweight, flexible, and resilient material. Although installation of HDPE pipe requires use of specialized equipment for the butt fusion joints, HDPE pipe can be installed in narrower trench, using the same types of excavation equipment as DI or PVC pipe. Additionally, the fusion-bonded segments of HDPE pipe can be bent to closely follow curved roads, without the need for additional special bends or deflected pipe joints required for DI or PVC pipe.

HDPE pipe segments installed with fusion joints essentially become one fully restrained length of HDPE pipe. HDPE pipe has a flow cross-sectional area smaller than either DI or PVC pipe, due to the wall thickness required to resist internal pressure. To compensate for this, HDPE pipe has a higher Hazen-Williams flow coefficient, with a typical “C” value for design of 155 for new pipe.

HDPE pipe is resistant to ultraviolet radiation and thermal degradation during storage prior to construction, two environmental factors responsible for brittle failure of PVC pipe.

There are several methods available to effect either temporary or permanent repairs of damaged HDPE Pipe. Temporary repairs can be performed while the line is under pressure by covering the damaged section of pipe with a service saddle or a dresser type coupling. The damaged section of HDPE pipe may be permanently repaired by first removing the damaged section of pipe, and then installing a new section of pipe using butt fusion joints, electro-fusion couplings, or a combination of both.

HDPE pipe has a thermal expansion coefficient that is significantly higher than DI or PVC. While the heat fusion joints will produce one fully restrained length of HDPE pipe, temperature changes in the pipe may overcome the friction resistance of the soil and have adverse effects on pipeline valves, fittings, and appurtenances. One solution to this potential problem is the installation of concrete thrust anchors at intervals along the length of the pipeline. Construction of the thrust anchor involves first excavating an area perpendicular to the pipeline, then heat welding an HDPE collar (with an integral webbed ridge) to the pipeline, and finally embedding the HDPE collar in low strength concrete poured against undisturbed soil.

For HDPE pipe, the pressure class/maximum working pressure rating is based on maximum water temperatures of up to 80°F. Above this temperature, the pressure class is derated by the appropriate temperature compensation multiplier. Again, since water temperatures in the distribution system are seldom above 70°F, this does not appear to be an issue for use of HDPE pipe in this project.

HDPE pipe is difficult to repair due to the complexity in performing the fusion welding process in inclement weather (i.e. rain, snow, freezing temperatures). It is very difficult, often impossible, to perform the fusion welding process in a trench filled with water, which is often the case during pipe repairs.

8.2.2.5 Pipe Bursting

Pipe Bursting is a method of pipe replacement of gas, water and sewer mains usually from excavations 20-600 feet apart. Typically, two pits (a receiving pit and launching pit) are constructed. Then, an expanding device which, may be either pneumatic or hydraulic, is introduced into the defective pipeline, shattering the pipe and pulling in the new line behind it. Insertion of short lengths may be made from pits but this involves jointing of the pipeline within the pit. Two methods are commonly used as outlined below.

- **Static Bursting:** The hydraulic burster is positioned in the excavation and the rods individually connected and pushed into the old pipe. Each rod is unscrewed and removed, or in the case of two-way bursting, fed directly into the old pipe, on reaching the hydraulic burster the tooling is disconnected from the new pipe and removed
- **Pneumatic Bursting-** the pipe to be replaced is exposed and cut out at two points, a winch cable is pulled into the old pipe with a flexible rod. Using the flexible rodding, the air supply hoses are pulled through the new pipe to be installed and connected to the compressor and the rear of the pipe bursting head. The new pipe is secured to the rear of the pipe bursting head, which is in turn connected to the winch cable and drawn into the launch pit and the air supply turned on.

8.2.2.6 Pipe Splitting

Pipe splitting is similar in technique to pipe bursting but is used on non-fragmental pipes such as steel, ductile iron or polyethylene. The process of pipe splitting involves pulling a bladed cutting wheel through an existing pipeline, and cutting the pipe along the crown and invert as a new, typically larger pipe, is being drawn through the space where the existing pipeline was. The bladed wheels actually split the host pipe instead of ripping or tearing it like single fin-type static systems. The bladed wheel system is a very "clean" process that requires less power than other static systems and helps prevent potential damage to the product pipe. An attached expander spreads and displaces the split pipe into the surrounding soil while simultaneously pulling in the new pipe. This method avoids sizable surface damage and costly restoration required for open trenching methods. HDPE or fusible PVC can be the new pipe material to be pulled.

Pipe splitting is preferable in situations when a reduction in pipe diameter is undesirable or when it is necessary to increase the pipe diameter. It is also preferable that there are few lateral connections on the pipeline.

8.2.2.7 Pre-chlorinated Pipe Bursting

Pre-chlorination pipe-bursting is a specialized technique for potable water lines in which the HDPE pipe is bacteriologically disinfected and pressure tested above grade prior to installation, allowing it to be placed into service immediately after installation.

The method of pre-chlorinated pipe bursting has a history that extends many decades as the process was first adapted by water companies in the U.K. It has become a preferred method of replacement in Europe, with over 100 million feet replaced to date. The method utilizes HDPE pipe and entails the pre-assembly and testing of approximately 300 to 600 foot lengths of pipe above grade at a nearby staging location. This work is completed in advance of pipe bursting operations. Once the pipe string is proven to be sound by the testing and disinfection procedures, bursting operations can begin. In the area of water main being replaced, a series of small excavations are made and the new pipe is pulled into place by pipe bursting the existing main. A post-chlorination and flushing of the main is then performed and the new line is connected into the distribution system.

Ultimately, all services are connected into the new main and the surface area is backfilled to preconstruction grade. The entire process is completed within, 6 to 8 hours, thereby minimizing the disruption to area residents.

8.2.2.8 Slip Lining

Slip lining is the insertion of a new pipe into an existing host pipe, with the outside diameter of the new pipe being smaller than the inside diameter of the existing host pipe. This method allows the lining to fit inside of the existing pipe without excavating or removing the pipeline. The decrease in diameter of the new pipe will depend on the thickness of the lining and the standard pipe sizes available. Common materials used for slip lining include steel, fiberglass mortar pipe (FMP), high density polyethylene (HDPE) and fusible poly-vinyl chloride (FPVC), a material that is increasing in popularity. Following the insertion of the new liner, the annular space left between the host pipe and the new pipe would typically be filled with a grout. Following grouting, the newly installed liner pipe has full structural strength like the host pipe, but does not rely on the host pipe.

Although slip lining does not require an open-cut trench for the entire length of pipe, access pits are required for inserting and receiving the pipe as well as for changes in alignment or connections to branch mains. Curved sections of pipe are of particular concern and may also require an open cut, unless a flexible pipe material such as HDPE or PVC is used to maneuver through the curves. The minimum bending radius for flexible slip liners varies depending on the pipe material and diameter. For example the minimum bending radius for HDPE is significantly smaller than that of FPVC, which would allow the HDPE to navigate much tighter curves.

In addition to the ability of HDPE and FPVC to maneuver through curved sections of existing pipe, these types of liners can also be disinfected and pressure tested before installation and can be fused together in long sections. The fusion process results in strong joints which are equal in strength to that of the actual pipe. The length of pipe to be fused is based on the conditions of the existing pipe including slope, bends, and other considerations which go into locating access points, as well as the available lay down area nearby.

Slip lining is considered a structural solution because the liner acts as a pipe within a pipe and is capable of serving without the host pipe, if necessary. Slip lining is typically used when the existing pipe is in poor structural condition and a structural repair is necessary. Slip lining will eliminate internal and external pipe corrosion on the slip liner and extend the life of the existing main by arresting internal corrosion by the placement of grout. A factor when considering slip lining is that it typically reduces the internal cross sectional area of the existing pipe, reducing overall hydraulic capacity. If hydraulic capacity cannot be reduced, then slip lining may not be the appropriate method to select. However, sometimes the low friction material of the slip liner can make it hydraulically comparable to a tuberculated pipe of larger nominal diameter.

The design considerations for slip lining projects include selection of a pipe diameter, determination of required wall thickness, analysis of flow capacity, and locating access points. The pipe diameter of the liner is typically 10 percent less than the diameter of the existing host pipe. The liner diameter depends on available sizes and the size and condition of the existing host pipe. The wall thickness of the new pipe depends on the estimated internal pressure that the pipe will be exposed to. The new pipe is also sized so that it is capable of withstanding all external forces including soil, groundwater and static water pressure loads.

As mentioned above, for HDPE and FPVC liners, the lengths that can be pulled through each access point will depend on the available lay down area, the physical variations in the existing pipe, the tensile strength of the new pipe material and the total pipe weight that can be pulled by the Contractor's equipment. The size of access pits are determined primarily by the pipe depth and the features of the location such as traffic and topography. Other factors include the soil conditions and size of pipe.

8.2.2.9 Tight Fit Lining

This technology and approach consists of an HDPE pipe (4" to 60") that is inserted into the host pipe. Tension on the HDPE pipe, in conjunction with rollers will create a temporary reduction in the HDPE pipe diameter to allow it to be inserted into the host pipe.

The host pipe must be thoroughly cleaned and then inspected with CCTV cameras. Before the pipe is installed a "proofing pig" that is the same outside diameter as the diameter of the HDPE liner pipe is pulled through the host pipe. The proofing pig assures the new HDPE liner pipe can be installed. In addition, the surface of the pig can be inspected for damage to determine if internal corrective work is required in the host pipe.

Once the towing head exits the host pipe at the receiving or pulling pit, the pulling cable is disconnected and the pipe is able to return to its original size creating a tight-formed fit pipe within a pipe. It takes 1 to 3 days for the pipe to completely relax before any connections can be made. Depending on the liner thickness, the liner can be semi- or fully structural. The insertion lengths for a single pull can be up to several thousand feet long.

This technology is nearly identical to slip-lining with HDPE as discussed above with a few key differences including:

- It is tight fitting and does not need grouting

- The pipe requires more extensive pipe cleaning / pigging and proofing that the proposed pipe can be pulled through the pipeline.

8.2.2.10 Cured in Place Pressure Pipe Liner (CIPP)

This method consists of an epoxy resin impregnated, glass or carbon fiber reinforced, flexible felt tube that is inserted through an inversion process utilizing water pressure to force the liner to travel through the host pipe and stay against the host pipe wall. The pressurized water is then heated to cure the thermosetting epoxy resin against the host pipe. It is considered a tight fitting liner with no grouting required between the liner and the host pipe. The CIPP has an internal polyethylene layer in contact with the water that is NSF 61 approved.

CIPP is considered a semi-structural liner for the diameter and pressure in this application, acting as a pipe within a pipe, but relying on the host pipe for a measure of overall structural support. CIPP is installed as a continuous liner without joints; therefore it is able to repair holes, gaps, and leaks in the existing pipe. CIPP is also capable of navigating through modest vertical and horizontal bends in the existing pipe, likely reducing the number of necessary access pits. CIPP will improve existing pipe hydraulics, is a non-corrosive material and will extend the life of the existing main by arresting internal corrosion. A CIPP liner is an AWWA M28 Class III semi-structural liner.

DWSD has successfully used CIPP as a technology for lining sewers, however this technology is not considered applicable to most water distribution mains. This technology may be applicable to certain larger diameter transmission mains within the city.

8.2.2.11 Polymeric Liner

This lining is a rapid-setting polymeric material that is centrifugally sprayed onto the host pipe's interior after a thorough cleaning of the pipe has been performed. Certain products include a polyurea blend that restores pipe width while increasing water flow rates and minimizing water loss throughout the system. The thermoset coating is essentially an inert plastic that is resistant to corrosive environments. With its rapid curing time, pipes are ready to be put back into service on the same day of the application. The pipe must be dewatered to apply a polymeric liner. A polymeric liner is considered semi-structural, able to span small holes in the host pipe. It is not a class IV structural rating but rather relies on the host pipe strength and provides a barrier to prevent or eliminate future internal corrosion of the metallic pipe wall.

This spray-on lining system has been developed for installation for pipe sizes up to 12-inch diameter, however this technology is not considered applicable to most water distribution mains due to the age and structural condition of many distribution mains.

8.2.2.12 Horizontal Directional Drilling

Horizontal directional drilling is commonly used to minimize construction impacts relative to open cut construction within urban settings, or within environmentally sensitive areas. It is an applicable trenchless construction technique in appropriate situations for installation of Fusible PVC, HDPE, and in some situations ductile iron pipe.

This technique is often used to replace failing water mains while leaving the existing pipe in service while the construction of the parallel water main is installed. After the construction is complete, any valves, taps and services are reinstated on the new pipeline.

First a pilot hole is drilled to a predetermined depth and exit point. A reamer is then attached to the pilot drill and the water main is attached to the reamer. The water main is then pulled back through the reamed-out pilot hole.

The head of the drill contains a transmitter which enables the crew to track its location and make adjustments in the direction needed. Some variation in horizontal and vertical alignment can be expected, but the possible deviation is within the tolerance of the drill.

Except for the construction of the entry and exit pits on each side of the river, which will occupy relatively small area, minimal environmental impact to the surrounding area is anticipated. With the HDD and protection of the work areas, sensitive environmental receptors in and around the river will not be disturbed.

8.2.3 Asset Management Prioritization of Water Distribution Renewal Projects

Many older water distribution systems face the same challenges as Detroit relative to the expense of replacing or rehabilitating water distribution mains within a service life of 100 years. That is why rehabilitation and replacement is increasingly implemented on a risk-based asset management approach to identify those mains most at risk of leaking, breaking and internal or external corrosion. The approach to evaluating risk in a rehabilitation and replacement program considers the severity of the impact of a water main failure and the probability that the failure will occur. The level of service goal, in conjunction with field data and leak and break history is used to quantify severity of risk.

Asset condition ratings, such as Excellent, Good, Fair, Poor, or more precise numerical ratings, are assigned to each city block of water main. Lower asset condition ratings imply higher risk hazards, which are defined as potential negative events that will reduce performance and level of service. Assets with low condition ratings have greater probability that a failure will occur. As a result, for two assets in equally poor condition, the probability of “failure” is the same but the risk to the utility is greater for the more critical asset. The level of risk reflects both the severity of impact and the probability of occurrence. By using the risk assessment approach that is linked to the level of service targets, future risk levels can be predicted and a plan for prioritizing rehabilitation and replacement for each City block can be assigned.

DWSD uses the following criteria that are similar to a risk based approach:

- Age
- Material
- Number of Breaks
- Population Density

- Coordination with other roadway improvement or redevelopment projects by others

The data that DWSD has collected on break repair and pipeline condition within its GIS system and record books should be migrated to an asset management software application for ongoing use to assess the highest risk water mains and rehabilitation or replacement needs.

The criteria for population density reflects the situation that certain areas of the city have lost population and some streets have no active water users, or only a limited number of residential or commercial customers. DWSD has typically not replaced or rehabilitated water mains on vacant or low residency streets. However, these old mains and their service connections could be a significant source of water loss. The following section on 'System Consolidation' addresses distribution mains in these locations with a proposed program of water main retirement.

8.3 System Consolidation

While rehabilitation and replacement is needed in at least 70 percent of the City, 30 percent of the City includes areas where the population is less than 2 persons per acre, or is projected to reach that population density in the next ten years. In these areas, consolidation of the distribution system is proposed to reduce the number of miles of active water mains.

Consolidation is achieved by retiring water mains. In the simplest situation, where all properties on both sides of the street are vacant, a main can be retired by shutting the gate valves at each end, and then cutting and capping both ends. The retired main can be left in place, if it is structurally sound. If corrosion is suspected, then flowable fill should be injected for structural support.

A preliminary estimate of the cost of retiring water mains was prepared. The estimated cost is \$40,000 per mile, and it is based on the use of DWSD field crews and equipment. It is estimated that three crews would be required to complete the retirement of 1 mile of main within one week. Locations for main retirement would be planned in advance.

Before retiring a main, an evaluation needs to be done to assure that the main is not providing a transmission or redundancy function for another main. Also the ability to provide fire protection needs to be assessed by considering distance to surrounding fire hydrants and fire hose length analysis. A general outcome of this would be for the Fire Department to be equipped with longer 1,000 foot 4-inch diameter fire hoses, instead of the standard 100- to 300-foot hoses. Additional training for Fire Department personnel would be required.

DTE established a natural gas main consolidation program in Southeast Michigan in 2011. A total of 160 miles of gas main has been retired, and 76 miles is planned in 2014. Within the City of Detroit, 15 miles were retired in 2013, and another 15 miles is planned for retirement in 2015. DTE has a methodology to select mains for retirement based on numbers of properties abandoned. In some cases, where one or two properties remain customers in mid-block, the gas main is retired, but service is replaced with a new smaller diameter and shorter pipe in an easement through one or more vacant lots from a nearby active main.

Figure 8-1 shows the location of DTE gas main retirements since 2012.

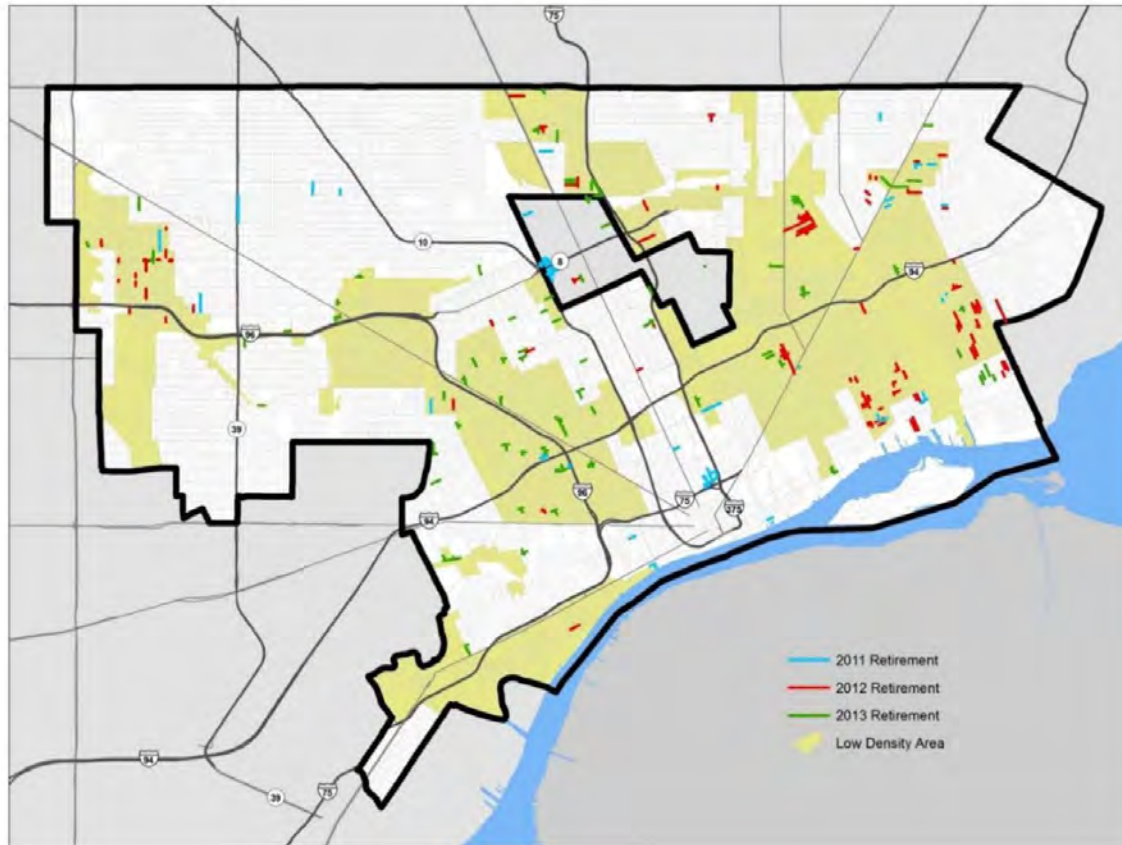


Figure 8-1: DTE Gas Main Retirement Program 2011 to 2013

The experience of DTE provides a viable model for DWSD to use in identifying potential locations for retiring water mains.

It is proposed that DWSD retire at least 3 to 5 miles of water main in the next two years, based on the locations where DTE has retired mains. After basic procedures are established, then it is proposed that DWSD prepare an annual plan for water main retirement in coordination with other City departments and DTE. Annual planning should have a goal consistent with the City's framework for consolidation of services. It is anticipated that this goal would be in the range of 10 to 15 miles per year for the next 5 years, and then goals should be reassessed in 2020.

8.4 Lead Pipe Services

DWSD estimates that 95 percent of properties in the city have lead pipe service connections. When DWSD replaces water mains, the portion of the service connection in the street right of way is changed to copper pipe. However, the portion of the service connection on the property is only changed if the property owner makes the change.

The City of Detroit should evaluate options for replacing lead service pipes. AWWA provides guidance documents on options for lead service pipe replacement. As long as DWSD (or GLWA) are in compliance with the Lead and Copper Rule, and long as Detroit's lead service lines have stabilized with phosphate or other chemical coating, there is no driver to replace the lead services lines in the near future. However, some utilities want to eliminate phosphate in their drinking water and

wastewater. These utilities are considering full lead service line replacement (both in the right of way and on private properties). Madison, Wisconsin is taking this approach. Partial lead service line replacement (only in the right of way) is not recommended due to leaching of lead in contact with a dissimilar metal interface and potential disturbance to the lead service lines. Some utilities, such as Chicago, are replacing lead service lines all the way to the customer meter during water main replacement projects. This approach is relatively low cost, because it replaces lead services that have already been exposed during water main replacement.

The proposed 20-year CIP does not explicitly include funding for lead service elimination in Detroit. As stated earlier, chemical addition is being performed at the water treatment plants in compliance with the Lead and Copper Rule to protect public health from lead pipe and fittings. However, DWSD-Retail should develop a long term strategy to eliminate lead service pipes outside of the public right-of-way.

8.5 Proposed Capital Improvement Plan

Accompanying this master plan report in TM-17 is a 20-year Capital Improvement Plan (CIP) spreadsheet that lists approximately 330 CIP projects. These projects are organized by specific programs. There are three programs pertaining to the Detroit water distribution system:

- Program for Piloting New Water Main Renewal Technologies
- Program for Water Main Renewal
- Program for Water Main Retirement

Each program is described below.

8.5.1 Program for Piloting New Water Main Renewal Technologies

This program should start with a review of DWSD's mapping and GIS data for water distribution mains within the city limits. The review should produce an updated GIS system of water distribution mains that will be under the jurisdiction of the City of Detroit as of July 2015. The updated GIS data should be structured to produce an inventory of water main locations, lengths, diameter, and material. Field inspections and internal reviews should determine if certain older mains, such as those constructed on Historic Fort Wayne, are owned by the City or have been transferred to the property owner, and therefore no longer in the City's inventory of water mains. The objective of the inventory and GIS is to refine the reported estimate of 2,700 miles of water distribution main in the city.

After completion of the inventory, the pilot program for new technologies for water main renewal should be conducted. A three year pilot program is recommended for DWSD to determine the optimal application of the following technologies:

- Traditional ductile iron pipe
- PVC pipe
- HDPE pipe

- Pipe Bursting
- Slip-lining

The pilot program would include the development of specifications, standard installation details, construction inspection protocols, and evaluation criteria testing completed construction. The outcome of the pilot program would be guidelines regarding which technologies are most cost-effective and constructible in which type of street, public right-of-way, and development density situation.

This program should start by 2017 and finish in 2020. When results of this program are complete, then scheduling of new water main renewal projects can proceed using the most cost-effective technologies for each location.

8.5.2 Program for Water Main Renewal

In the spring of 2015, DWSD was underway with 7 new contracts for water distribution and smaller transmission replacement. These contracts total approximately 45 miles of new main.

The preliminary CIP spreadsheet includes water main renewal projects currently in the FY2016 approved CIP plus approximately 80 future renewal projects. These future renewal projects extend from 2020 to 2035, and each project include 5 miles of water main renewal. Past practice by DWSD has found that 5-mile lengths for water main renewal provide competitive prices and can be completed within one or two construction seasons.

8.5.3 Program for Water Main Retirement

The preliminary CIP spreadsheet includes a series of water main retirement projects that are scheduled from 2016 to 2035. The first years of the retirement program can follow the approximately 45 miles of streets where DTE has already retired gas mains. In order to proceed beyond the streets where DTE has completed its work, the City should convene an annual meeting of all service departments to coordinate planning and level of service for streets that could be candidates for water main retirement. Input from the Fire Department is especially critical. An update to the Fire Department's master plan should be considered. Water main retirements will impact the distance to hydrants. Options for fire protection include longer hose lengths and tanker trucks. Costs for new fire protection strategies need to weigh against the cost of replacing water mains on vacant streets, or the cost of water loss through a leaking water main that is left in service.

The City's Green Infrastructure Program would also be closely coordinated with the water main retirement program. The Green Infrastructure Program will be identifying new open space areas which could benefit from water main retirements in the green infrastructure project area.

9 Metering and Non-Revenue Water

9.1 Trends in Nonrevenue Water

Nonrevenue water is the difference between the volume of water produced and the volume of authorized water used. Water utilities generally have water production that exceeds water sales. This is due to meter inaccuracies, losses of water through leaks and breaks, and authorized unmetered public use of water for firefighting, water main construction and other purposes.

DWSD's rate setting process includes detailed reporting to wholesale and retail customers on revenues, prior period expenditures and proposed budgets for capital improvements, financing, and operation and maintenance. The information presented in the rates process documents the estimated volume of nonrevenue. In the last ten years, nonrevenue water has been increasing in volume and as a percentage of total water production, as shown in **Table 9-1**.

Table 9-1: Trends in Nonrevenue Water

Period	Annual Volume of Nonrevenue Water (MGD)	Percentage of Total Water Production
July 2004 to June 2005	108.0	18.6
July 2005 to June 2006	106.2	17.7
July 2006 to June 2007	101.9	17.7
July 2007 to June 2008	139.2	23.2
July 2008 to June 2009	125.0	22.7
July 2009 to June 2010	113.6	22.0
July 2010 to June 2011	127.1	23.4
July 2011 to June 2012	145.5	26.2
July 2012 to June 2013	152.3	27.7
July 2013 to June 2014	164.1	30.0

9.2 Preliminary Water Balance for FY 2012

The American Water Works Association (AWWA) has established terminology and procedures for analyzing and managing nonrevenue water. DWSD performed its first water audits in accordance with the AWWA procedures in 2006 and completed a second audit in 2014. The purpose of these water audits is to analyze nonrevenue water and develop estimates for leakage, meter inaccuracy, and authorized, but un-metered public uses and develop strategies to reduce the nonrevenue water volume.

The first step in management of nonrevenue water is to establish best estimates of apparent causes of water loss. **Table 9-2** shows preliminary estimates prepared for this Phase 1 Interim Report based on results from DWSD's water audit program and its rate setting methodology.

Table 9-2: Preliminary Water Balance for FY 2012

Category	Estimated Volume (Million Gallons)	Approximate Percent of Total Production	Basis
Total Production	203,600		FY2012 recorded water production
Over-Registration at WTPs	10,000 to 20,000	5 to 10%	DWSD water audit 2006 pump tests and rate setting methodology
Net Production	183,600 to 193,600		
Water Sold to Suburbs	121,200		Actual sales to wholesale customers
Allowance for Transmission and Wholesale Meter Loss	10,000 to 20,000	5 to 10%	DWSD water audits and rate setting methodology
Water Sold to Dearborn	4,500		Actual sale to Dearborn
Allowance for NRW in Dearborn	800		DWSD water audit and rate setting methodology
Water Sold in Detroit	29,200		Meter readings and estimated billings
Allowance for NRW in Detroit	20,000 to 30,000	10 to 15%	Rate setting methodology
Total Production	203,600		
Total Sales	150,400		
Nonrevenue Water (Total Production – Total Sales)	53,200	26.2%	Annual financial report

Source: CS 1396 2006 Draft Report; Sept 2013 BOWC Finance Committee Water Supply System Revenue Analysis; and Water Rate Methodology Table 14 *Allocation of Non-Revenue Water*.

9.3 Goals for Nonrevenue Water

Many water utilities operate with nonrevenue water of less than 10 percent. For example, Orlando, Florida, and Singapore, maintain nonrevenue water at 5 percent or less. Note that these utilities have limited supplies of potable water. In the United States, northern water utilities with abundant supply may economically operate with 10 to 15 percent, or higher, nonrevenue water.

A goal of 15 percent nonrevenue water in the year 2035 is proposed by this Master Plan Update. The goal of 15 percent was selected because it is representative of nonrevenue water for larger water utilities serving older northern cities in the United States. Setting a target is important for several reasons:

- Nonrevenue water has been growing over the last 10 years.
- Nonrevenue water is an increasing component of retail and wholesale water rate setting.
- More accurate information on actual water production and pumping rates is important to new initiatives proposed for electrical demand management.

- Management of nonrevenue water leverages DWSD's previous investments in wholesale customer and retail customer metering.

9.4 Current Practice for Metering and Water Audits

9.4.1 Production Metering

Production is currently estimated at four plants based on pump curves and discharge pressures measured at regular time intervals. Depending on the number of pumps running and throttling conditions, pressure is measured at either the discharge header or at the individual pump casing. For variable speed pumps running at reduced speed, the plant operators use additional information on pump speed to calculate flow rates. At Lake Huron, water production is reported by totalizing individual filter effluent flow meter readings. All pressures and estimated production rates are manually recorded.

All of DWSD's water treatment plants were initially constructed with differential pressure meters to measure water production. With the exception of Lake Huron, Venturi meters were installed at all plants. The Lake Huron meter was an Annubar type, which has multiple ports for measuring differential pressures across the full cross section of flow.

The meters themselves are highly accurate, require little maintenance, and have a long service life. However, the original instrumentation and recording systems were older hydraulic analog technology, difficult to maintain, and produced inconsistent results. Consequently, DWSD has been using pump curve data for at least 20 years.

Flow measurement using pump curve data is generally used to estimate accuracy of actual flow meters. Over the long term, the accuracy of pump curves degrades as wetted parts of pumps wear and corrode. Also, because pressure and flow measurements are recorded hourly, actual flow variations may be missed, and the manual recording process is subject to human error. Human error is minimized by having flow and pressure recorded in the Ovation system, so that the information can be retrieved and checked.

DWSD recently completed an assessment of the Venturi meters to determine the feasibility and practicality of restoring the Venturi meters versus replacing them with new meters. Findings are summarized in **Table 9-3**. DWSD is proceeding in 2015 with a new project to design the rehabilitation of the Venturi meters at Southwest, Northeast, and Water Works Park.

Table 9-3: Characteristics of Existing WTP Meters

Plant	Existing Meters	Installation Date	Condition and Configuration
Lake Huron	1-Annubar meter	1970 circa	<ul style="list-style-type: none"> Abandoned
Water Works Park	14-Venturi meters	Unknown; believed to be prior to 1915	<ul style="list-style-type: none"> Limited access, full inspection not yet performed due to inability to operate yard piping. Vaults in satisfactory condition, but small. External condition of two meters is satisfactory; others expected to be satisfactory.
Springwells	6-Venturi meters 1-Venturi meter	1931 1958	<ul style="list-style-type: none"> Vaults in excellent condition and adequate size. Access into four vaults is adequate; access into three vaults should be improved External condition of four meters is satisfactory to excellent. External condition of three meters is poor or unknown. Internal condition of Meter 5 is satisfactory, others are expected to be satisfactory.
Southwest	5-Venturi	1961	<ul style="list-style-type: none"> Vaults in excellent condition and adequate size. Access into vaults for employees and for removal of Venturi throats is satisfactory. External condition of all meters is very good. Internal condition of one meter is excellent; others expected to be the same.
Northeast	6-Venturi	1956	<ul style="list-style-type: none"> Vaults in satisfactory condition and adequate size. Access into vaults for employees and for removal of Venturi throats needs improvement. External conditions of Venturi meters are satisfactory. Internal condition of one meter is satisfactory; others expected to be same.

9.4.2 Water Audits

DWSD performed its first comprehensive water audit in 2006. In addition to the audit, the work included a leak survey for portions of the distribution system, assessing the water plant metering, an assessment of water loss in both the City of Detroit and the suburban system, and recommendations to improve metering, improve the recording of non-metered beneficial uses, and reduce real losses.

Pump testing at three of the water plants (Water Works Park, Southwest, and Northeast) was conducted as part of the water loss work. The purpose of the work was to improve the recording of plant flows into the system until the permanent solution of upgrading the existing plant meters was completed.

DWSD completed another water audit for the system based on year 2012 data. This second audit provides for a categorical evaluation between the two audits to assist the Department in assessing

how the volume of water loss has changed and what recommendations can be implemented to reduce real and apparent losses.

In addition to the updated water audit, DWSD is continuing with the leak survey and investigating new technologies that would assist in reducing real losses. The new technologies are in the process of being piloted to evaluate their effectiveness within the Detroit system. Two of these include helium injection and sounding technology that is fixed to the distribution system.

Finally, an investigation is being performed to assess water loss associated with theft. To evaluate this, billing accounts will be overlaid in GIS with parcels to identify parcels without water accounts. A sample set will be identified and investigated to determine if any of the homes on these parcels are still using water. Based on the results, recommendations will be made to investigate the other sites.

9.5 Techniques for Managing Nonrevenue Water

The AWWA Manual of Practice 36 Water Audits and Loss Control Programs outlines a wide range of operational and management approaches to controlling water loss. The following four techniques have been selected based on their success in other large water utilities.

9.5.1 Plant Metering

DWSD is moving forward with a project to re-institute metering at its water treatment plants. Previous investigations by DWSD has shown that rehabilitation of Venturi meters and replacement of the associated instrumentation systems is cost-effective and can be completed in the shortest time and least cost.

9.5.2 District Metered Area Approach to Leak Detection and Management

DWSD currently performs leak detection within the City of Detroit using a combination of its own forces and contract crews. This program generally measures leakage on each street every two years. Measurements are made with acoustic devices, and new technologies, such as helium and sounding pods have recently been used. DWSD also practices active leak management and pressure management to reduce leakage. When leaks are detected, work orders are issued for leak repair.

The District Meter Area (DMA) approach proposed below would supplement DWSD's existing leak detection efforts by providing geographic focus with specific areas of up to 3,000 customer accounts. The DMA approach is described below.

Some large water utilities use DMAs to allow for sub-dividing the system for monitoring leakage and other types of actual water loss. In the United Kingdom, DMAs are generally used. DMAs are typically established for groups of 2,000 to 3,000 customer accounts. Where beneficial for managing pressure and reducing real water loss due to high pressure, the DMA district can be used for pressure management.

Depending on location and other factors, leakage testing would be performed on a biennial or triennial basis for approximately a 1-week duration. During test events, most valves to the DMA would be closed. Test events would be scheduled during the night hours when water use is at a minimum. Valves are closed except for two or more locations where flow into the area, or out of the area, is measured. Flow is measured with temporary pitot tube or insertion magnetic flow meter. A high level

of accuracy is not needed, because the flow measurements are intended to find large water leaks. Flow into the area is compared to calculated water use for the area. If the flow into the DMA is larger than what is calculated for the area, then street by street investigation with leak detection monitoring is scheduled. DMAs are typically measured every two or three years to monitor long term progress in leak reduction.

There is a new body of knowledge and technologies being developed around the world to utilize DMA information. Several companies are currently using computer algorithms and advanced statistical analysis to use DMA data to “pinpoint” leaks and breaks within DMA areas in a more accurate way. DMAs have some other advantages: they facilitate model calibration and improve water security by facilitating isolation of areas during contamination events.

The City of Detroit has a land area of approximately 140 square miles, and most of the area is within one large pressure district. The City of Dearborn has 42 unmetered connections with Detroit, and Dearborn spans 25 square miles. Highland Park adds another 3 square miles. So, in total there are approximately 168 square miles and approximately 300,000 residential, commercial and industrial water accounts without the master metering or the ability to measure leakage systematically in the area.

The DMA concept proposed for Detroit would be to generally have valves open during non-test events. Larger DMA areas can be established initially, and these sub-divided in later years as the leak detection program matures. The DMA is established by assuring that valves are operable around the group of customers.

Figures 9-1 and 9-2 show concepts for DMAs in Detroit. Figure 9-1 is a proposed DMA in northwest Detroit for the area generally bounded by McNichols, Greenfield, Grand River, and Southfield Expressway. Figure 9-2 is a proposed DMA on the east side of Detroit for the area generally bounded by the Detroit River, Grosse Pointe Park, Mack Avenue and the Water Works Park water treatment plant. This DMA is designed to directly overlay a collection system metering district. In this situation, leakage from the water system can be directly correlated with infiltration/inflow in the wastewater collection system.

The colors for valves in Figures 9-1 and 9-2 depict the following:

- Green is an existing valve location
- Yellow is a proposed new valve
- Red is a valve that would be closed during the DMA test.

A preliminary cost estimate was prepared for establishing DMAs at the two sites. The McNichols/Grand River area is estimated to be \$150,000. The East Jefferson area is estimated to be \$250,000. These estimates do not include the replacement of 24-inch manhole rims and covers which provide access to the gate valves.

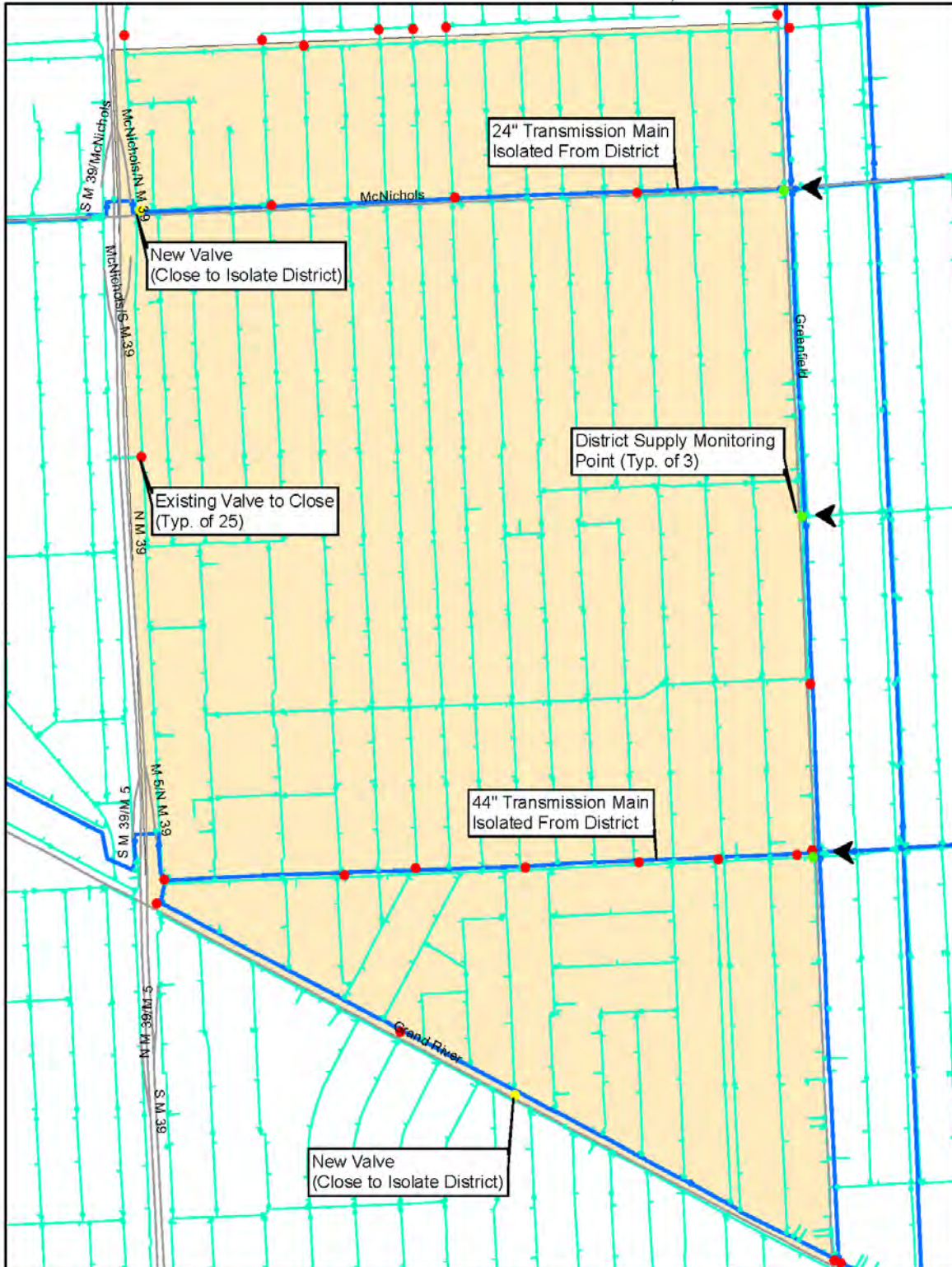


Figure 9-1: McNichols-Greenfield-Grand River-Southfield Survey Area

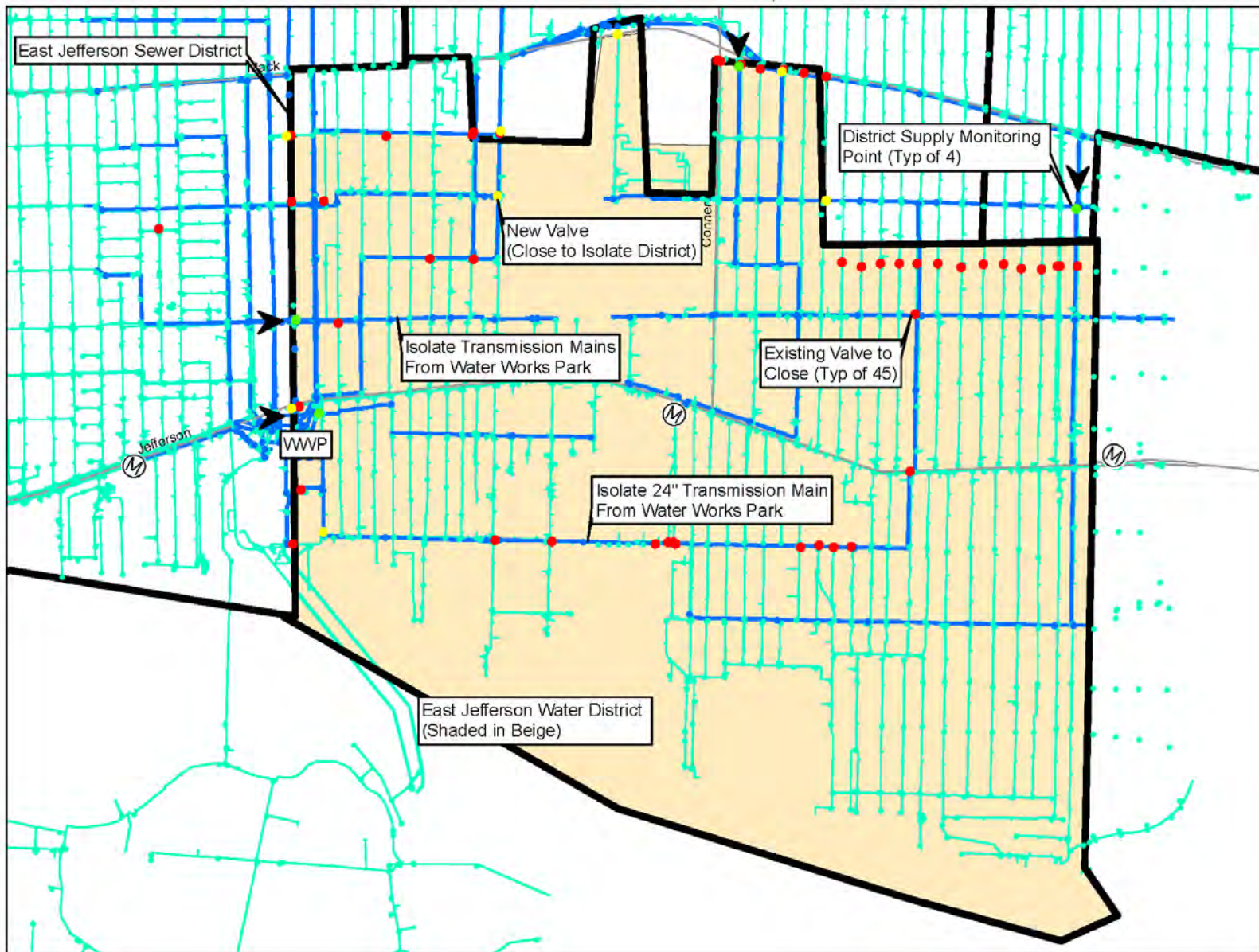


Figure 9-2: East Jefferson Survey Area

DMAs would be established with the following general criteria:

1. Boundaries of areas of the distribution system where inspection, rehabilitation and replacement is scheduled in the first 10 years of the planning period.
2. Boundaries of areas where mains may be shut-off due to low residential population and changing land use. See Chapter 8.
3. Boundaries for potential future permanent pressure districts or master metering districts in the City.
4. Highland Park's distribution system could be configured as one DMA.
5. Boundaries of the master meters on the wastewater collection system that were established in the last 15 years to monitor infiltration and inflow. Overlaying the distribution system leakage monitoring districts with the collection system Infiltration/inflow monitoring districts provides benefits for managing both volumes of water.

A distribution system gate valve upgrade program is the highest cost of establishing DMAs. Gate valves must be in good working condition and be able to close completely. For the two proposed DMAs shown in Figures 9-1 and 9-2, the total number of new or replacement valves is approximately 75, and it is assumed that 80 percent of the existing valves need to be replaced or rehabilitated. Beyond the benefit of creating the DMA, there are other benefits of operable valves, such as for flushing mains, main inspection, ongoing repair and replacement programs, and potentially to manage the district at a reduced pressure.

The DMA program would be conducted for 20 to 40 years, and would be phased out as new wholesale meters between the GLWA regional system and the City of Detroit retail system are put in place. A plan for wholesale meters for Detroit is presented in Section 9.7.

9.5.3 Distribution System Inspection, Rehabilitation and Replacement

The programs for water main inspection, renewal and retirement will reduce water loss as the pipeline infrastructure is replaced. The renewal and retirement programs should be sequenced in alignment with the district meter areas, so that water loss reduction can be documented, and then resources for leak detection can be moved to other district meter areas. .

Previously in Chapter 8, a plan was presented to retire underutilized water main in Detroit over the next 20 years. Mains could be retired in vacant streets in areas of the City where population is less than approximately 2 persons per acre. The retirements would be planned annually in coordination with other gas, electric, and cable utilities, as well as with the Fire Department, and Department of Transportation and other city services. In planning these shut-offs, existing rates of leakage would be considered in prioritizing shut-off locations. A major benefit of reducing the number of active mains is that leakage will also be reduced.

9.6 Wholesale Metering for Dearborn and Highland Park

DWSD provides water service to Dearborn and Highland Park through unmetered delivery points. Dearborn has traditionally been an un-metered customer due to the configuration of its system and its

many connections to the DWSD transmission system. There are currently 42 water delivery connections from DWSD to Dearborn.

The City of Dearborn recently completed a water distribution master plan. That plan evaluated the supply connections to the DWSD system and determined that it would be feasible to reduce the connections to 18 delivery points.

It is proposed that DWSD and Dearborn develop a plan for metering, and complete the installation of wholesale meters during the 20-year planning period. The metering plan should be prepared by 2020.

Figure 9-3 shows a preliminary proposal of two locations where the first two wholesale meters could potentially be installed. These locations represent the two largest supply points for Dearborn. Based on a preliminary estimate, these two locations could provide about 20 percent of Dearborn's daily water demand.

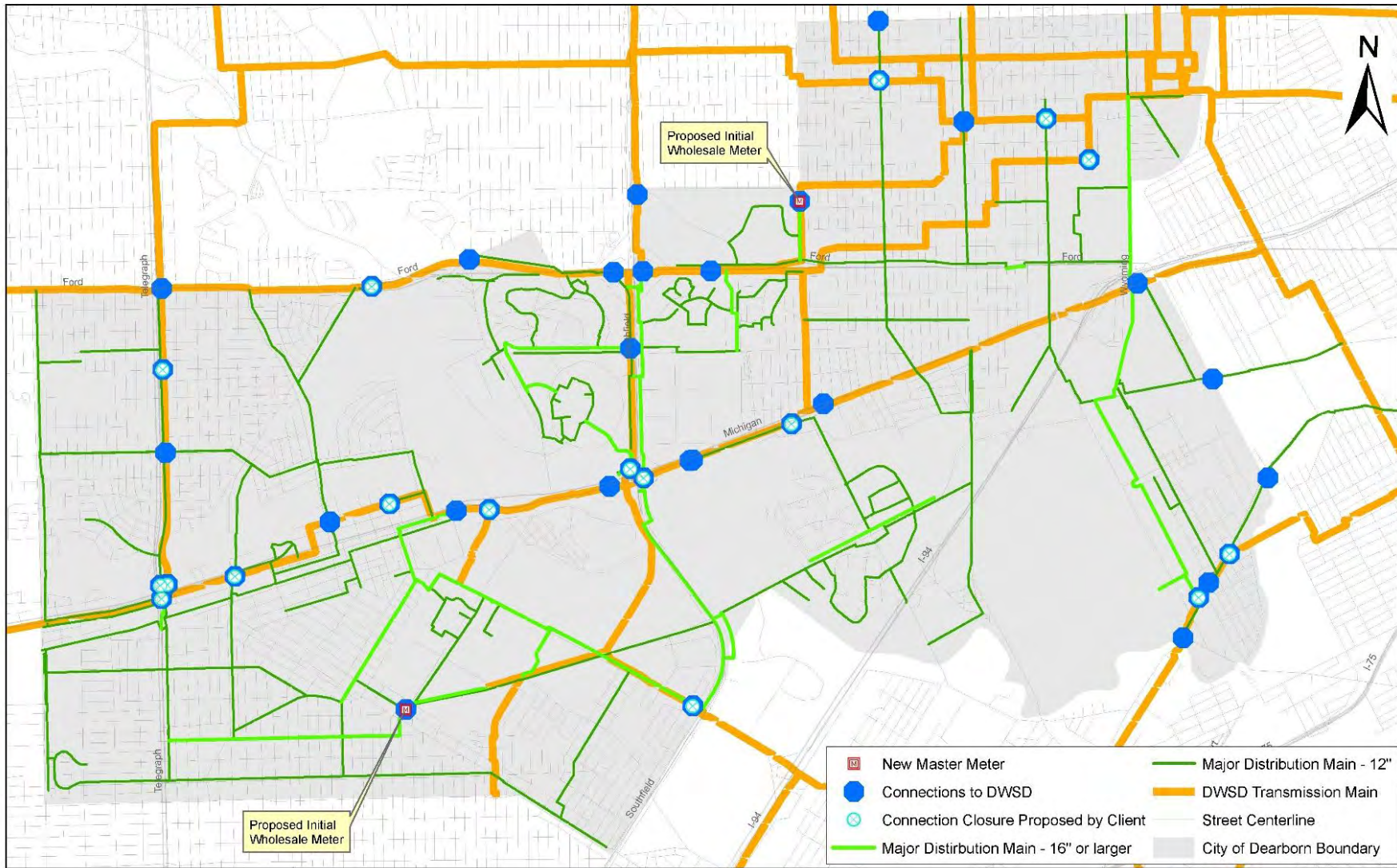


Figure 9-3: DWSD Wholesale Meters Installation for the City of Dearborn

Highland Park provided its own water supply prior to 2012. DWSD began retail service to Highland Park in 2012 under an emergency supply agreement. The City of Highland Park is less than 3 square miles in area. It is proposed that DWSD establish a district metering area, as discussed in Section 9.5.2 to monitor water consumption and nonrevenue water for Highland Park. In the future, permanent meters for Highland Park could be installed that would be “deduct meters” for water sold by GLWA to Detroit.

9.7 Wholesale Metering for Detroit

The installation of new wholesale meters between the GLWA transmission system and the Detroit retail system can be accomplished cost-effectively in conjunction with water distribution renewal projects in Detroit and transmission improvements by GLWA. **Figures 9-4 and 9-5** show two example areas for new wholesale meters on the east side of the City of Detroit. The estimated construction cost for a typical wholesale meter in Detroit is \$250,000, inclusive of a magnetic flow meter, automated cone valve to reduce pressure at night, bypass piping and underground vault. The concept plans shown on Figures 9-4 and 9-5 would require 9 meters and would cost approximately \$2,900,000 inclusive of engineering and contingencies. These 9 meters would measure approximately 15 percent of Detroit’s average daily demand.

The proposed 20-year CIP includes allowances to complete 50 percent of wholesale metering for the City of Detroit by the year 2035. See Section 9.9.3.

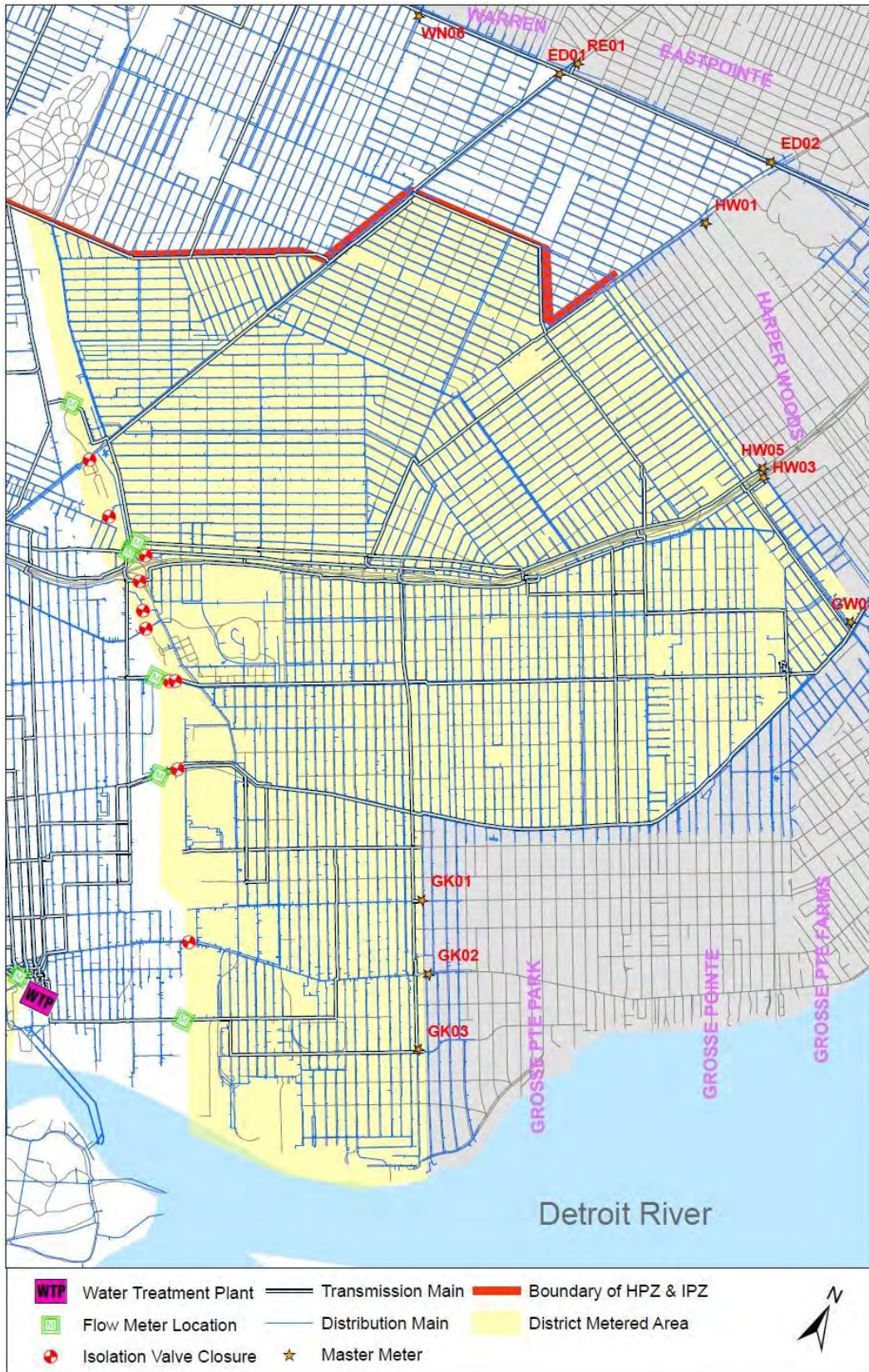


Figure 9-4: District Metered Area Example 1- WWP Intermediate-Low Pressure Zone Map for Flow Meter Installation & Valve Closure Locations

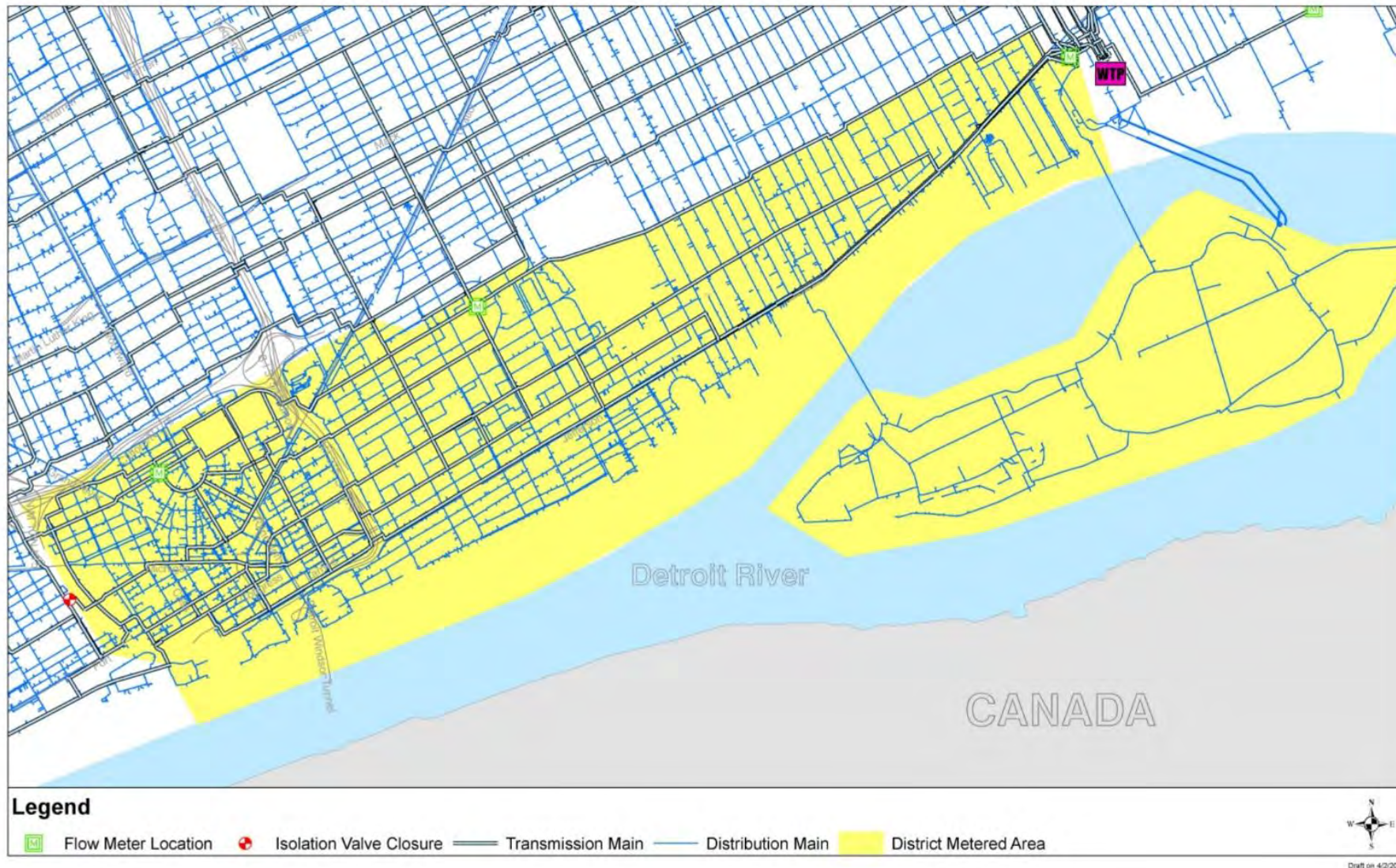


Figure 9-5: District Metered Area Example 2– WWP Intermediate-Low Pressure Zone Map for Flow Meter Installation & Valve Closure Locations

9.8 Retail Customer Metering in Detroit

DWSD began the installation of water meters with radio transmitters in 2007. The new technology, called Automated Meter Reading (AMR), reduces the cost of collecting meter readings and improves the accuracy of the data. The detailed time series data that is collected can be retrieved for analysis and verification.

The current status of retail account metering is shown in **Table 9-4**.

Table 9-4: Status of Retail Metering (January 2014)

Type of Metering	Number of Accounts
AMR	188,543
Non-AMR	44,239
Transitional Properties ⁽¹⁾	24,000
Total Accounts	256,782

⁽¹⁾ "Transitional properties" refer to properties that have water service, but water bills have been returned to DWSD for at least two months. After two months, DWSD performs an investigation and then based on results, may perform a water service shut-off. Properties that are declared abandoned are removed from the list of accounts.

From January 2014, to February 2015, DWSD installed an additional 8,217 AMR meters. The current CIP project for AMR meter installations calls for 10,000 new AMR meters starting in July 2015.

A preliminary estimate of the cost to add new 5/8-inch AMR meters in remaining properties is \$400 per meter. The total cost to complete metering in Detroit is approximately \$20 million.

9.9 Proposed Capital Improvement Plan

Accompanying this master plan report is a 20-year Capital Improvement Plan (CIP) spreadsheet that lists approximately 330 CIP projects. These projects are organized by specific programs. There are four programs pertaining to the Detroit water distribution system:

- Program for Plant Production Metering
- Program for Nonrevenue Water Reduction
- Program for New Wholesale Metering
- Program for Completion of AMR Metering in Detroit

Each program is described below.

9.9.1 Program for Plant Production Metering

This program includes the current CIP project for rehabilitation of Venturi meters at Northeast, Water Works Park and Southwest, plus future projects for production metering at Lake Huron and Springwells.

9.9.2 Program for Nonrevenue Water Reduction

The program for nonrevenue water reduction includes ongoing water audit projects and other operational initiatives. It is proposed that both GLWA and the City of Detroit conduct their respective programs for nonrevenue water reduction.

9.9.3 Program for New Wholesale Metering

The program for new wholesale metering includes the installation of new wholesale meters for Dearborn, Highland Park and Detroit. Also included in this program are scheduled replacements and improvements to existing wholesale customer meter pits and meters. The proposed CIP includes allowances to complete the wholesale metering of the City of Dearborn by 2035, and to complete 50 percent of the City of Detroit in the same time period.

9.9.4 Program for Completion of AMR Metering in Detroit

The program for completion of AMR metering in Detroit includes the current CIP project for FY2016 and subsequent annual projects to complete the remaining 44,000 non-AMR accounts.

GLWA and DWSD-Retail should perform annual water audits to provide current information that will guide the four capital improvement projects discussed above, and to provide key performance indicators relative to overall progress on reducing nonrevenue water. Chapter 11 includes a discussion of future implementation policies, including policy for water audits, which should form the basis for designing the annual water audit process.

10 Financial Analysis

10.1 Introduction

The Water Master Plan Update proposes a 20-year capital improvement program with total capital expenditures in excess of \$3 billion. The recommendations of the Water Master Plan Update are consistent with the current direction of DWSD to optimize operations, therefore there are no major anticipated increases in annual costs for operations and maintenance costs. Anticipated savings in operation and maintenance costs through optimization could partially mitigate future costs of price inflation.

Concurrent with preparation of the Water Master Plan Update, independent forecasts of long-term DWSD financial performance are being prepared by others. The projections in those forecasts are intended to facilitate evaluation of the establishment of independent wholesale and retail utilities, as envisioned by the Great Lakes Water Authority, to support refinancing a portion of the current DWSD debt portfolio, and to support issuance of bonds to finance capital improvements envisioned by the Master Plan Update. This Water Master Plan Update provided key data and assumptions to those projections. Specifically, the independent projections utilized the projected 20-year capital improvement program and referenced the Water Master Plan Update's assumptions regarding future water sales and demands. The results of the independent projections support an overall strategy of gradually employing more revenue (and less debt) financing for capital, and indicate that financing the proposed capital improvement program can be supported within the four percent annual revenue requirement increases envisioned by the Memorandum of Understanding for establishing the Great Lakes Water Authority.

Also concurrent with preparation of the Water Master Plan Update, DWSD has recently made changes to its wholesale water rate structure, increasing the portion of revenue recovered via fixed monthly charges to 60 percent (from 40 percent) and decreasing the portion recovered from commodity charges to 40 percent (from 60 percent). In recent years, wholesale water sales have been below customer projections, therefore rates did not sufficiently recover the total revenue requirements of the system, which are largely fixed in nature. In the last two years, projections were 8 percent higher than actual sales. Approximately 90 percent of DWSD's costs are fixed costs. The FY 2016 wholesale water charges reflect an increase of approximately 11 percent, relative to FY2015, to eliminate these recent revenue shortfalls.

Given the ongoing efforts by others, this chapter includes the following sections:

- Recent Financial Performance of DWSD
- Key Performance Indicators Relative to Peer Utilities
- Recent Organizational Initiatives to Improve Performance
- Review of 20-Year Capital Improvement Program

10.2 Recent Financial Performance

DWSD has experienced significant impacts in governance, finance, customer base, and operations in the last decade which have impacted financial performance.

For the period of time from Fiscal Year 2001 through Fiscal Year 2007, sales remained relatively stable. The recession, which began in 2007, hit the Detroit area particularly hard, and led to the decline in population and decline in water sales. From 2007 to 2015 sales declined nearly 30 percent, as shown in **Table 10-1**.

<u>Fiscal Year</u>	<u>Reported</u>	<u>Water Sales</u>			<u>Non-Revenue Water</u>	
	<u>Production</u>	<u>Wholesale</u>	<u>Retail</u>	<u>Total</u>	<u>Amount</u>	<u>Percent of</u>
	<i>Mcf</i>	<i>Mcf</i>	<i>Mcf</i>	<i>Mcf</i>	<i>Mcf</i>	<u>Production</u>
2001	31,965,700	20,297,200	6,046,900	26,344,100	5,621,600	17.6%
2002	30,223,800	19,289,500	5,799,600	25,089,100	5,134,700	17.0%
2003	32,253,100	19,543,800	5,374,400	24,918,200	7,334,900	22.7%
2004	31,902,900	20,197,000	4,997,000	25,194,000	6,708,900	21.0%
2005	29,643,700	19,195,700	5,405,000	24,600,700	5,043,000	17.0%
2006	28,367,300	19,372,200	4,850,800	24,223,000	4,144,300	14.6%
2007	29,266,700	19,156,300	4,672,800	23,829,100	5,437,600	18.6%
2008	28,063,000	18,417,900	4,927,000	23,344,900	4,718,100	16.8%
2009	29,360,700	18,405,500	4,145,500	22,551,000	6,809,700	23.2%
2010	25,142,700	15,676,300	3,924,000	19,600,300	5,542,400	22.0%
2011	26,513,000	16,094,700	4,217,500	20,312,200	6,200,800	23.4%
2012	27,219,500	16,280,300	3,903,100	20,183,400	7,036,100	25.8%
2013	26,832,800	15,687,900	3,660,300	19,348,200	7,484,600	27.9%
2014	26,088,800	14,778,500	3,410,600	18,189,100	7,899,700	30.3%
2015	23,237,700	13,547,100	3,153,300	16,700,400	6,537,300	28.1%

The wholesale water charge reforms mentioned above are designed to reflect the declining water demands and stabilize financial performance.

In April 2014, DWSD's largest wholesale customer, the City of Flint, formally notified DWSD that it would immediately be terminating service. Flint represented approximately 3% of total system revenue, and ceased purchasing water in May 2014. Genesee County, which was formerly served by DWSD through Flint, continues to purchase water, but has also notified DWSD that it is constructing its own facilities to supply water to its customers and will discontinue purchases from DWSD once those facilities are in service. DWSD estimates that service to Genesee County will be discontinued as of July 1, 2017. This represents an additional loss of 4% total system revenue to DWSD.

Over this same time period, DWSD has experienced an increase in non-revenue water. Non-revenue water is the difference between the amount of water treated at the plant and the amount of water billed to the customer. Nonrevenue water rose from approximately 17.6 percent in FY2001 to

approximately 28.1 percent for FY2015. DWSD has undertaken a number of initiatives to mitigate this loss, as described in previous sections of this report, and these initiatives, plus recommendations of this Water Master Plan Update are designed to reduce nonrevenue water below 15 percent by 2035.

10.3 Key Performance Indicators Relative to Peer Utilities

Many utilities like to get a sense of performance relative to their peers. One way is through evaluation of Key Performance Indicators (KPIs). KPIs are measures to assess operational performance and help provide context for future targets and improvement. Periodically, the American Water Works Association (AWWA) collects information on KPIs for a cross-section of utilities throughout the Country. Results of the most recent survey, Performance Indicators for Water and Wastewater Utilities: Survey Data and Analysis Report (2012), was used to compare DWSD performance against an aggregate representation of DWSD peers. While each utility operates under a unique set of circumstances and in a unique environment, it is often helpful to compare KPIs against a broad cross-section of peers. Table 10.1 shows DWSD data are for FY2014, except where information from the August 2014 refinancing was used for comparison to the Benchmarking Report.

The AWWA Benchmarking Survey collects information from a cross-section of utilities and represents those utilities that provide only water service to its customers, as well as utilities which provide both water and wastewater service to its customers. These are distinguished by “water only operations” and “combined operations” respectively. The survey compiles the results into an aggregate average for all participants, and then groups the metrics into quartiles indicating performance relative to the measure. So, for example, the top quartile for operation and maintenance cost indicates poorer performance than the bottom quartile, while the top quartile for cash reserves indicates better performance than the bottom quartile. Each quartile should be interpreted specifically for the metric.

Table 10-2: Comparison of Key Performance Indicators to AWWA Benchmark Survey (published 2014, 2012 data)

	Detroit Water and Sewerage	Water Only Operations			Combined Water & Sewer Operations		
		Top Quartile	Median Quartile	Bottom Quartile	Top Quartile	Median Quartile	Bottom Quartile
Debt Ratio (Total Liabilities/Total Assets*100%) – measure of utility indebtedness	102%	18.0%	34.0%	53.0%	21.0%	37.0%	53.0%
Cash Reserves (DAYS) – measure of financial liquidity – number of days of available cash reserves	246	474	265	159	391	225	118
O&M Cost Ratios							
O&M Cost/Customer Account	\$626	\$330	\$395	\$557	\$281	\$408	\$608
O&M Cost/MGD	\$782	\$1,853	\$2,425	\$3,313	\$1,873	\$2,565	\$3,406
Return on Assets – measure of estimate of financial effectiveness	0.28%	3.10%	2.20%	1.00%	2.60%	1.7%	0.50%
Staff Efficiency Ratios – measure of overall staff efficiency relative to operations							
MGD Water Produced/Employee	0.38	0.32	0.21	0.16	0.35	0.24	0.16
Disruption of Service <i>Planned outages/1,000 customers</i>							
Outage less than 4 hours							
Outage between 4 and 12 hours	0	0.52	1.27	3.07	0.14	0.77	4.56
Outage greater than 12 hours	0	0.08	0.44	1.12	0	0.15	0.68
	0	0	0	0.01	0	0	0.01
<i>Unplanned Outages</i>							
Outage less than 4 hours	0	0.38	2.23	3.9	0	0.75	3.39
Outage between 4 and 12 hours	0	0.18	0.75	1.57	0	0.09	0.57
Drinking Water Compliance Percentage (100% * # of days in full compliance / 365)	98%	100%	100%	100%	100%	100%	100%

In comparing KPIs to benchmarks it is important to recognize the unique configuration and customer composition of the DWSD system. Very few utilities in the United States are comprised of significantly more wholesale customers than retail customers (measured by population served), which is reflected in the performance of DWSD relative to peers.

Cash reserves for the utility compared to combined utility operations ranks between the top and median quartiles indicating solid financial liquidity. For O&M Cost ratio comparisons, a more effective measure of O&M costs for DWSD is the O&M cost/MGD treated. This measure is more reflective of the cost of delivering water to DWSD customers.

Difficult financial circumstances are reflected in several of the financial KPIs: Debt Ratio and return on assets. However the utility has taken a number of steps to move these KPIs from the bottom quartile. Recent staff restructuring efforts have resulted in improvement in the staff efficiency ratios and puts DWSD in the top quartile relative to combined water and sewer utility survey participants.

Water quality percentage in FY2014 was impacted by MCL incidents in the Trenton, MI wholesale customer system in August and September, 2013. There were no other violations in the system for the year. Following September 2013, there have been no violations in the Trenton, MI wholesale customer system.

10.4 Organizational Initiatives to Improve Performance

DWSD has taken a number of actions since 2012 to address the recent history of adverse financial impacts.

10.4.1 Regional Perspective and GLWA

DWSD management has taken a leadership role in the formation and transition to the Great Lakes Water Authority. The large percentage of sales from wholesale accounts was a primary financial driver for the transition to the Great Lakes Water Authority (GLWA).

DWSD actively leads efforts in reorganization transition activities for GLWA, and it provides financial analysis and monitoring for start-up in mid-2015 for GLWA and the redefined City of Detroit retail service provider arrangement. These activities involve implementing systems and reporting mechanisms that support bond indentures and a “credit positive” objective for the GLWA and City of Detroit. The DWSD Water Rates working group together with Board of Water Commissioners and Great Lakes Water Authority have been meeting to address the issues with revenue forecasting.

The goal in creating the GLWA, along with a new retail organization for the City of Detroit’s distribution system is to enable long-term financial sustainability of the system.

10.4.2 DWSD’s Internal Strategic Initiatives

In addition to its leadership in the formation of GLWA and City of Detroit retail water operations, DWSD is performing a wide range of strategic initiatives for performance optimization and improvement. Key initiatives are described below:

Enhanced Information Technology

1. Completion of Enterprise network transformation and implementation of managed datacenter and disaster recovery services.
2. Implementation and enhancement of core systems including ERP, GIS, Work and Asset Management, Enterprise Content Management and Reporting
3. Delivery of year 2 of IT transformation of staff to match organizations needs and structure

Water Operations

1. Investigate energy contract, equipment use, and energy rebate initiatives to further reduce energy consumption and expense.
2. Maintain 100% compliance – the delivery of pure safe drinking water is an essential element of DWSD’s mission.
3. Investigate new programs around efficiency in utilities and chemicals and evaluate the investment versus return on investment for each of the programs.

Field Services

1. Focus on Water Loss Prevention through the use of Detroit Delivers and Click Fix, together with improved, coordinated, and timely response (48 hours to abandoned building running water reports, repair of main breaks within 4 days, will help reduce unaccounted for water loss within the system. This will result in improved customer experience as well as reduce system expense.
2. Address reports of running water within abandoned buildings within 48 hours
3. Valve and gate assessment project
4. Hydrant assessment time frame implementation (minor=4 days, major=7 days)

Customer Service

1. Update interim collection policy and procedure
2. Retail customer portal and app development and launch

Organizational Development

1. Complete placement of all positions in new classifications
2. Move to implementation of enterprise resource planning through cross-functional collaboration. Will result in improved decision making for stakeholders, efficient operations and effective internal controls.
3. DWSD will continue the staff optimization project started in Fiscal Year 2012. Staffing goals will be based on forecasted investment in technology to ensure changes will not affect levels of service.

Financial

1. Implement Phase 2 of Finance Transformation – Move from addressing backlog and first layer of business process redesign and staff re-deployment to building best financial management practices state to result in operational efficiency and deployment of cost-effective strategies.
2. Improved monthly financial reporting and cash flow analysis for stakeholders. Will result in improved financial management and decision making.
3. Establish Performance Benchmarks of +/- 2% of actual sales to projected sales revenue net of bad debt expense. It is anticipated that operating within this benchmark will improve financial performance, bond ratings, less reliance on debt to finance capital improvements, and a resultant lower cost of borrowing over time.
4. Reduction of lower variable operations and maintenance costs. The continuation of staffing optimization efforts has resulted in a decrease of 37 percent of the positions at DWSD since the beginning of FY2012 and savings in personnel related costs.

5. Annual debt service coverage is set forth in accordance with Board policy which establishes coverage targets at least 15 percentage points higher than bond ordinance covenant requirements. The most recent official statement indicates that DWSD has consistently ensured sufficient revenue to meet these requirements.

10.5 Review of FY 2016 Budget and Capital Improvement Plan

10.5.1 20 Year Capital Improvement Plan

The 20-year Capital Improvement Plan is presented in Technical Memorandum 17 in an appendix to this report. A summary of the CIP is presented in **Table 10-3**. The proposed CIP identifies significant capital improvements to the regional water system of the new GLWA and the distribution system of the City of Detroit over the next 20 years. While all known variables have been taken into account as part of this plan, over such a long time span there will be many unknown variables which can impact the need for capital improvements for the system.

Table 10-3: Summary of Proposed 20-Year Capital Improvement Program³

Asset Group	Program	GLWA	City of Detroit	Total
Treatment	Repurpose the Northeast Plant	\$ 230,860,000	\$ -	\$ 30,860,000
	Regulatory Compliance	\$ 117,909,000	\$ -	\$ 17,909,000
	Treatment Renewal, Reliability and Energy Management	\$ 1,019,996,601	\$ -	\$ 1,019,996,601
Transmission	Decommission Certain Booster Stations	\$ 23,337,253	\$ -	\$ 23,337,253
	Optimize Service Delivery	\$ 165,443,550	\$ -	\$ 165,443,550
	Improve Transmission Redundancy	\$ 82,947,000	\$ -	\$ 82,947,000
	Transmission and Reservoir Renewal and Reliability	\$ 927,846,205	\$ -	\$ 927,846,205
Distribution	Piloting New Water Main Renewal Technologies	\$ 13,500,000	\$ -	\$ 13,500,000
	Water Main Renewal	\$ -	\$ 448,798,000	\$ 448,798,000
	Water Main Retirement	\$ -	\$ 13,770,000	\$ 13,770,000
Metering	Plant Production Metering	\$ 21,000,000	\$ -	\$ 21,000,000
	Wholesale Customer Metering	\$ 47,250,000	\$ -	\$ 47,250,000
	Completion of Detroit AMR Metering	\$ -	\$ 20,000,000	\$ 20,000,000
Total		\$ 2,650,089,610	\$ 482,568,000	\$ 3,132,657,610

³ The total cost of the CIP shown in the table includes several alternative projects. Depending on which alternatives are chosen, the total cost of the CIP ranges from approximately \$2.8 billion to \$3.0 billion.

In anticipation of this variability, the Water Master Plan Update evaluated four scenarios for water sales over the 20 year planning period.

1. Most Probable – this scenario provides neither an overly optimistic nor overly pessimistic forecast of water sales.
2. Best Case – this scenario provides the most optimistic view of water sales.
3. Worst Case – this scenario provides a moderately pessimistic view of water sales.
4. Worst-worst case – this scenario builds upon worst case and considers possible further impacts to the worst case.

Table 10-4 shows in detail the inputs as classified by each of the scenarios.

Table 10-4: Detailed inputs as classified by each of the scenarios

Scenario	Population	Domestic Per Capital Water Consumption	Commercial and Industrial Water Consumption	Real Water Loss for Wholesale Customers	Real Water Loss for Detroit
Most Probable	SEMCOG forecast updated by wholesale customers	Decrease 5% by 2035	GPED decreases 5% by 2035	Decrease 0.5% per year	Decrease 0.5% per year
Best Case	4% higher than Most Probable	Stays at current level	Stays at current level	Stays at current level	Decrease 1% per year
Worst Case	4% lower than Most Probable	Decrease 10% by 2035	GPED decreases 10% by 2035	Decrease 1% per year	Stays at current level
Worst Worst Case	10% lower than Most Probable	Decrease 10% by 2035	GPED decreases 10% by 2035	Decrease 1% per year	Stays at current level

Figure 10-1 shows the projections of water sales and Table 10-5 shows the projections for each scenario.

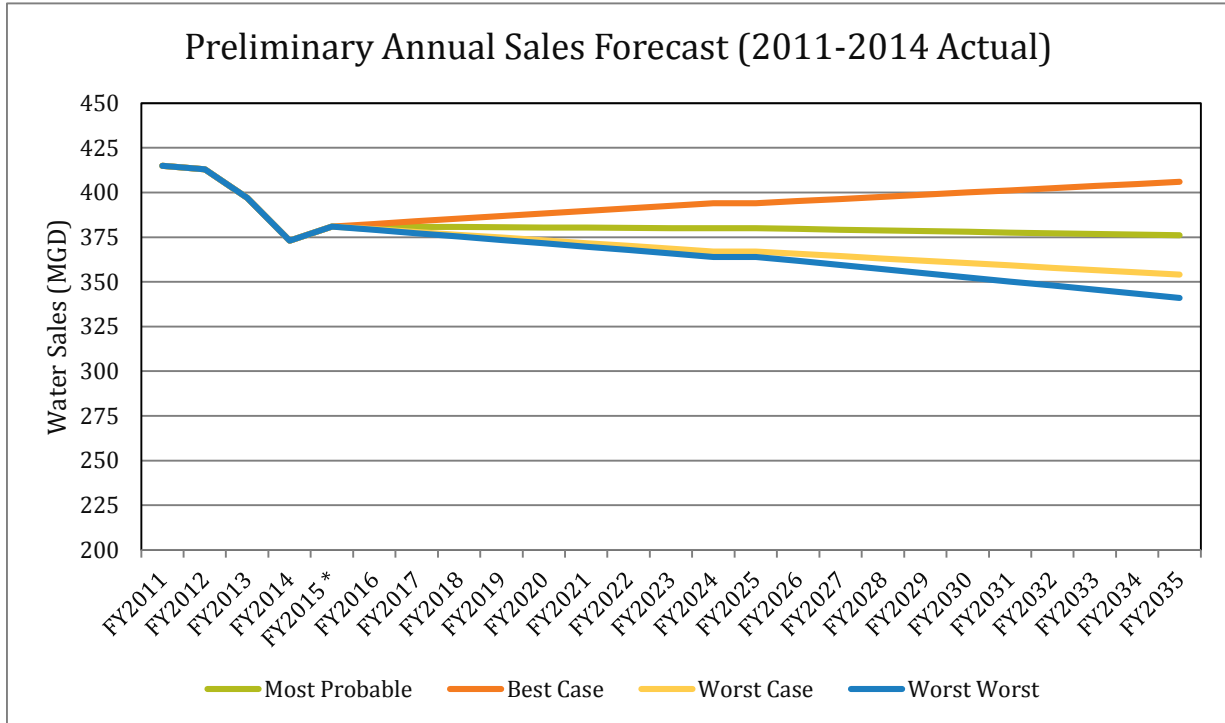


Figure 10-1: Projections of Water Sales under Different Growth Scenarios

Table 10-5. Preliminary Forecast of Water Sales

Year	Most Probable	Best Case	Worst Case	Worst Worst
FY2011	415	415	415	415
FY2012	413	413	413	413
FY2013	397	397	397	397
FY2014	373	373	373	373
FY2015*	381	381	381	381
FY2016	381	381	381	381
FY2017	381	381	381	381
FY2018	381	381	381	381
FY2019	381	381	381	381
FY2020	381	381	381	381
FY2021	381	381	381	381
FY2022	381	381	381	381
FY2023	381	381	381	381
FY2024	381	381	381	381
FY2025	380	394	367	364
FY2026	380	394	367	364
FY2027	380	394	367	364
FY2028	380	394	367	364
FY2029	380	394	367	364
FY2030	380	394	367	364
FY2031	380	394	367	364
FY2032	380	394	367	364
FY2033	380	394	367	364
FY2034	380	394	367	364
FY2035	376	406	354	341

*FY2015 values were estimated based on a weighted average of FY2013 and FY2014 values.

As noted in the opening of this Financial Analysis, there are independent forecasts of long-term DWSD financial performance are being prepared by others. The projections in those forecasts are intended to facilitate evaluation of the establishment of independent wholesale and retail utilities, as envisioned by the Great Lakes Water Authority, and to support refinancing a portion of the current DWSD debt portfolio. The independent projections utilized the projected 20-year capital improvement program and referenced the Water Master Plan Update's assumptions regarding future water sales and demands. The results of the independent projections support an overall strategy of gradually employing more revenue (and less debt) financing for capital, and indicate that financing the proposed capital improvement program can be supported within the four percent annual revenue requirement increases envisioned by the Memorandum of Understanding for establishing the Great Lakes Water Authority.

11 Implementation

11.1 Introduction

This chapter recommends early activities for implementing the recommendations of the Water Master Plan Update. It includes:

1. Policies to Implement the Water Master Plan Update
2. Project Initiation Activities
3. CIP Execution Management

11.2 Policies to Implement the Master Plan

It is anticipated that GLWA and DWSD-Retail will develop a wide range of policies and procedures to direct their respective organizational missions. New or updated policies are anticipated for finance, budgeting, procurement, document management, design, construction, customer service, human resources, and other business areas. Policies will be important tools for communication from the GLWA and the City of Detroit to their staff, customers, and stakeholders. Policy communications will provide organizational alignment for staff, and will communicate priorities and values to customers and stakeholders.

Certain policies will be important to the successful implementation of this Water Master Plan Update. These policies cover the following areas:

- Planning Processes
- Water Rates
- Water Audits
- Wholesale Customer Service
- Asset Management

In the sections below, key questions and issues to be addressed by new policy are listed. These questions and issues are based on discussion of a preliminary framework of policy ideas among DWSD staff and retail and wholesale customer representatives. It is recommended that GLWA convene a Technical Advisory Committee Policy Work Group in the fourth quarter of 2015 to develop policy statements, procedures, measures of effectiveness, and the process to update policies in the future. Similarly, DWSD-Retail would convene a working group to develop policies and procedures to successfully implement its new mission in water distribution for the City.

11.2.1 Planning Processes

Both GLWA and Detroit should develop policies for water supply planning. DWSD had a strong planning group which guided system expansion from the 1940s to decades thereafter. Going forward, planning must strike a balance between the economic development and sustainability goals of the region. Recently, the City of Detroit hired a new City Planner who will address the full range of city services and redevelopment, including those for water, sewer, and green infrastructure issues. There is discussion within GLWA regarding the formation of a planning group. New planning policy should address the following questions and issues:

Who will be responsible for planning?

- Role and responsibilities of dedicated groups for GLWA and DWSD-Retail.
- The relationship between planning and development of 5-year annual capital improvement plans

What processes will be used?

- How to integrate information from the recently introduced new process of 5-year wholesale contract negotiations into planning forecasts
- How to integrate information from the annual Lease Renewal process
- Processes for data collection, regional forecasts, condition assessment, and protocols to update planning projections every 5 years, or other designated frequency
- Sharing of data between GLWA and wholesale customers, such as: water sales data, hydraulic modeling tools and results, technology
- Tracking progress on execution of the CIP and updated needs assessments
- Processes for the City of Detroit to create geographic alignment of multiple programs for home ownership, redevelopment, new infrastructure, and retirement of infrastructure that has reached its service life

How will planning be coordinated and communicated?

- Sharing of data with SEMCOG, MDOT, DTE and others
- Annual planning conference
- Information technology applications for reporting progress on projects and assessing new needs to be addressed through planning

11.2.2 Water Rates

There are ongoing initiatives related to water rates, including rate simplification and reallocation for fixed costs and water purchase costs. Energy is a major factor in the cost of delivering wholesale water supply. Across the USA, one third of all electrical energy is used for moving water or wastewater. For DWSD, energy represents 25 percent of the water operations budget and 40 percent

of its pumping and treatment budget. DWSD is one of a relatively few regional water utilities in the United States that provides peak hour water supply to wholesale customers.

To build on the accomplishments of the Technical Advisory Committee Water Rates Work Group, and to recognize the impact of energy costs of water rates, new policies for GLWA should address the following questions and issues:

How will rates take into consideration the repurposing of one or more water treatment plants in the elevation and distance formula?

- Review applicability of elevation and distance formula for water rates

Should rates consider more directly the cost of energy to serve customer pressure requirements?

- Evaluate whether to align water rates to pressure, volume and location of actual metered water use, rather than meter size and location.
- Evaluate surcharges for water supplied at the highest grade line for customers who have multiple meters.
- Evaluate requests for higher volume and pressure by particular customers.

How can the use of energy be made more immediate to GLWA operators and wholesale customers?

- Establish real time measurement and reporting of energy use at all pumping stations
- Provide additional training to System Control Operators on methods to reduce demand charges
- Establish annual goals for energy use, renewable energy supply, and sustainability indicators.

What additional information should be considered in the process of setting water rates?

- Consider financial performance and key performance indicators (KPI) in Table 10-2 with quarterly KPI updates.

11.2.3. Water Audits

DWSD's own work, and confirmed by this Water Master Plan Update, have demonstrated the magnitude of nonrevenue water and the need to reduce this operating metric. Water supply is abundant, but nonrevenue water diminishes operating efficiency, distracts management focus, and clouds capital planning decisions. DWSD has performed water audits since 2006 in accordance with standards of the AWWA. Continuing water audits conforming to AWWA standards are recommended in the future, coupled with greater participation by wholesale customer representatives in the audit processes. This Master Plan Update recommends production metering at all treatment plants, new wholesale metering for Detroit, Dearborn, and Highland Park and continuing investment in upgrade and improvement of existing wholesale customer meters. This Master Plan Update also recommends new methods for leak detection and pressure management, with district metering areas, to supplement existing techniques for water loss reduction within the City of Detroit

New policy for GLWA should address the following questions and issues:

What goals and schedules should be established?

- Schedule to complete production metering improvements
- Time period to address identified wholesale metering problems when they arise
- Schedule for metering Dearborn, Detroit and Highland Park

How can water audit and leak detection and correction processes be made more effective?

- Regular auditing of wholesale meters and creation of a task force analogous to the Wastewater Flow Metering Task Force
- Testing program for meters
- Implement information technology “dashboard” that displays goals and results

What leak detection and metering programs will the City of Detroit prioritize?

- Completion of AMR meters.
- Procedures for non-AMR customers and estimated bills.
- Goals and benchmarks for improving collections over the planning period.
- District Meter Areas as a temporary approach while wholesale metering is being implemented.

11.2.4 Wholesale Customer Service

DWSD has conducted ambitious and effective customer outreach and involvement programs for its water customers since 2003. Similar programs for wastewater customers were started in the 1990’s. It is understood that both GLWA and City of Detroit plan to continue customer involvement as a key tool for management of their operations as well as to assure effective customer service.

New policies should be based on the practices that have been most effective in the past, and what new emphasis is needed in the future. Every aspect of wholesale customer service should be considered, from marketing to potential new customers, to water service contracts, and programs to involve customers in working groups. New policies should address the following questions and issues:

Should marketing and new forms of water service be offered?

- Process for marketing, incentivizing, identifying and facilitating new wholesale customers who seek water service.
- Identify new forms of water service– low pressure (delivered at 35-PSI and new customer boosts its own pressure), raw water, and seasonal service.
- Different water rate structure for a different type water service?
- Re-evaluate the current Growth Pays for Growth policy; consider business case issues and benefits for the system for each situation.

How can water service contracting procedures be improved?

- Procedure for updating existing water service contracts, when needed.
- Procedures and timelines for handling expiring contracts.

How can regional water quality services be extended?

- Provide regional information and messaging in the annual Consumer Confidence Reports (CCR) for each wholesale customer
- Market regional water quality testing and compliance reporting services to wholesale customers

How can emergency response notification and communications be improved?

- Re-set emergency response objectives and notification processes
- Use **Figure 11-1** as a point for discussion with wholesale customers.

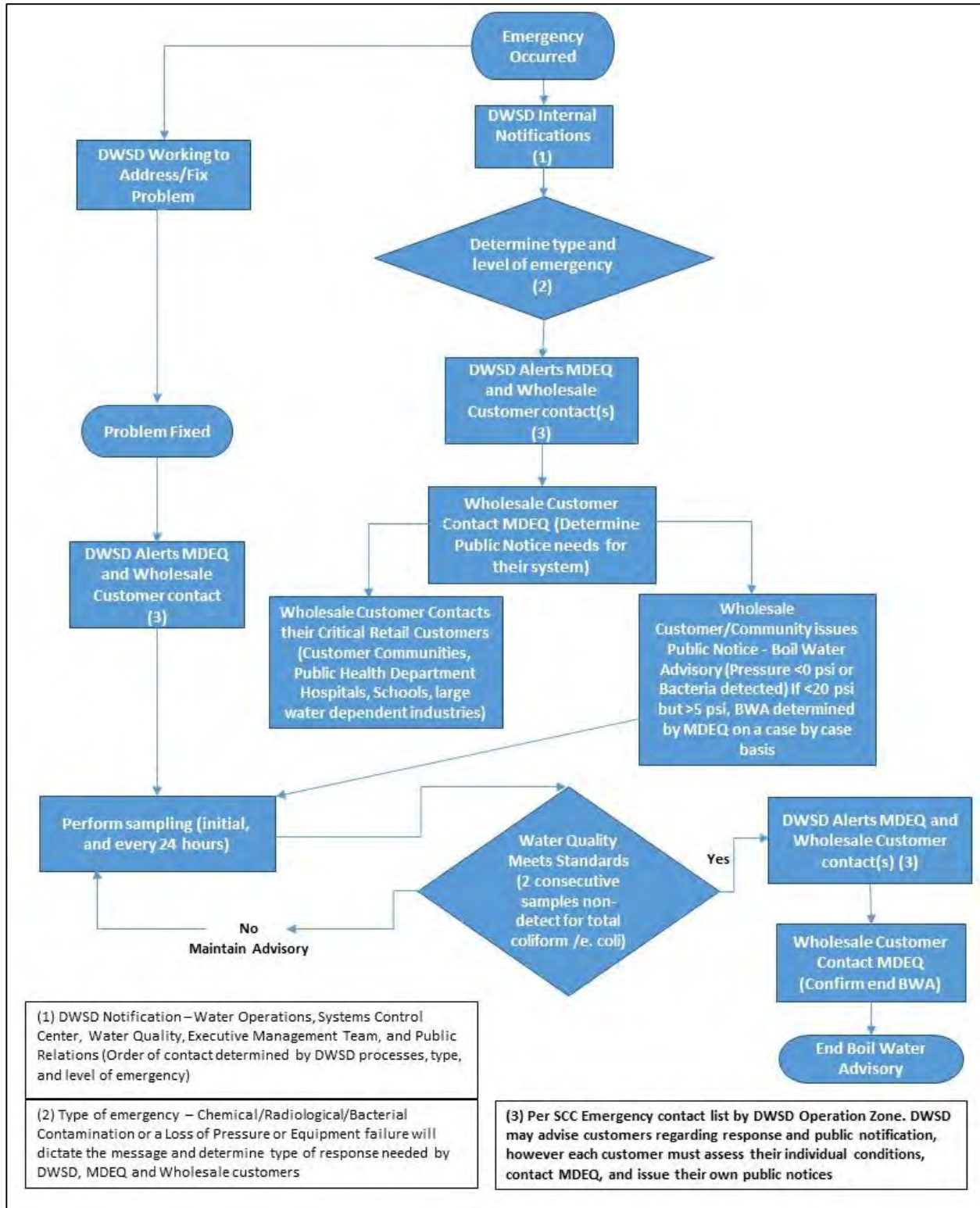


Figure 11-1: Preliminary Emergency Response Notification Flow Chart

This preliminary flow chart captures a simplified representation of the current notification process between DWSD, Wholesale customers, MDEQ, and the public (via issuance of Public Notices). The first line of defense for preventing an emergency situation or handling a minor emergency before it

becomes a notification situation are the immediate actions of the trained water treatment plant operators and system control center operators. When the system conditions or emergency occurrence is beyond a readily correctable condition, then the DWSD internal notification process begins.

For situations that originate at a water treatment plant, notification goes from the operator or chemist to their team leader then the plant manager. The plant manager notifies the Water Operations Director. The Water Operations Director notifies all the other parties within DWSD who need to be involved. This may include the Executive Management Team and Public Affairs. If it is a water quality emergency that will include notification to the Water Quality manager. For situations that originate from a loss of pressure or transmission main break, the System Control Center (SCC) is typically the first group to be aware of the emergency. The SCC manager will notify the Water Operations Group manager and then determine who will contact the Executive Management Team and Public Affairs.

As indicated by the flow chart, DWSD (typically the Water Quality manager) will notify both MDEQ and the affected wholesale customer(s). DWSD will work with MDEQ and customer(s) to inform them of the Water Supply Conditions: Bacteriological, chemical, radiological maximum contaminant level (MCL) violation, treatment technique (TT) violation, Loss of Pressure (<20 psi) or other water quality events and advise them on determining system conditions, contacting MDEQ, and any other next steps. The direction that DWSD has received from MDEQ is that each community/customer is responsible for issuing public notices for boil water advisories based on MDEQ protocols and the individual community's internal protocols for notifications.

In parallel to these notifications the various group managers are working with operators and staff to address the emergency situation to restore service and maintain water quality. Site-specific Emergency Response plans are also available at each WTP and should be followed.

DWSD will notify MDEQ and the customer(s) when the problem is fixed. Sampling is required for at least 24 hours after the pressure is restored, until bacteria is below detection in two consecutive samples. The Customer should stay in contact with MDEQ regarding the determination to lift the boil water advisory.

The process described above will change in some respects with the transition to the GLWA.

11.2.5 Asset Management

Asset management is a systematic approach to monitoring, maintaining, and replacing the regional water infrastructure. At this scale, the regional DWSD water system has 500,000 to 1,000,000 or more individual components of equipment and infrastructure, depending on how these pieces are counted. Within the City of Detroit, there are additional water assets that exceed 400,000 when counting customer meters, pipe line by the block, hydrants, and valves. The annual volume of maintenance work orders on these facilities exceeds 25,000 per year.

Computerized systems for asset management, work order management, and maintenance management allow for data to be collected and reported on age, frequency of maintenance and condition. Much of DWSD's system was built for historically larger manufacturing water use and a larger population, therefore there are now excess assets in some locations.

Policy should address the following questions and issues:

How should asset management data be utilized for GLWA and DWSD-Retail planning and reporting?

- Role and responsibility of an Asset Management Group
- Objectives for asset management and reporting on assets.
- Improving Asset Management capabilities so that the system can provide a preliminary report on capital replacement needs, and this report would then be reviewed and vetted by a committee knowledgeable of the operating and capital needs.
- Role of an advisory group of staff, customers and consultants to continuously review the quality of asset management data.

What new procedures are needed for asset ownership and costs?

- Procedures to transfer ownership of water facilities, either from wholesale customers to the regional utility, or from the regional utility to wholesale customers.
- How the system is reimbursed for assets that transfer, to assure that parties remain financially whole; consideration of who is impacted by the transfer of an asset.
- Handling of stranded assets, and handling of excess assets.
- Consider adding a new provision in the Water Service Contract for stranded asset, similar to that in the Wastewater Service Contract.
- Ownership and responsibility for pressure reducing valves at wholesale customer metering points.
- Removal of PRVs that are no longer needed.

11.3 Project Initiation

It is anticipated that after GLWA begins operational responsibilities, it will implement the regional capital improvements and operations, and City of Detroit will implement projects for its retail system. DWSD is expected to have a transitional implementation role as requested by either GLWA or City of Detroit, depending on the timing when the new organizations are fully authorized.

The 20-year capital improvement program provided in TM-17 identifies projects for GLWA and for City of Detroit. Approximately 30 percent of the projects are for implementation by the City of Detroit, and approximately 70 percent by the GLWA.

The 20-year capital improvement program identifies for each project a 'project type'. Major project types include:

- Study
- Design/Construction

- Design-Build
- Program Management
- Small Capital

The approach to initiating a project varies by project type.

Study Projects: Approximately 10 percent of the projects are of the study type. These projects would include collection and review of new information, development of criteria for evaluation or design, evaluation of alternatives, and development of recommendations.

Most study projects would lead to subsequent design projects.

Small Capital Projects: The Small Capital Program has an upper limit of \$2,000,000 per project, which includes engineering and construction. These projects are typically executed by DWSD staff and on-call suppliers, or by as-needed engineering services providers.

Design Projects: Approximately 90 percent of the projects are design followed by bidding for construction, design-build, or the Small Capital Program. Where a project type indicates Design/Construction, it is implied that design would proceed directly from this Water Master Plan Update. Design should include the three major tasks shown in Table 11-1. In the Preliminary Design task, the work includes review of the information in the Water Master Plan Update and other previous applicable studies, collection and review of new information, development of design criteria, evaluation of alternatives where appropriate, and development of a recommended alternative for final design.

Table 11-1: Standard Design Tasks

Task Name	Purpose	Key Milestones	End Product
Preliminary Design	<ul style="list-style-type: none"> ▪ Review master plan recommendations ▪ Add project specific criteria and new information ▪ Provide for value engineering, where needed 	<ul style="list-style-type: none"> ▪ Workshop on data sources and design criteria ▪ Workshop on alternatives ▪ Workshop on recommended design ▪ Value Engineering, if applicable 	<ul style="list-style-type: none"> ▪ Basis of Design Report (30% design) ▪ Updated Cost Estimate
Design Development	<ul style="list-style-type: none"> ▪ Initiate permitting ▪ Final coordination with other utilities, projects 	<ul style="list-style-type: none"> ▪ 60 percent design ▪ Specifications ▪ 60 percent design ▪ List of permits 	<ul style="list-style-type: none"> ▪ Design Development Report, Drawings, Materials and Equipment Specifications
Final Design	<ul style="list-style-type: none"> ▪ Finish permitting ▪ Procure Construction 	<ul style="list-style-type: none"> ▪ 90 percent ▪ 100 percent ▪ Construction contract documents 	<ul style="list-style-type: none"> ▪ Contract Documents ready for bid

Program Management: DWSD has used program management to execute multiple projects that are related by location or by type of facility. Initiating new projects under a program management approach is applicable when there is sufficient information about the scope, costs, and risk management issues for the projects to be executed. Program management can be an effect method for project execution where cost, schedule, and risk can be reduced.

11.4 CIP Execution Management

The proposed 20-year capital improvement plan includes over 330 projects. Certain projects need to be completed within the first five years of the planning period in order to provide the greatest benefit for operating cost savings and avoidance of costs for new equipment in excess facilities. Certain projects are related to potential future regulatory requirements, which may, or may not take effect during the planning period. Other projects, such as transmission main condition assessment, have discretionary starting dates, but should be initiated in the first half of the planning period.

The projects in the 20-year CIP have been organized into a series of programs which group projects by specific objectives of the Master Plan Update. See Table 11-2. Each program should be led by a senior project manager, and major responsibilities should include:

- Manage the implementation process in accordance with the recommended schedule and estimated cost.
- Make additions and changes to the recommended projects based on new information in the future.

- Manage related annual operational initiatives that augment the capital projects and measure results achieved.
- Provide quarterly progress reporting to management relative to the approved CIP schedule and budget.

The first five year programs would sunset when the objectives are complete, anticipated to be the year 2020.

Table 11-2. Proposed Programs for Managing the Execution of the 20-Year CIP

Time Frame	Treatment	Transmission	Distribution	Metering & NRW
First Five Years	Repurposing Northeast WTP	High Lift Pumping Station Optimization		Production Metering
Full Planning Period	Regulatory Compliance	Renewal and Replacement	Renewal and Replacement	Wholesale Customer Metering
	Renewal	Booster Pumping Station Optimization	Lead Service Connections	Water Loss Reduction
		Service Optimization	AMR Metering	

The Northeast Repurposing Program should be completed within the first five years. A preliminary schedule for execution of this program is shown in **Figure 11-2**. The schedule is based on the following criteria:

- Accurate condition assessment of existing infrastructure before design for rehabilitation.
- Increase the reliability and serviceability of the 96-inch main by installation of new isolation gates
- Minimize risk by scheduling the use of the existing Garland main during fall-winter-spring (not summer).
- Minimize the number of years that Northeast plant would remain in operation.
- Discuss the anticipated pressure changes shown in **Table 7-11** with Madison Heights, Troy, Sterling Heights, and Warren.

THIS PAGE INTENTIONALLY LEFT BLANK.

THIS PAGE INTENTIONALLY LEFT BLANK.